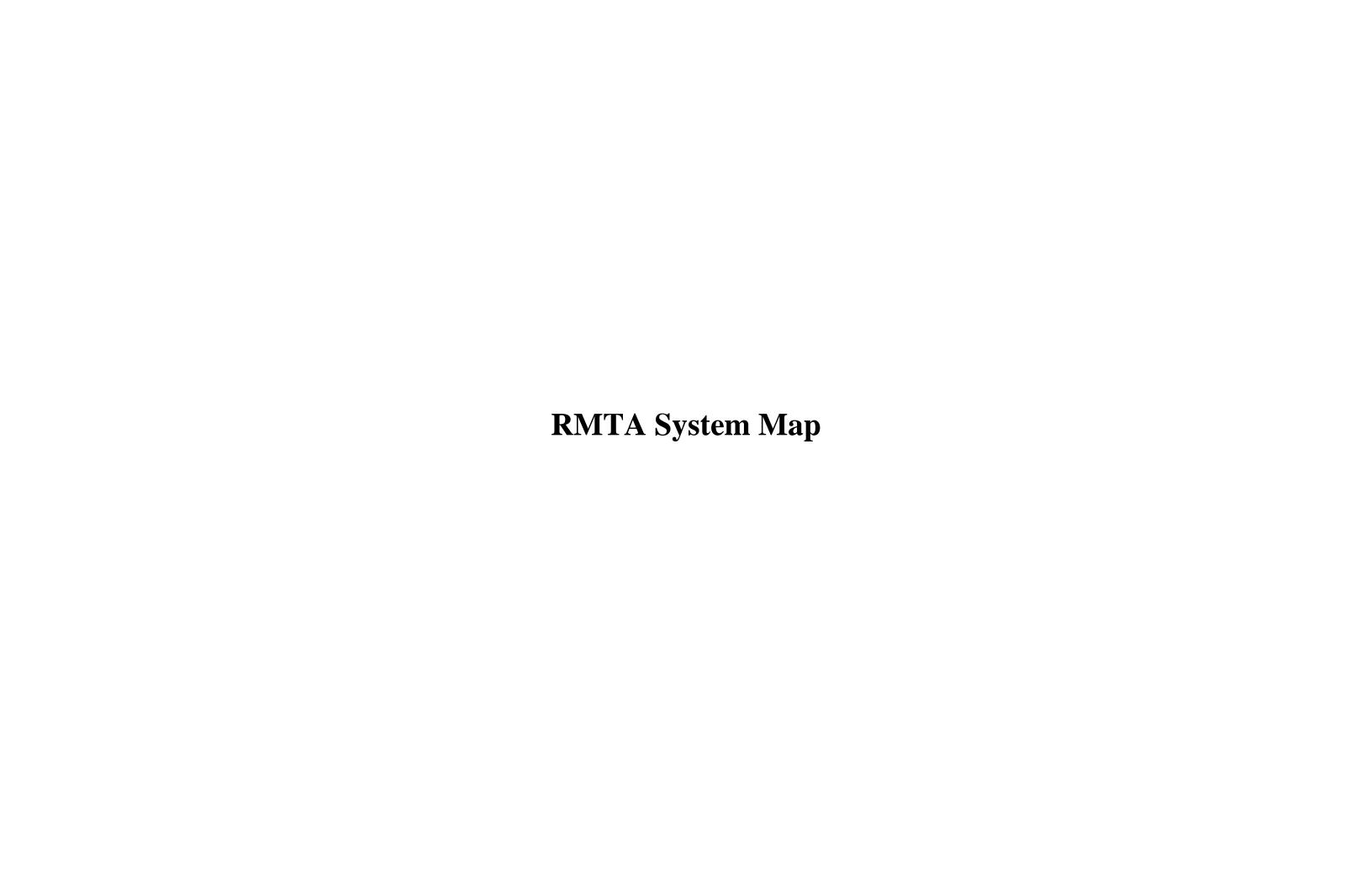
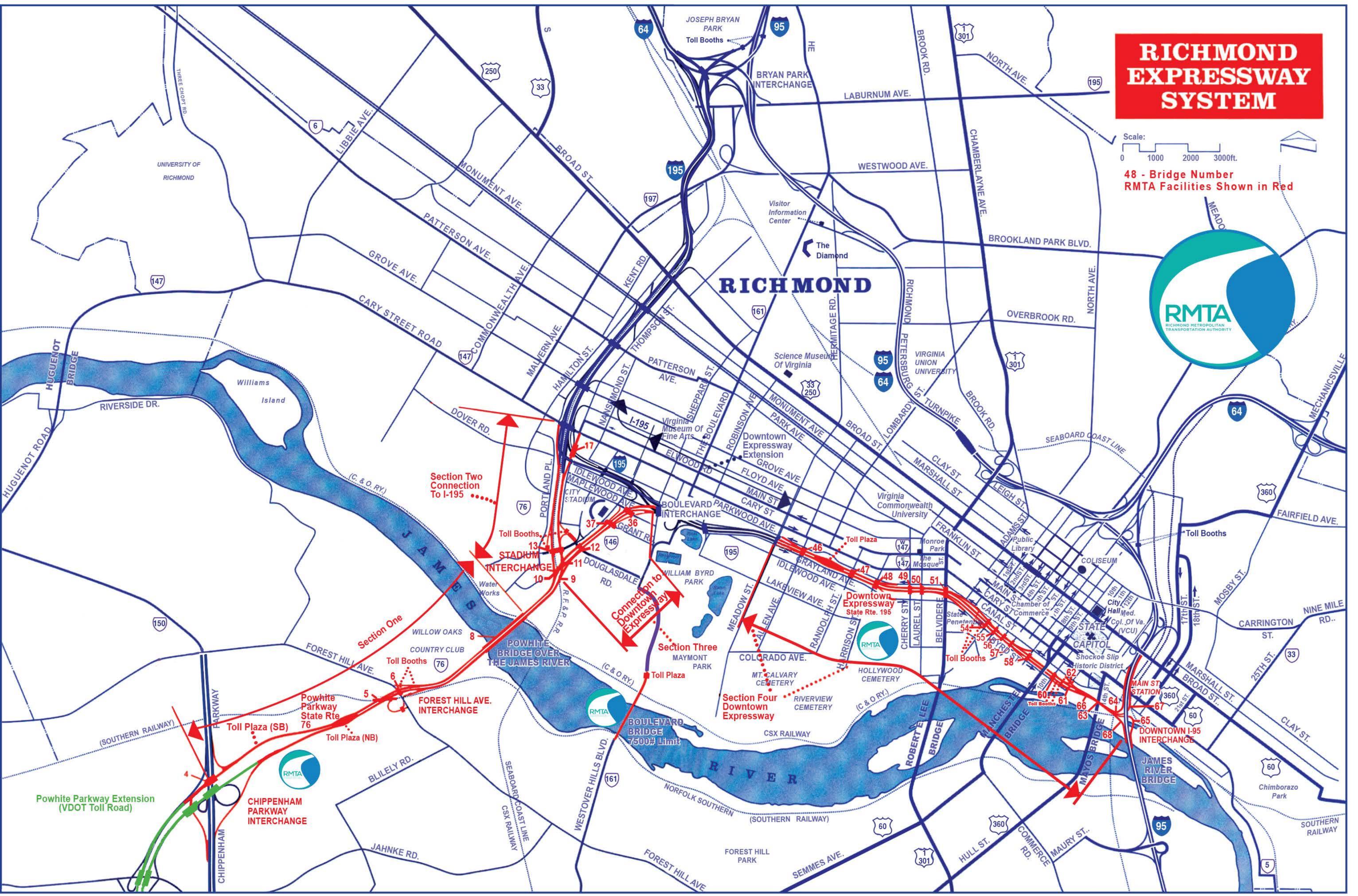
APPENDIX DR-2018

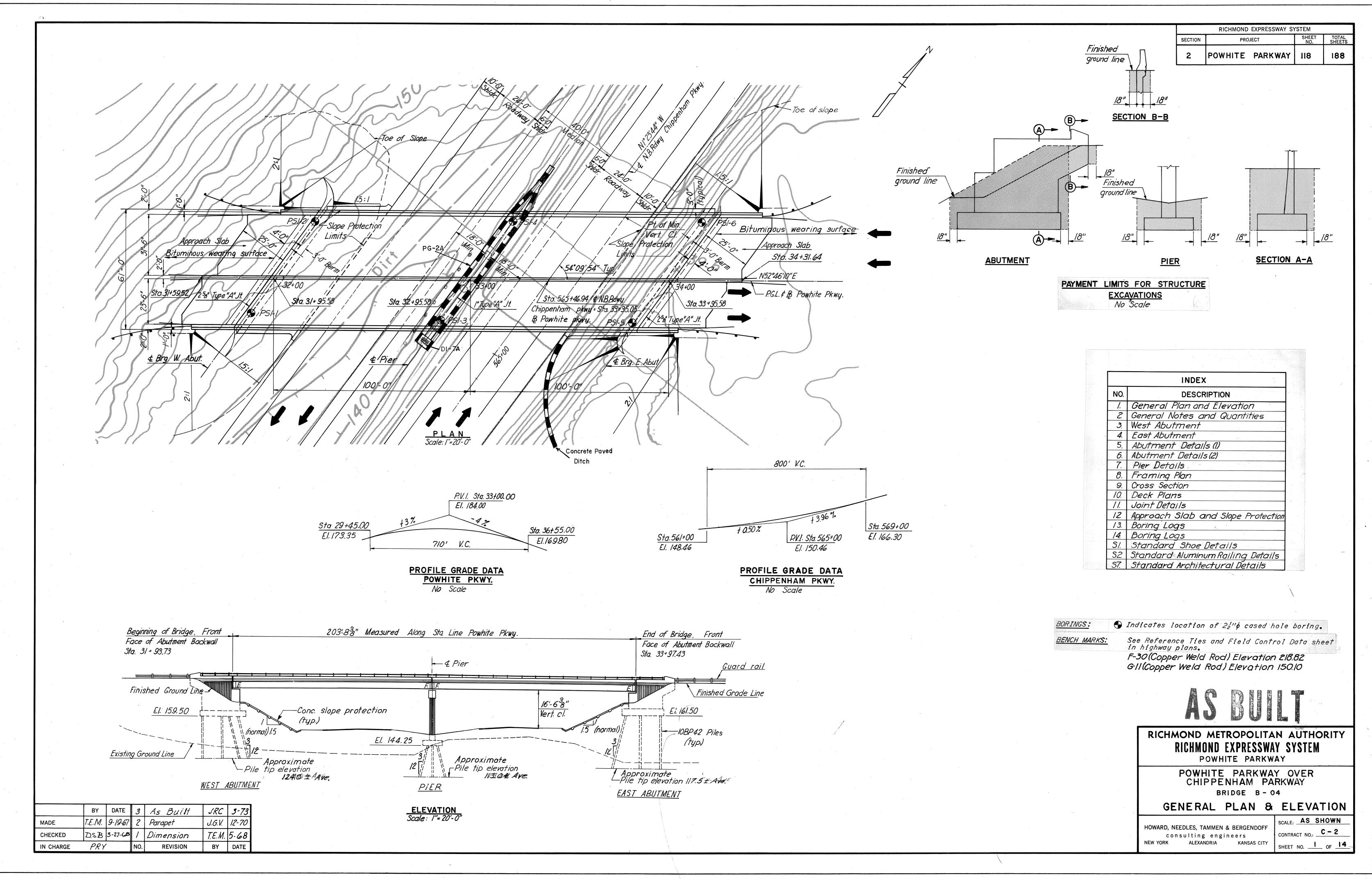
RECORD DRAWINGS

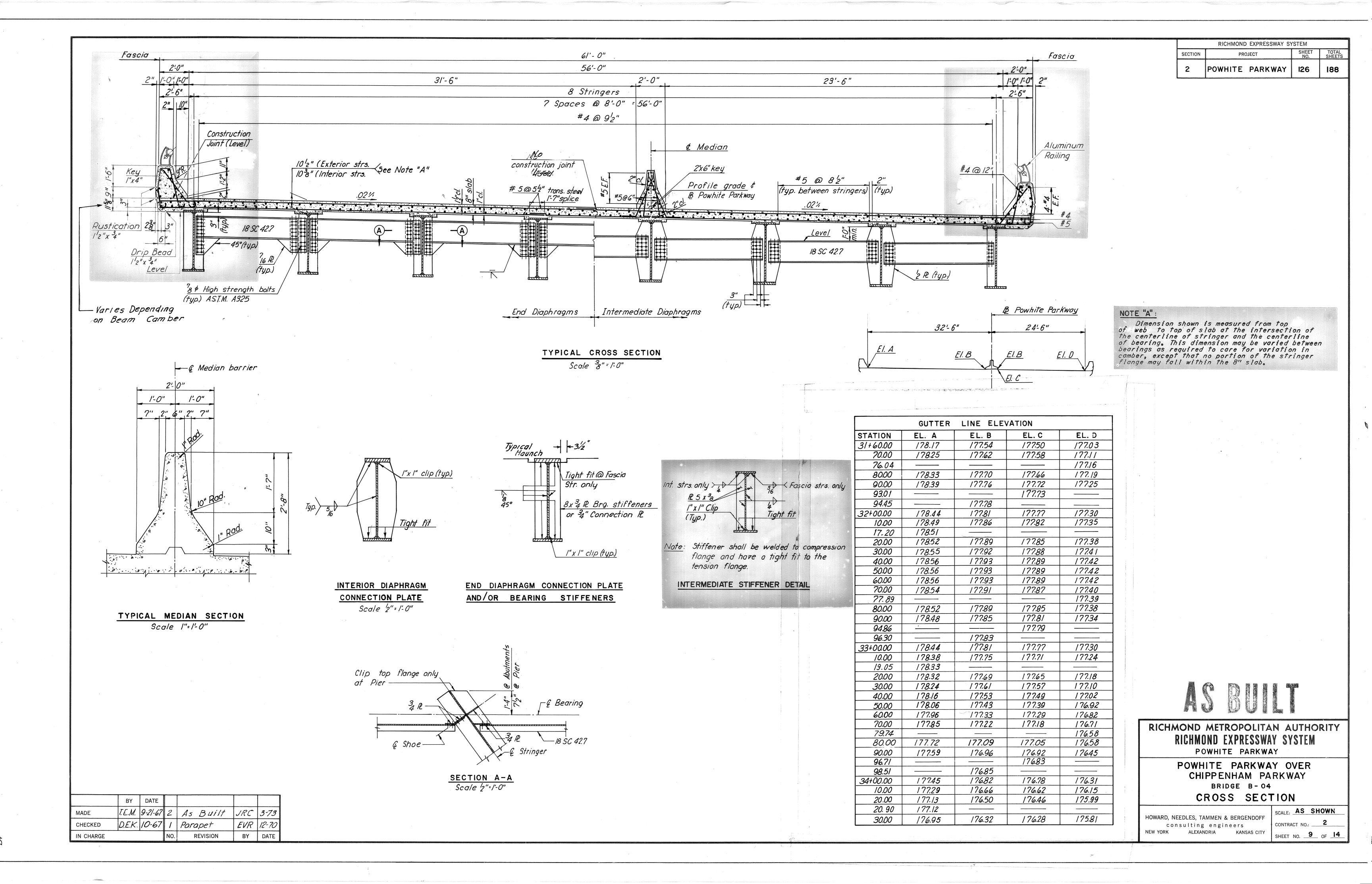
RMTA BRIDGES 4, 36, 46, 47, 48, 49, 50, 51, 54, 55, 56, 57, 58, 60 & 62



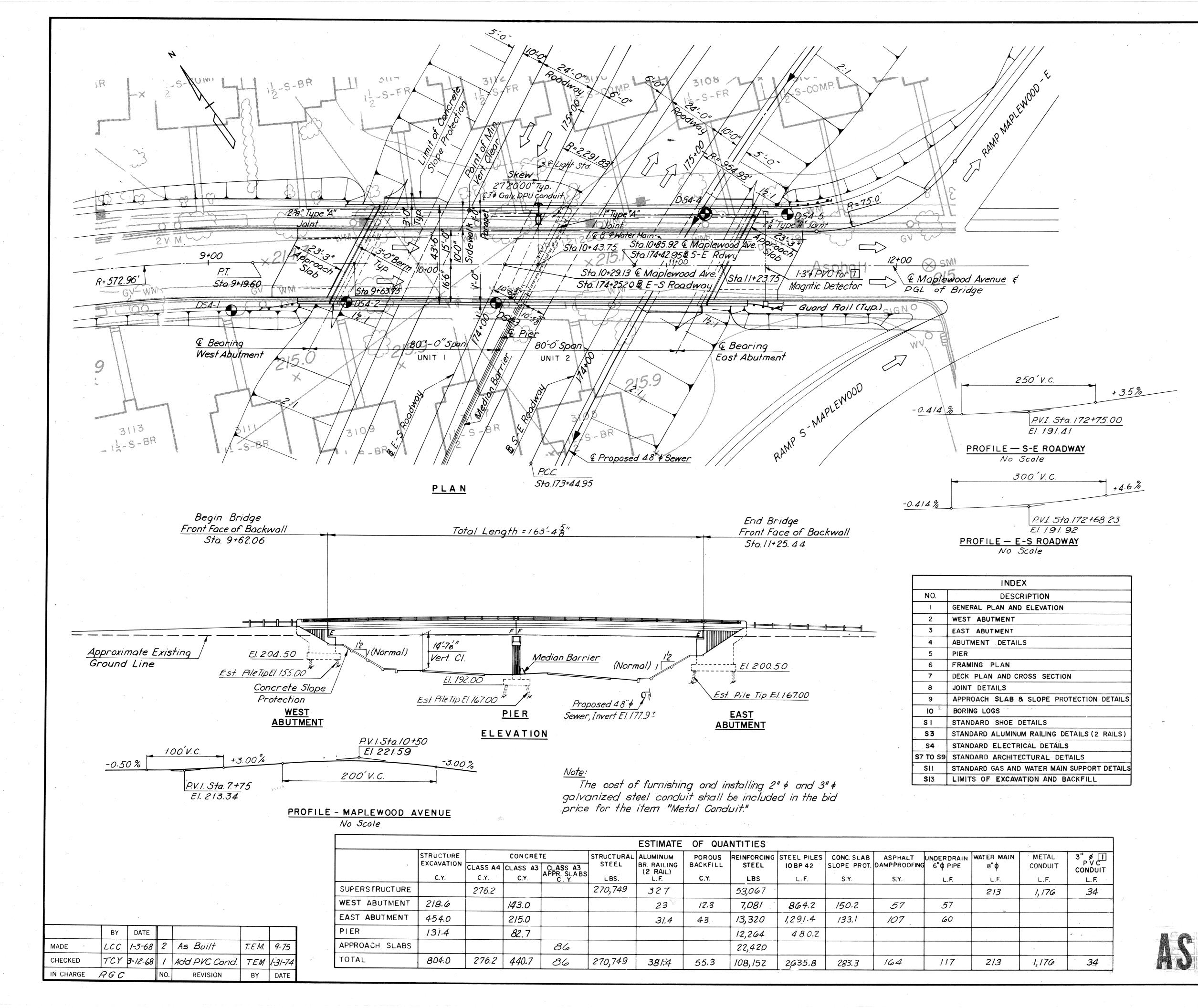


(Southbound Powhite Parkway {Rte. 76} Over Chippenham Parkway {Rte. 150})





(Maplewood Avenue Over Downtown Expressway Connector {Rte. 146})



RICHMOND EXPRESSWAY SYSTEM **PROJECT** 96 DOWNTOWN EXPRESSWAY

GENERAL NOTES

36-1" Face of rail to edge of sidewalk. ROADWAY:

Dead Load-Includes 15 lbs. per sq. ft. for future wearing surface. Live Loads-HS20-44 loading and B.P.R. modified for military vehicles. CAPACITY:

SPECIFICATIONS:

GENERAL-Virginia Department of Highway Road and Bridge Specifications, 1970.

DESIGN-A.A.S.H.O. Standard Specifications for Highway Bridges, 1969 and 1970—72 Interim Specifications, modified by Special Design Provisions.

WELDING-1972 Structural Welding Code of the American Welding Society.

CONTRACT SPECIAL PROVISIONS

Specifications and Contract Special Provisions re-ferred to above are necessary to make these plans complete.

CITY OF RICHMOND DATUM:

The normal temperature referred to on the plan is 60° F. The temperature range for movement is 0° F, to 120° F.

DIMENSIONS: All dimensions are measured horizontally and vertically unless otherwise noted.

EXCAVATION: Excavation below subgrade and cut slope template shall be classified as Structure Excavation.

All excavation above these limits shall be classified as Regular Excavation and is not in-cluded in the Structural Quantities.

Prov. concerning preparation of found. for ftgs.

FOUNDATIONS: Footings shall rest on firm material. Found. mat'l. shall be kept dry & special attn. is called to Sec. 401.05 of the Gen. Specs. & to the Contract Special

CONCRETE NOTES:

Concrete in superstructure shall be Class A4.
All other concrete shall be Class A3. All exposed edges and corners shall have a 3" chamfer or fillet unless otherwise noted. Care in the method of vibration, the use of low-slump concrete, and/or other means shall be employed to prevent downgrade movement of newly placed slab concrete. (When gradient is over 2%).

Finishing concrete surfaces: See the Standard Architectural Detail Sheets and the Contract Special Provisions for types and details.

All reinforcing steel shall be intermediate grade. All reinforcing bar dimensions on the detailed drawings are to centers of bars unless otherwise noted. Clear distance between reinforcing steel and face of concrete shall be as noted on the plans. All bar laps shall be 30 diameters of the smaller diameter bar unless otherwise noted.

STEEL NOTES: Structural steel shall conform to A.S.T.M. Specification A36 except as noted.

All Field connections shall be made with high strength bolts. High strength bolts shall be diameter unless otherwise noted and shall conform to A.S.T.M. Specification A-325.

C-11. Copper Weld Rod, South side of Maplewood Ave. at Belmont Ave. Elev. 215.37.

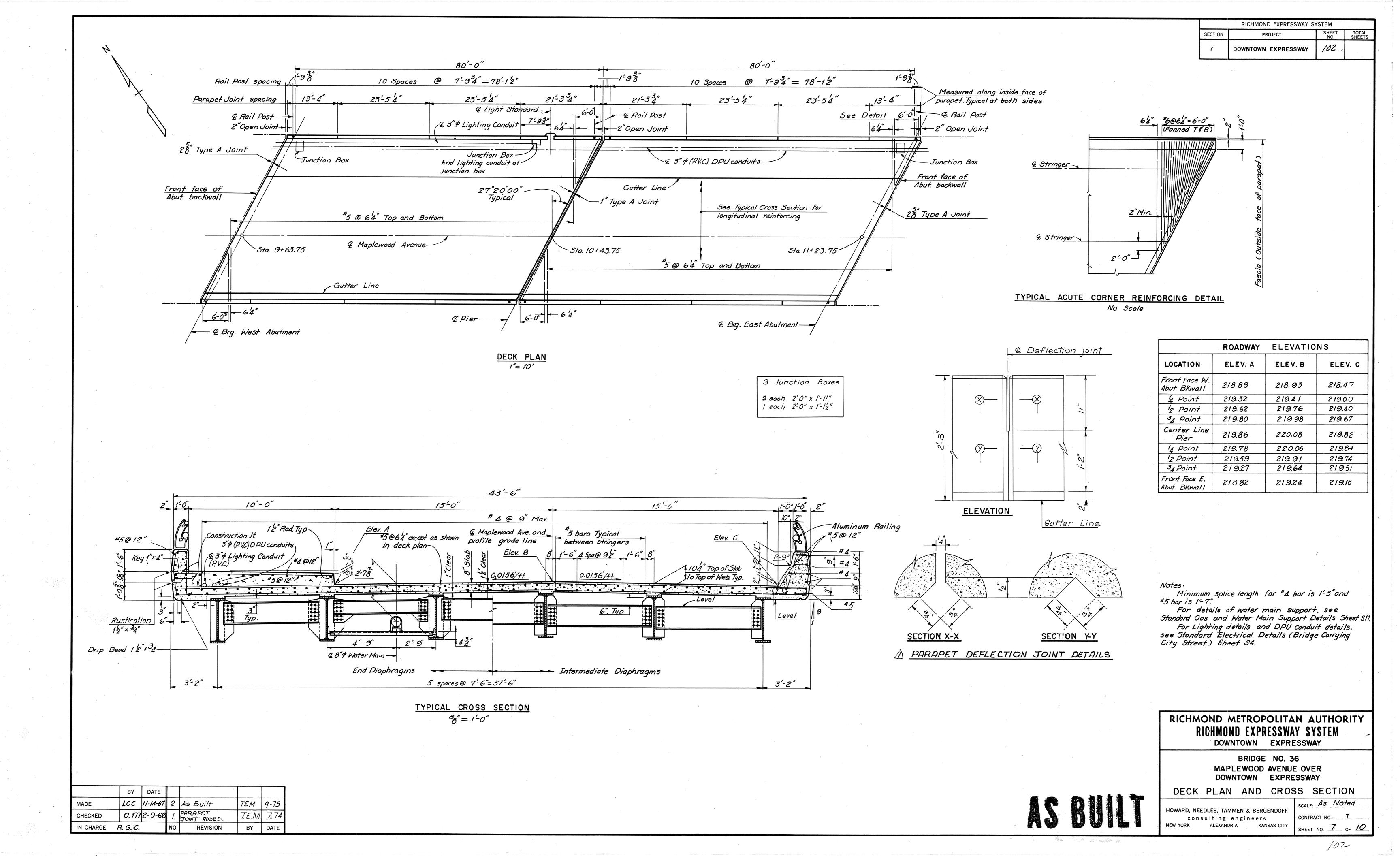
Indicates 22" & cased hole boring. Indicates 4" & cased hole boring.

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM DOWNTOWN EXPRESSWAY

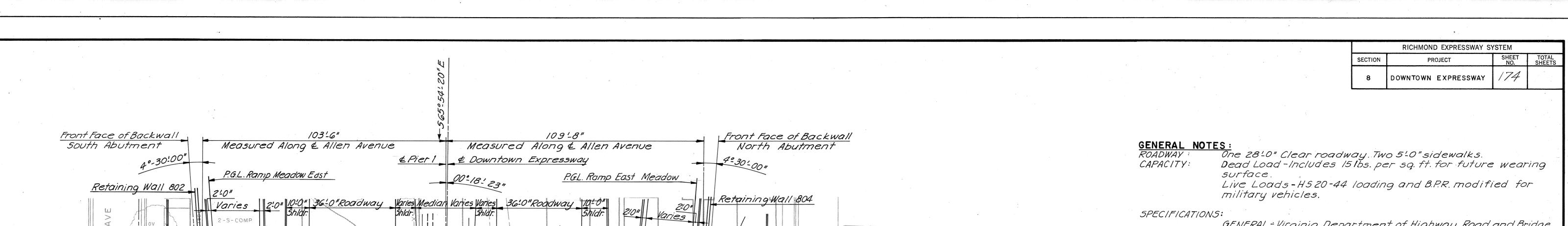
BRIDGE NO. 36 MAPLEWOOD AVENUE OVER DOWNTOWN EXPRESSWAY GENERAL PLAN AND ELEVATION

consulting engineers

SCALE: I"= 20'UNLESS NOTE HOWARD, NEEDLES, TAMMEN & BERGENDOFF CONTRACT NO.: _____7 NEW YORK ALEXANDRIA KANSAS CITY



(South Allen Street Over Downtown Expressway {Rte. 195})



Horiz Clear £ 6 - 4" o Telephone Conduits Sta. 11+82.95 Elev 212.47 £ 8" b Water Main Elev. 209.45 VOINT Sta. 11+62.89 Elev. 212.59 8-22 A Sta. 14+34.88 - Allen Avenue Sta. 133+15.88 Reloc. Parkwood Ave. 25"Exp. Jt. 2 = "Exp. Ut. | | 5 -10" & Light Post Sta. 11+55.59-Allen Avenue Sta 12+8645-Allen Avenue Sto. 333+17.37-Grayland Ave. Sta. 233+16.67 Downtown Expuy. N 23°-47-17'E 1 Sta. 11+91147-Atten Avenue Sta. 133+17.93-Reloc. Parkwood Ave. - <u>5ta 13+8751-Allen Ave.</u> - 5ta 14+79,82-Ramp East & and P.G.L. Allen Avenue Sta. 18+14,00 -Ramp Meadow East |" Fixed Jt. Elev. 211.68 5tq.14+16.18 Elev. 209.03 ---| | | | Meadow | | | | | M& 2-6" & Electrical Conduits (VEPCO) £8% Gas Main Point of Min. Vert Z"Exp. Ut. £3" + Electrica Wood Pole 2 Gonduit, é 6 a Gas Main By Others 1:3" & Brg £4-3"

Electrical Conduits 000 10:0" PGL 6-04 Horiz.Clr 24" & Storm Sewer ■ Indicates Junction Box

PLAN

Sta. 10+63.48 Sta. 16+50.00

RAMP EAST MEADOW

Spline Grade

-0.53% -2.50% N

AS BUILT

REVISION

Rev. No. & Quantity DGT 11-12-74

BY DATE

P.V.I. Sta.13+00

ALLEN AVENUE

PROFILE DATA

Elev. 211.85

V.C. = 150'

DATE

W.E.O. 10-67

W.E.O.

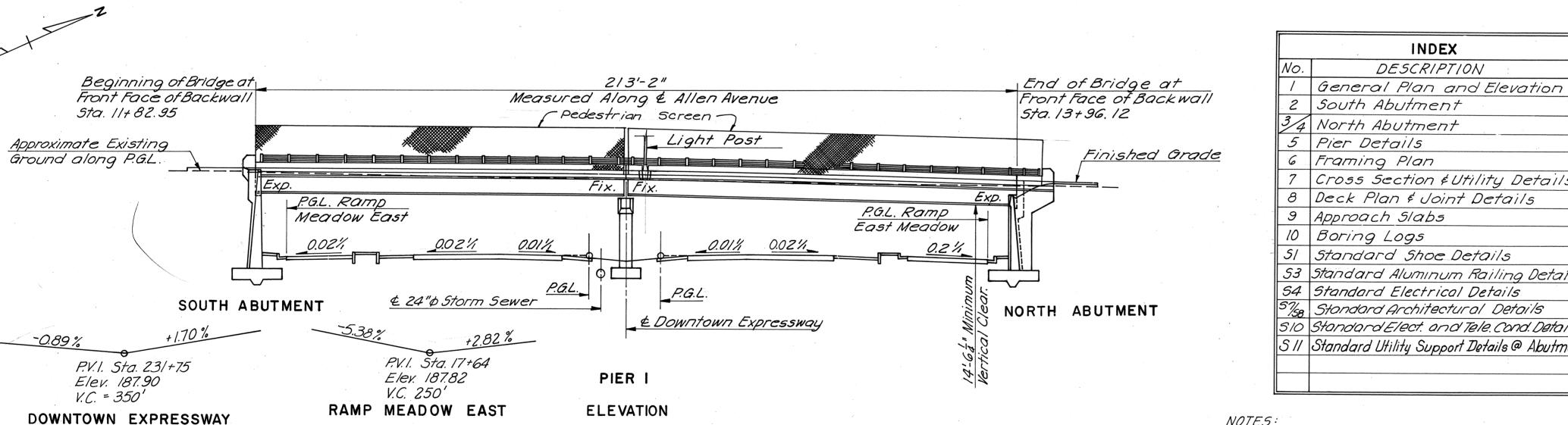
BY

MADE

CHECKED

IN CHARGE

Elev. 196.07 Elev. 194.88 +6.62%



Cross Section & Utility Details 53 Standard Aluminum Railing Details. 510 Standard Elect. and Tele. Cond. Details S // Standard Utility Support Details @ Abutment

Top of Pavement Elevations at ends of Deck along P.G.L. are given on Plan, remaining Pavement Elevation's are given on Sheet 8. ● Indicates 21 p Cased Hole Boring.

GENERAL - Virginia Department of Highway Road and Bridge

Specifications 1970 DESIGN-A.A.S.H.O. Standard Specifications for Highway Bridges,

1961 modified by Special Design Provisions. WELDING-1969 Standard Specifications for welded Highway and Railway Bridges of the American Welding Society.

CONTRACT SPECIAL PROVISIONS

Specifications and Contract Special Provisions referred to above are necessary to make these plans complete.

DATUM: CITY OF RICHMOND

TEMPERATURE: The normal temperature referred to on the plan is 60°F. The temperature range for movement is 0°F, to 120°F.

DIMENSIONS: All dimensions are measured horizontally and vertically

unless otherwise noted.

of foundations for footings.

Excavation below subgrade and cut slope template shall be classified as Structure Excavation. All excavation above these limits shall be classified as Regular Excava-

tion and is not included in the Structural Quantities.

FOUNDATIONS: Footings shall rest on firm material. Foundation material shall be kept dry and special attention is called to Section 401.05 of the General Specifications, and to the Contract Special Provisions, concerning preparation

CONCRETE NOTES

Concrete in superstructure shall be Class A4. All other concrete shall be Class A3. All exposed edges and corners shall have a 3" chamfer or fillet unless otherwise noted. Care in the method of vibration, the use of low-slump concrete and/or other means shall be employed to prevent downgrade movement of newly placed slab concrete (When gradient is over 2%) 'Finishing concrete surfaces : See the Standard Architectural Detail Sheets and the Contract Special Provisions for tupes and details.

All reinforcing steel snall conform to ASTM A615 Grade 40. All reinforcing bar dimensions on the detailed drawings are to centers of bars unless otherwise noted. Clear distance between reinforcing steel and face of concrete shall be as noted on the plans. All bar laps shall be 30 diameters of the smaller diameter bar unless otherwise noted.

STEEL NOTES: Structural steel shall conform to A.S.T.M. Specification A36 except as noted.

All field connections shall be made with high strength bolts. High strength bolts shall be &"diameter unless otherwise noted and shall conform to ASTM. Specification

BENCH MARK: C-28. Monument located in walk S.W. Corner Idlewood and South Allen . Elev. 217.02.

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

DOWNTOWN EXPRESSWAY

STRUCTURE B 46 ALLEN AVENUE OVER DOWNTOWN EXPRESSWAY

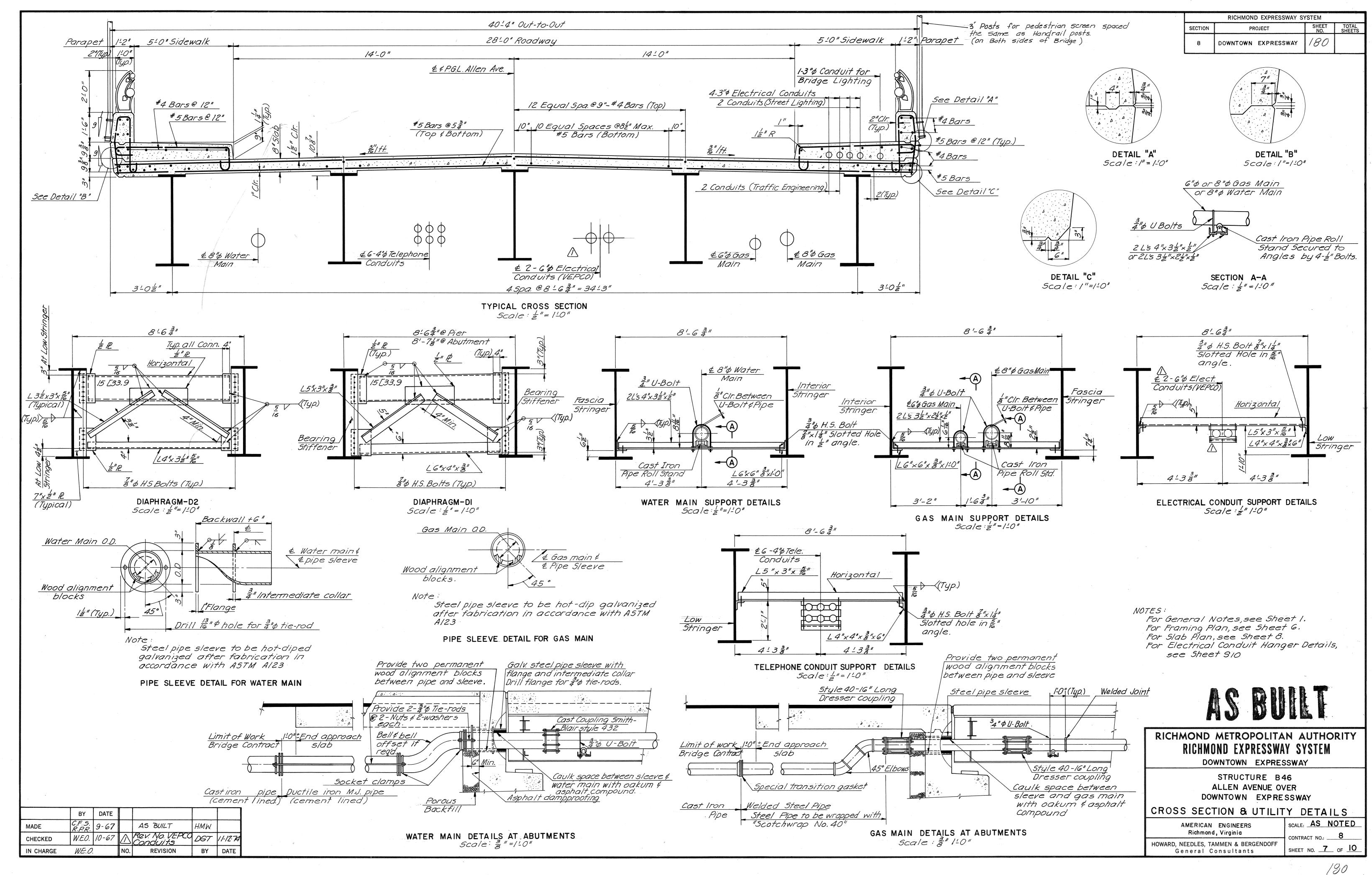
GENERAL PLAN AND ELEVATION

AMERICAN ENGINEERS Richmond, Virginia HOWARD, NEEDLES, TAMMEN & BERGENDOFF

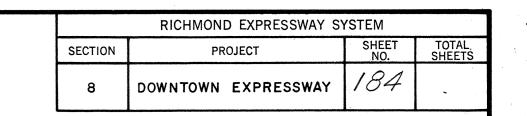
General Consultants

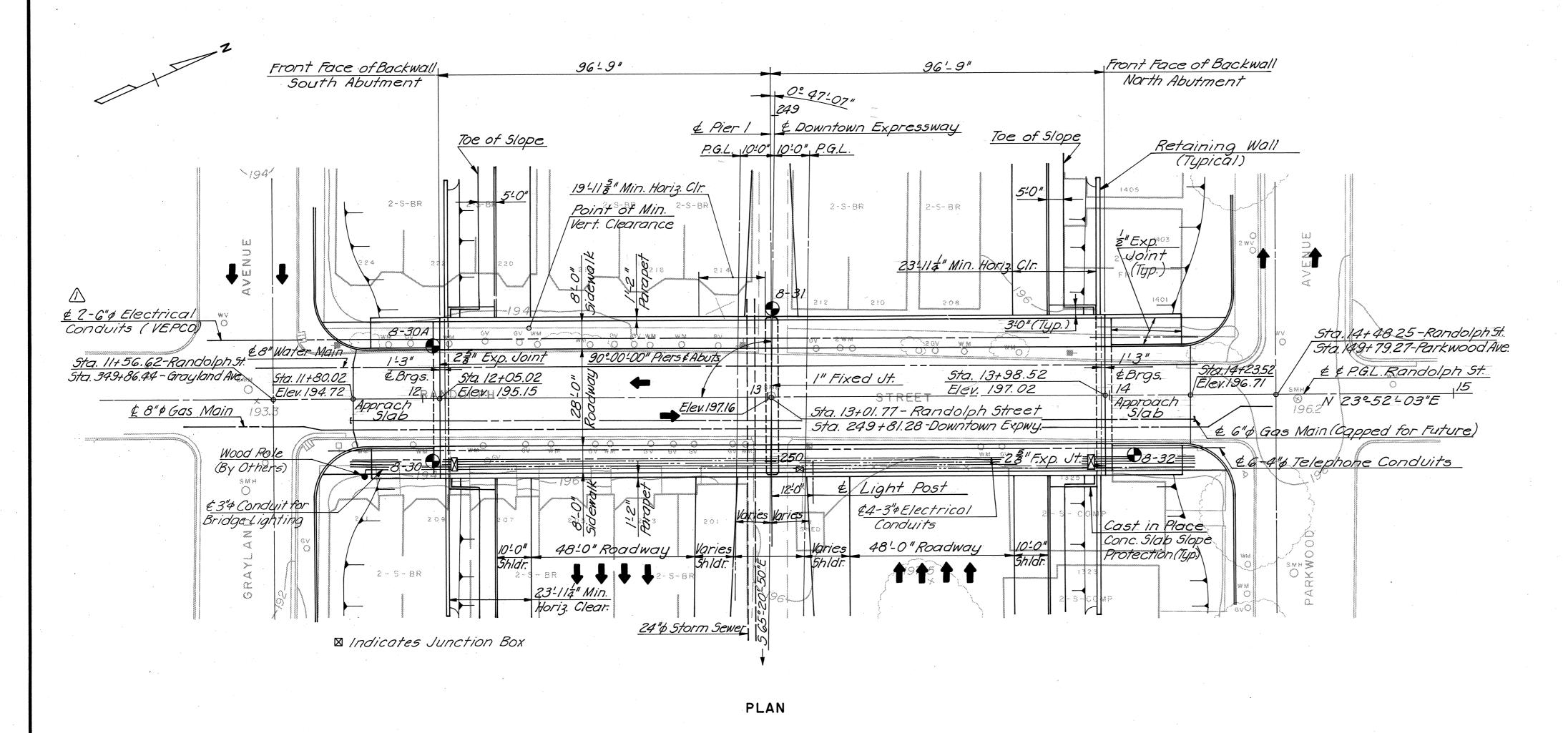
SCALE: | | = 20' ONTRACT NO.: SHEET NO. 1 OF 10

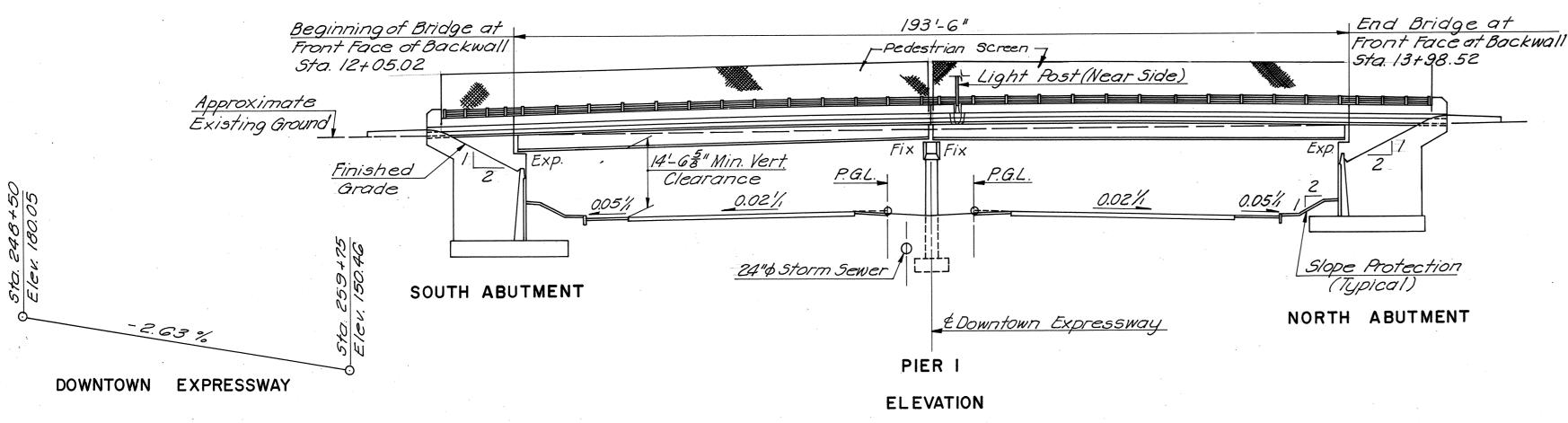
ESTIMATED QUANTITIES CONCRETE CONCRETE
CLASS CLASS
A4 A 3 REINFORCING STRUCTURAL BRIDGE STEEL - A3G RAILING PEDESTRIAN ASPHALT GINCH DAMP - PIPE SCREEN PROOFING UNDERDRAIN GAS MAIN 8" \$ POROUS BACKFILL GAS MAIN 6" \$ WATER CONDUIT
MAIN 6" \$
8" \$ VEPCO T CONDUIT 3" \$\psi\$ \textit{Z"\$\phi\$ \textit{PVC} \textit{METAL} \textit{TELEPHONE CONDUIT} \textit{CONDUIT} STRUCTURE STONE EXCAVATION BEDDING C.Y. C.Y.L.F. LBS. LB5. S.Y. C.Y. L.F. L.F. TON L.F. 3*50,43*3 Superstructure 431 339.7 68,968 259 1,278 1,275 441 262 254 514 4 South Abutment 165 13, 503 117 48 195.4 68 Pier / 15,928 84 64.2 7.92 North Abutment /68 191.9 14, 323 36 //3 61 21.98 Approach Slabs 14,887 65.6 514 1,278 1,275 Total 417 441 254 339.7 517.1. *350,43*3 *230* 84 262 259 127,609 431 29.90



(South Randolph Street Over Downtown Expressway {Rte. 195})







5ta. 12+05 Elev. 195.15

MADE

CHECKED

IN CHARGE

1.53%

AS BUILT

Rev. No. \$ Quantin VEPCO Condui:

REVISION

DGT

BY

11-12-74

DATE

P.V.I. Sta. 13+25

Elev. 198.15

RANDOLPH STREET

PROFILE DATA

NO.

V.C. = 160 '

BY DATE

W.D.U. 8-67

W.E.O. 11-67

W.E.O.

ight Post (Near Side)	INDEX
	No. DESCRIPTION
	I General Plan and Elevation
Exp	2/3 South Abutment
P.G.L.	4 5 North Abutment
0.021/1 0.051/1	6 Retaining Wall Details-North & South Abut
	7 Pier Details
	8 Framing Plan
Slope Protection	9 Cross Section and Utility Details
(Typical)	10 Deck Plan and Joint Details
NODIL A DUITMENT	11 Approach Slab & Slope Protection Details.
ntown Expressway NORTH ABUTMENT	12 Boring Logs
	51 Standard Shoe Details
	53 Standard Aluminum Railing Details.
	54 Standard Electrical Details
	Sys Standard Architectural Details
NOTES:	510 Standard Elect and Tele. Cond. Details
Top of Pavement Elevations at ends of deck along	SII Standard Utility Support details @ Abutment
P.G.L. are given on plan; Remaining pavement elevations are given on Sheet 10.	

GENERAL NOTES:

ROADWAY CAPACITY :

One 28'-0" Clear roadway. Two 8'-0" sidewalks. Dead Load - Includes 15/bs. per sq. ft. for future wearing

surface.

Live Loads - HS20-44 loading and B.P.R. modified for

military vehicles.

SPECIFICATIONS

GENERAL - Virginia Department of Highway Road and Bridge Specifications 1970

DESIGN -AASHO. Standard Specifications for Highway Bridges, 1961 modified by Special Design Provisions. WELDING - 1969 Standard Specifications for welded Highway and Railway Bridges of the American Welding Society.

CONTRACT SPECIAL PROVISIONS .

Specifications and Contract Special Provisions referred to above are necessary to make these plans complete.

CITY OF RICHMOND DATUM:

TEMPERATURE: The normal temperature referred to on the plan is 60°F. The temperature range for movement is 0° F, to 120° F.

DIMENSIONS: All dimensions are measured horizontally and vertically unless otherwise noted.

Excavation below subgrade and cut slope template EXCAVATION : shall be classified as Structure Excavation. All excavation above these limits shall be classified as Regular Excavation and is not included in the Structural Quantities.

FOUNDATIONS: Footings shall rest on firm material. Foundation material shall be kept dry and special attention is called to section 401.05 of the General Specifications, and to the Contract Special Provisions, concerning preparation of foundations for footings.

CONCRETE NOTES

Concrete in superstructure shall be Class A4. All other concrete shall be Class A3. All exposed edges and corners shall have a 3"chamfer or fillet unless otherwise noted. Care in the method of vibration, the use of low slump concrete and/or other means shall be employed to prevent downgrade movement of newly placed slab concrete (When gradient is over 2%)

Finishing concrete surfaces : See the Standard Architect - ural Detail Sheets and the Contract Special Provisions for types and details.

All reinforcing steel shall conform to ASTM A615 Grade 40. All reinforcing bar dimensions on the detailed drawings are to centers of bars unless otherwise noted. Clear distance between reinforcing steel and face of concrete shall be as noted on the plans. All bar laps shall be 30 diameters of the smaller diameter bar unless otherwise noted.

STEEL NOTES: Structural steel shall conform to A.S.T.M. Specification A36 except as noted.

All field connections shall be made with high strength bolts. High Strength bolts shall be & diameter unless otherwise noted and shall conform to AST.M. Specification A-325.

BENCH MARK: C-32. Monument located in walk 5.W. Corner Idlewood and S. Randolph Sts. Elev. 192.25.

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

DOWNTOWN EXPRESSWAY

STRUCTURE B 47 RANDOLPH STREET OVER DOWNTOWN EXPRESSWAY

GENERAL PLAN AND ELEVATION

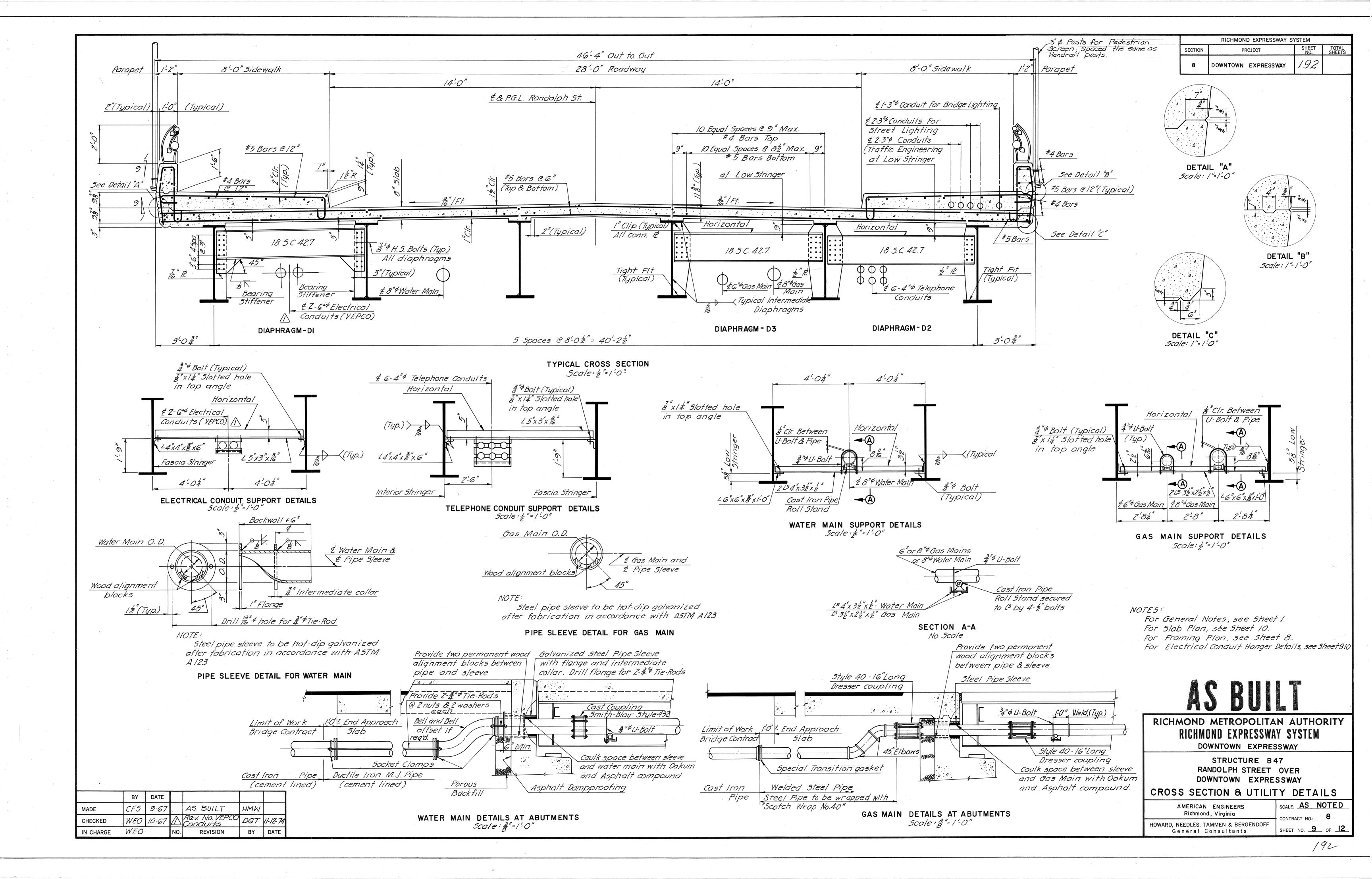
SCALE: | | = 20' AMERICAN ENGINEERS Richmond, Virginia CONTRACT NO.: HOWARD, NEEDLES, TAMMEN & BERGENDOFF

General Consultants

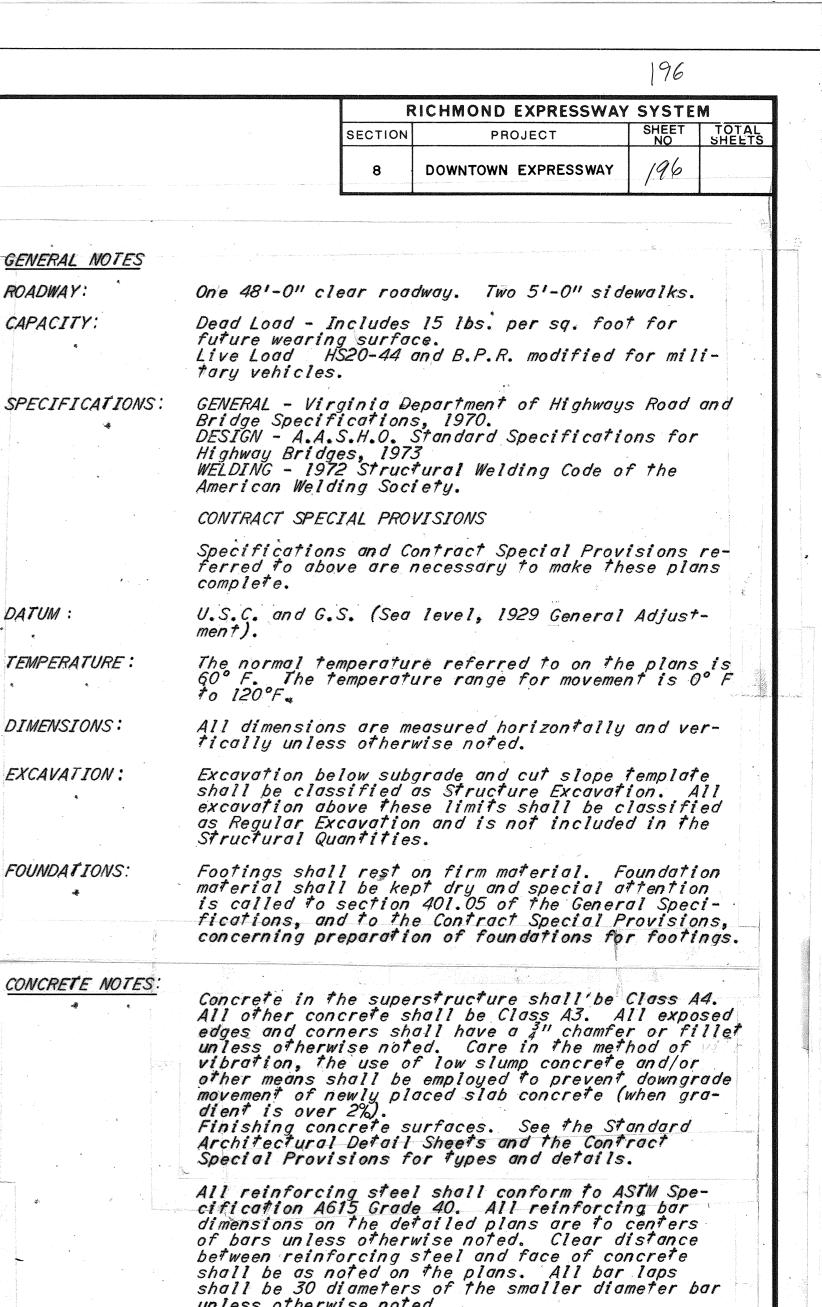
	ESTIMATED QUANTITIES																		
*	STRUCTURE EXCAVATION	CLASS A4	CLASS A3	REINFORCING STEE L	STEEL A3G	ALUMINUM BRIDGE RAILING	CONC. SLAB SLOPE PROTECTION	DAMP - PROOFING	UNUERURAIN	POROUS BACKFILL C.Y.	STONE BEDDING TON	GAS MAIN 8" ¢	GAS MAIN 6" \$	WATER MAIN 8" \$ L.F	CONDUIT G" \$ VEPCO L.F	CONDUIT 4" \$ TELE. L.F.	PVC CONDUIT 3"\$ F	METAL CONDUIT Z''ø	PEDESTRIAN SCREEN L.F.
	C.Y.	C.Y.	C.Y.	LBS.	LBS.	L.F.	<i>5. Y</i> .	5. Y.	L.F.	C.7.	7 O/ V	215 5	245.5	247	4831		1225	1	462
Superstructure	*	365.56	-	70, 753	334,842.8	462						245.5	240.0	241	40511	1110	1665	+	762
South Abutment	1304.9		5/6.27	36,863		e e e	82	225	123	87.5	52.47						<u> </u>		6
Pier 1	3/5.4		90.36	18,495					i i		23.06							ļ	
North Abutment	950.7	,	389.44	28, 117		*	82	216	124	67.5	38:57	1				,	<u> </u>		6.
Approach Slabs	 	-	78.3	16,951							-					·			
Total	257/	365.56	1074.37	171,179	334,842.8	462	164	441	247	155	114.10	245.5	245.5	247	4831	1170	12.25	4	474

1 Indicates 2 " Cased Hole Boring.

SHEET NO. 1 OF 12



(South Harrison Street Over Downtown Expressway {Rte. 195})



STEEL NOTES:

BENCH MARK:

NOTES

Structural Steel shall conform to ASTM Specification A36 except as noted. All field connections shall be made with high strength bolts. High strength bolts shall be i' diameter unless otherwise noted and shall conform to ASTM Specification A325.

All shop welded web splices, flange splices and web to flange welds shall be made by the submerged arc process.

C-34 Monument in walk on the East side of Harrison Street between Parkwood Avenue and Cary Street. Elev. 179.381.

Top of Pavement Elevations at ends of deck along P.G.L. are given on the plan, Remaining pavement elevations are given on Sheet || . The cost of furnishing and installing 2"0 and 3"0 galvanized steel conduit shall be included in the bid price for the item "Metal Conduit".

Indicates 2110 cased hole boring.

unless otherwise noted.

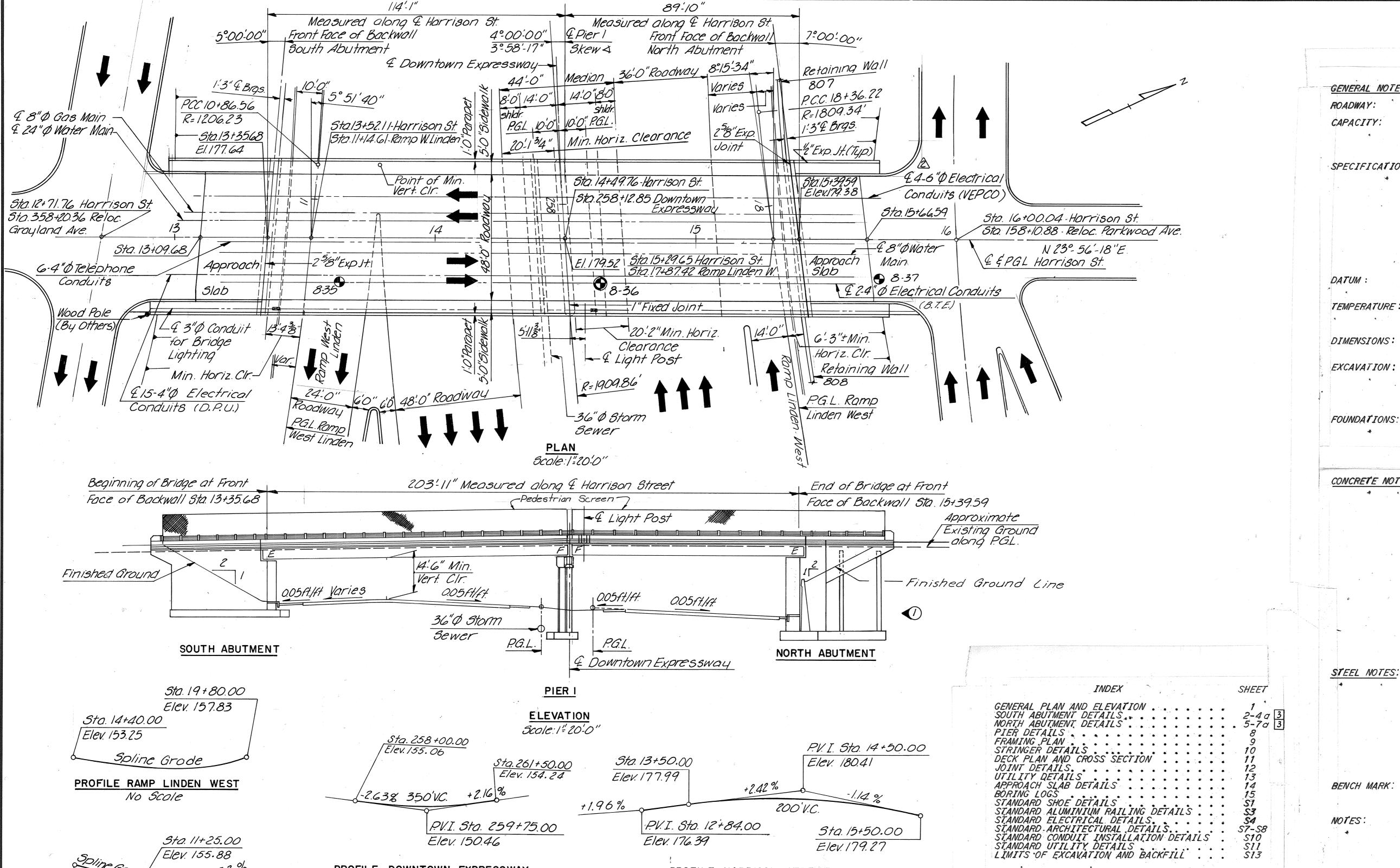
RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

DOWNTOWN EXPRESSWAY

STRUCTURE B48 HARRISON STREET OVER DOWNTOWN EXPRESSWAY

GENERAL PLAN AND ELEVATION SCALE AS Showr DATE NOV. 29,1973 SHEET / OF /5

HOWARD, NEEDLES, TAMMEN & BERGENDOFF CONSULTING ENGINEERS HNTB Alexandria, Virginia



PROFILE RAMP LINDEN WEST No Scole

Spline Grode

Sta. 11+25.00 Elev. 155.88 Elev. 163.51

Elev. 153.25

Sta. 14+50.00 Elev. 158.90 P.V.I. Sta. 13+00.00 Elev. 150.60

PROFILE RAMP WEST LINDEN No Scale

		TOTAL MATERIAL SERVICE				
				AS BUILT	HMW	
DESIGNED	P.R.M.C.	10/73	3	Sheets 4a & 7a added. Quantities revised	R.K.L.	12-9-74
DRAWN	R.P.C.	11/73	2	Rev. No. § Quantity Vepco Conduits		11-12-74
CHECKED	D.E.R.	11/73	/	No. Abut. FRe. Stl. Qty.	PRMC	12-12-73
IN CHA	RGE Z.	J.H.	NO.	REVISION	BY	DATE

PROFILE DOWNTOWN EXPRESSWAY

No Scale

Scale: 1 20'0" Sta. 258 +00.00 Elev. 155.06 Elev. 154.24 -2.63% 350'V.C. +1.96% PVI. Sto. 259+75.00 Elev. 150.46

Sta. 13+50.00 Elev. 177.99 +2.42% P.V.I. Sto. 12+84.00 Elev. 176.39

470, 378

116.12

P.V. I. Sto. 14+50.00 Elev. 180.41 200'V.C. Sta. 15+50.00

Elev. 179.27

541

PROFILE HARRISON STREET No Scale

259.1 3

ESTIMATE OF QUANTITIES STRUCTURE CONCRETE STRUCTURAL REINFORCING POROUS ASPHALT UNDERDRAIN WATERMAIN WATERMAIN GAS MAIN CONDUITS CONDUIT CONDUIT STEEL 3 PVC PEDESTRIAN EXCAVATION CLASS A4 CLASS A3 APPR. SLABS C.Y. C.Y. C.Y. STEEL CONDUIT 3" Ø L.É. STEEL BACKFILL BR RAILING DAMPPROOF- 6" Ø PIPE 24" Ø PIPE 8" Ø PIPE (VEPCO)6"Ø 4"Ø TELE-8" Ø PIPE 4" Ø DPU PILES . SCREEN (2 RAIL) L.F. ING S.Y. PHONE 8 BTE 10 BP 42 LBS. LBS. C.Y. L.F. L.F. L.F L.F. L.F. L.F. 437.12 SUPERSTRUCTURE 470, 378 92,871 408 255 1,024 B 1556 3400 259.5 249 249 408 1250 3 5/2.94 3 127.1 3 SOUTH ABUTMENT 51,897 3 280 🗵 75 1443 3 478.4 2100 3 NORTH ABUTMENT 3/91,112 132 626.94 3 3 5/2.3 58 300 63 64 PIER I 606 282.20 31,883 APPROACH SLABS 40,377 116.12 3956 437.12 1422.08 TOTAL 3 308,140 259.5

249

2073

255

1,024 /2 1556

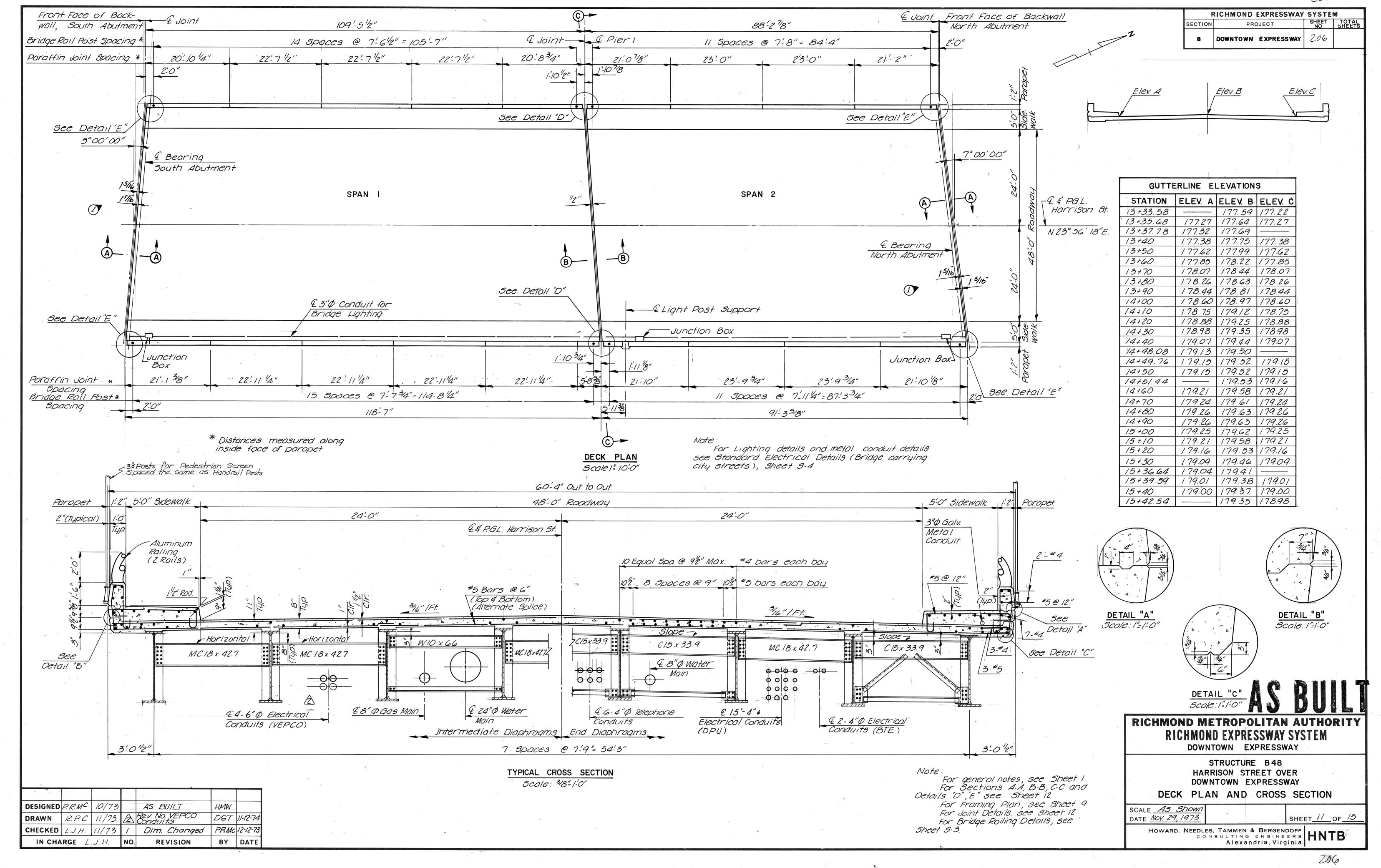
3400

3 990.7

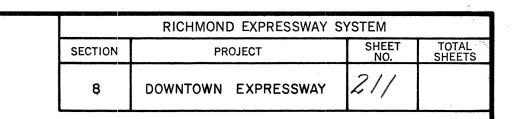
249

553





(South Cherry Street Over Downtown Expressway {Rte. 195})



GENERAL NOTES:

ROADWAY:

One 30'-0" Clear roadway. Two 8'-0" sidewalks.

Dead Load - Includes 15 lbs. per sq. foot for future wearing

Live Load - H520-44 loading and B.P.R. modified for

GENERAL - Virginia Department of Highway Road and Bridge

DESIGN - A.A.S.H.O. Standard Specifications for Highway Bridges, 1961 modified by Special Design Provisions. WELDING-1969 Standard Specifications for welded Highway and Railway Bridge of the American Welding Society.

CONTRACT SPECIAL PROVISIONS:

Specifications and Contract Special Provisions referred to above are necessary to make these plans complete.

CITY OF RICHMOND

TEMPERATURE: The normal temperature referred to on the plan is 60°F. The temperature range for movement is 0°F, to 120°F.

All dimensions are measured horizontally and vertically

EXCAVATION: Excavation below subgrade and cut slope template shall be classified as Structure Excavation. All excavation

and is not included in the Structural Quantities.

Footings shall rest on firm material. Foundation material shall be kept dry and special attention is called to section 401.05 of the General Specifications, and to the Contract Special Provisions, concerning preparation of foundations

Concrete in superstructure shall be Class A4. All other concrete shall be Class A3. All exposed edges and corners shall have a 3 chamfer or fillet unless otherwise noted. Care in the method of vibration, the use of low slump concrete and/or other means shall be employed to prevent

Finishing concrete surfaces: See the Standard Architect -

All reinforcing steel shall conform to ASTM. A 615 Grade 40. All reinforcing bar dimensions on the detailed drawings are to centers of bars unless otherwise noted. Clear distance between reinforcing steel and face of concrete shall be as noted on the plans. All bar laps shall be 30 diameters of the smaller diameter bar unless otherwise

STEEL NOTES: Structural steel shall conform to AST.M. Specification A 36

All field connections shall be made with high strenth bolts. High strength bolts shall be &"diameter unless otherwise noted and shall conform to A.S.T.M. Specifications A-325.

BENCH MARK: C-36 Monument in sidewalk, N.W. Corner Idlewood and South Cherry. Elev. 173.57.

CAPACITY:

military vehicles.

SPECIFICATIONS:

Specifications 1970

DATUM :

DIMENSIONS :

unless otherwise noted.

above these limits shall be classified as Regular Excavation

FOUNDATIONS:

for footings.

CONCRETE NOTES:

downgrade movement of newly placed slab concrete. (When gradient is over 2%)

ural Detail Sheets and the Contract Special Provisions for types and details.

except as noted.



RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

DOWNTOWN EXPRESSWAY

STRUCTURE B 49 CHERRY STREET OVER DOWNTOWN EXPRESSWAY

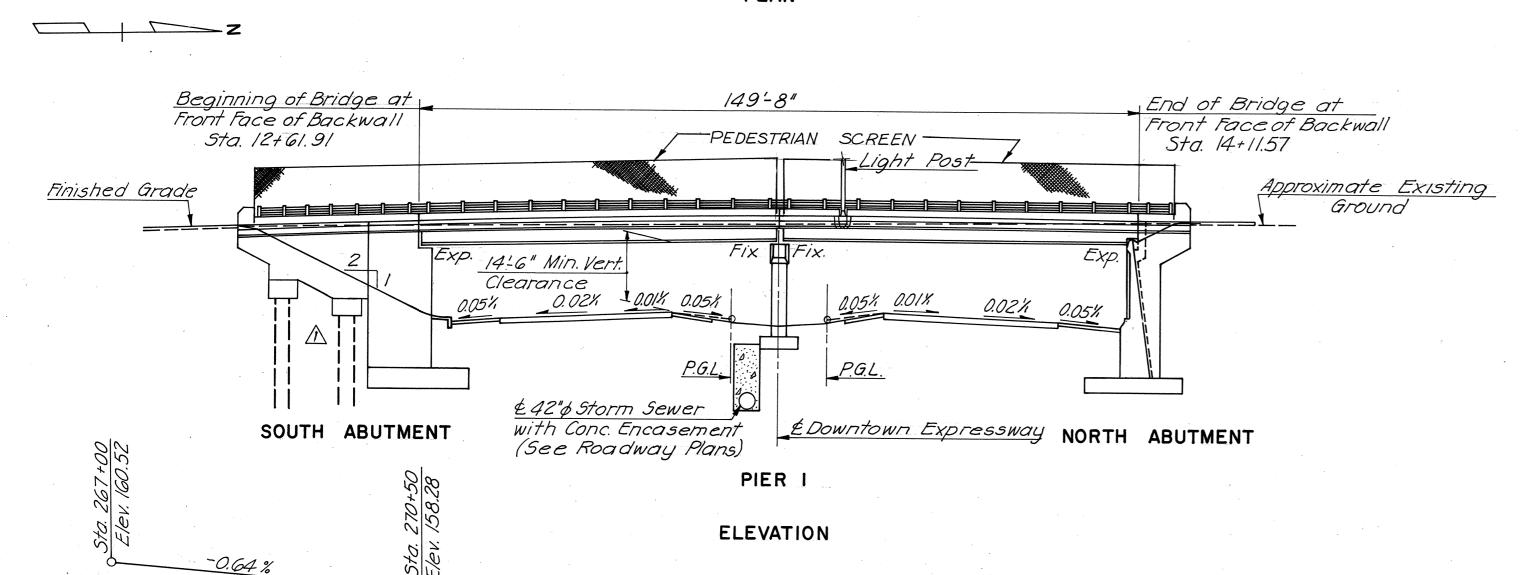
GENERAL PLAN AND ELEVATION

SCALE: | " = 20" AMERICAN ENGINEERS Richmond, Virginia

HOWARD, NEEDLES, TAMMEN & BERGENDOFF

General Consultants

Front Face of Backwall 74-10" 74-10" Front Face of Backwall South Abutment North Abutment & Downtown Expressway 00°- 45'- 33" Toe of Slope Retaining Wall 809 Roadway Median 2-S-8R £4-6" Electrical £ 6-4" Delephone Conduits (VEPCO) Conduits £8¢Water Main Clearance 90°(Tup. Abuts. & Piers) 8-44 75" Exp. Ut. Sta. 14+31.57 178.4 I" Fixed Ut Slab Approach. Elev. 178.30 + NOO -11'-27" E Sta. 12+61.91 Sta. 13+36.74 - Cherry St. £and P.G.L. Cherry 5t. Sta. 14+11.57 Slab Elev. 177.45 Sta. 269+18.08 DOWNTOWN EXDWL £8"\$ Gas Main Wood Pole 8-45 By Others \€ 3" + Conduit for # 12-4" Electrica Bridge Lighting 42" Storm Sewer Conduits Clearance 14:04" Min. Horiz. Clearance 2-S-FR ■ Indicates Junction Box PLAN



SII Standard Utility Support details @ Abutment NOTES Top of Pavement Elevations at ends of deck along P.G.L. are given on plan; Remaining pave-ment elevations are given on Sheet 9. ● Indicates 2 to Cased Hole Boring.

INDEX

DESCRIPTION

5 Typical Sections and Details - No. \$ 50. Abuts.

General Plan and Elevation

8 Cross Section and Utility Details

53 Standard Aluminum Railing Details (2-Rails)

SIO Stondard Elect. and Tele. Cond. Details

9 Deck Plan and Joint Details.

| SI | Standard Shoe Details

54 Standard Electrical Details

5/58 Standard Architectural Details

South Abutment

4 North Abutment

6 Pier Details

7 Framing Plan

11 Boring Logs

10 Approach Slabs

=	Ele V.C CHERF	? V. / . = / ?Y	-0.50 +a. 13+20 178.64 150 ' STREET DATA	2%	Sta 14+, Elev 178	
	DATE					-
J.	8-67	in day	AS BUILT	HMW		
).	12-67	\triangle	S. Abut Footing & Quantities	T.E.M.	3-75	
0.	· V	NO.	REVISION	BY	DATE	

DOWNTOWN EXPRESSWAY

BY DATE

W.D.U. 8-67

W.E.O. 12-67

W.E.O.

176.04

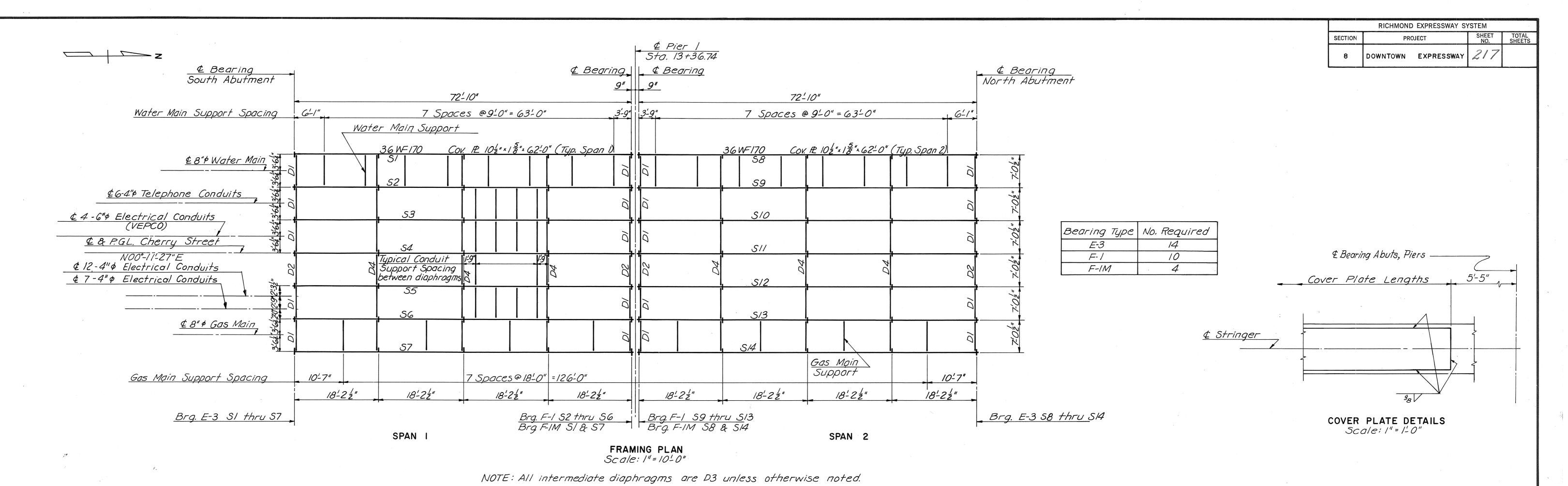
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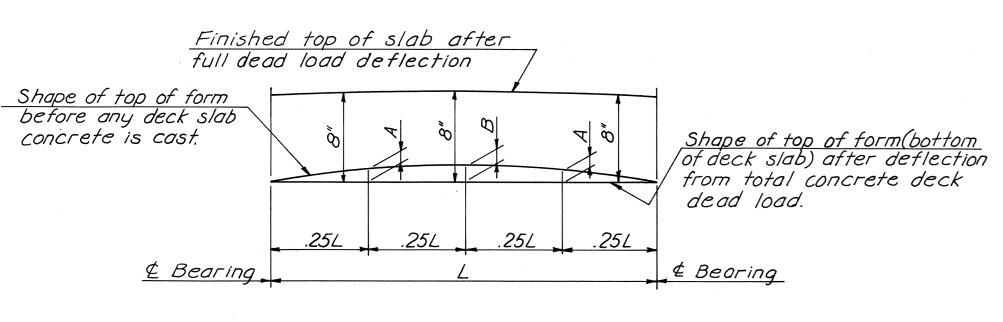
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IN CHARGE

						E	STIMATE	QUA	ANTITIES					-				
	STRUCTURE EXCAVATION C.Y.	CONCRETE CLASS A 4 C.Y.	CONCRETE CLASS A3 C.Y.	REINFORCING STEEL LBS.	STEEL	ALUMINUM BRIDGE RAILING L.F.	ASPHALT DAMP - PROOFING 5.Y.	GINCH PIPE UNDERDRAIN L.F.	POROLIS BACK FILL C.Y.	GAS MAIN 8" p L.F.	WATER MAIN 8" ¢ L.F.	CONDUIT 6" ¢ VEPCO L.F.	CONDUIT 4" ¢ TELE.	CONDUIT 4" \$ CITY		CONDUIT 3" \$ 1 F	METAL CONDUIT 2" \$	PEDESTRIA SCREEN L.F.
Superstructure		286.45		52,924	275,080.3	38/				195	197.8	788	918	2142		147.	4	381
South Abutment	1 000 2.3		457.64 △	49,884			188	47	107		1.0 #0	1	11.00	21.10	963	141-	24man .	7
Pier I	90	•	79.86	16,123						·					100			'
North Abutment	638		371.58	26,943			/57	57	83			1						
Approach Slabs			<i>74.5</i> 3	16,762														
Total	1613 🛕	286.45	983.GI <u></u>	162,636	275,080.3	381 .	345	104	. 190	195	197.8	788	918.	2142	963	147	4	395

SHEET NO. _ OF ____



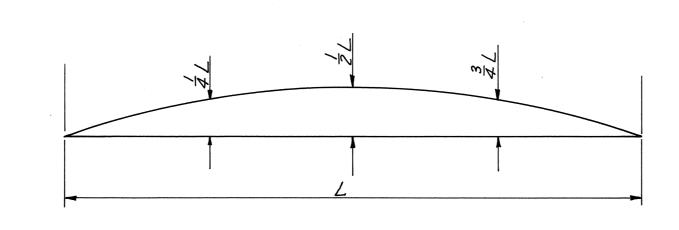


DEAD LOAD DEFLECTION DIAGRAM

Stringers	A	В
51, 57, 58, SI4	1/6"	1 ½ "
S2 thru S6, S9 thru S/3	15 " 16	/4"

NOTE TO CONTRACTOR:

The above deflections are those anticipated to occur in the stringer upon placement of the total concrete deck dead load. In practice the stringers in place are not likely to have the exact camber to compensate for these deflections during construction. The residual amounts shall be provided for by adjusting forms to vary the thickness of the concrete bolster between the bottom of the slab and the top of the stringer, without alteration of the slab thickness.



 Stringers
 $\frac{1}{4}L$, $\frac{3}{4}L$ $\frac{1}{2}L$

 S1, S6, S7
 $2\frac{1}{4}$ "
 $3\frac{3}{16}$ "

 S3 thru S5
 $2\frac{1}{4}$ "
 $3\frac{1}{8}$ "

 S8, S13, S14
 $2\frac{1}{4}$ "
 $3\frac{3}{16}$ "

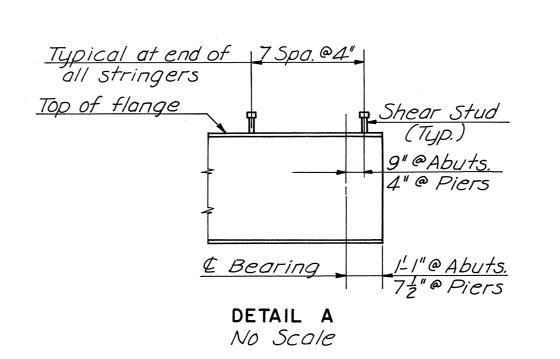
 S10 thru S12
 $2\frac{1}{4}$ "
 $3\frac{1}{8}$ "

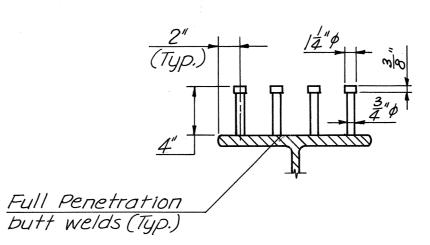
 S2 & S9
 $2\frac{3}{16}$ "
 $3\frac{1}{16}$ "

CAMBER DIAGRAM

NOTE TO FABRICATOR:

The above stringers shall be fabricated with an upward camber amounting to (see table). This will provide approximate compensation for deflection under full dead load and for conformity with finished grade. Stringers which are not required to be shop cambered shall be turned so that any mill tolerance deviation from straightness will be in the direction shown by the camber diagram above.





SHEAR STUD DETAILS

Scale: \(\frac{1}{2}'' = 1' - 0'' \)

SHEAR STUD NOTES:

Capacity = 3400 lbs. per stud

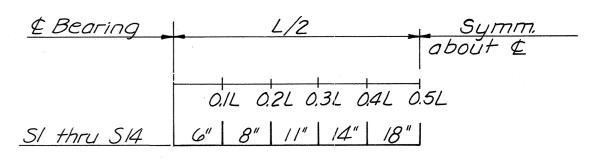
Three & diameter studs may be substituted
at the same longitudinal spacing as shown for

four & diameter studs.

Stud rows to be placed perpendicular to

£ stringer.

For end condition see Detail "A".



SHEAR STUD SPACING
No Scale

Shear Stud Spacing shown is Maximum

NOTES;
Structural steel shall conform to ASTM Specifications A36 (latest revision).
For General Notes see Sheet I.
For Superstructure Cross Section see Sheet 8.
For Bearing Shoe Dimensions see Standard Sheet SI.
For Diaphragm Details see Sheet 8.
For Utility Support Details see Sheet 8.

AS BUILT

RICHMOND METROPOLITAN AUTHORITY
RICHMOND EXPRESSWAY SYSTEM
DOWNTOWN EXPRESSWAY

STRUCTURE B 49
CHERRY STREET OVER
DOWNTOWN EXPRESSWAY

FRAMING PLAN

AMERICAN ENGINEERS
Richmond, Virginia

HOWARD, NEEDLES, TAMMEN & BERGENDOFF
General Consultants

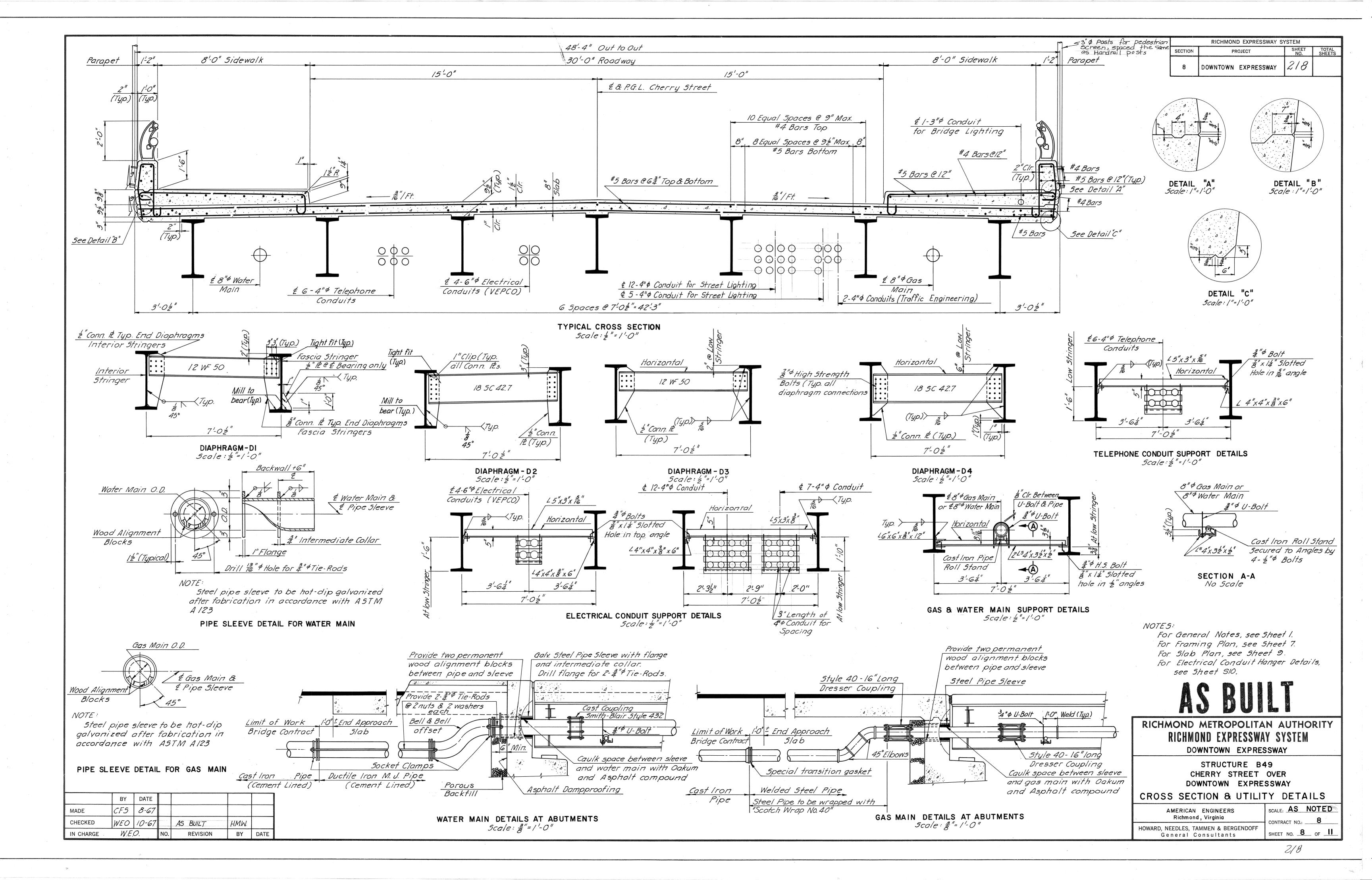
SCALE: AS NOTED
CONTRACT NO.: 8

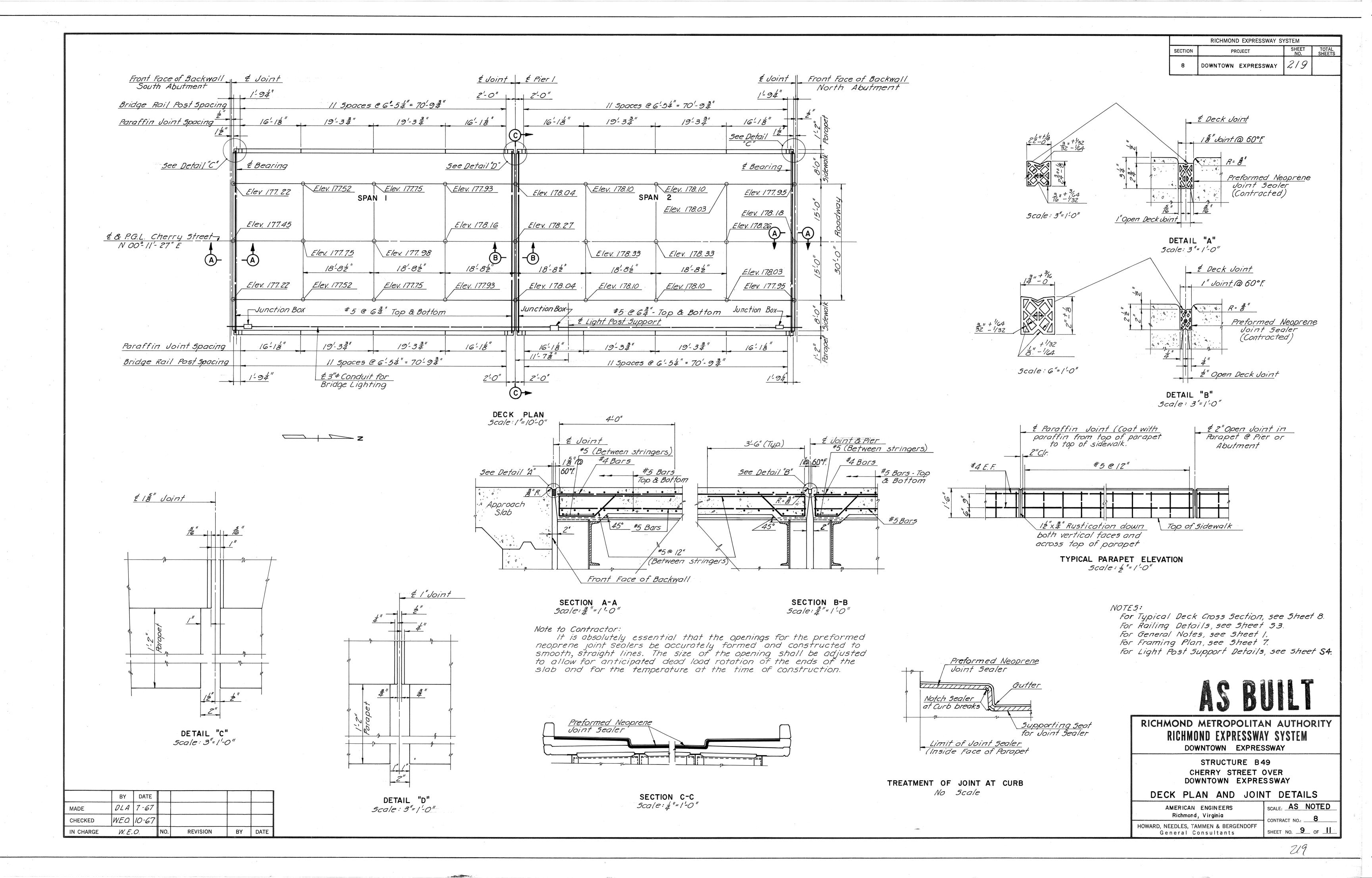
SHEET NO. 7 OF 11

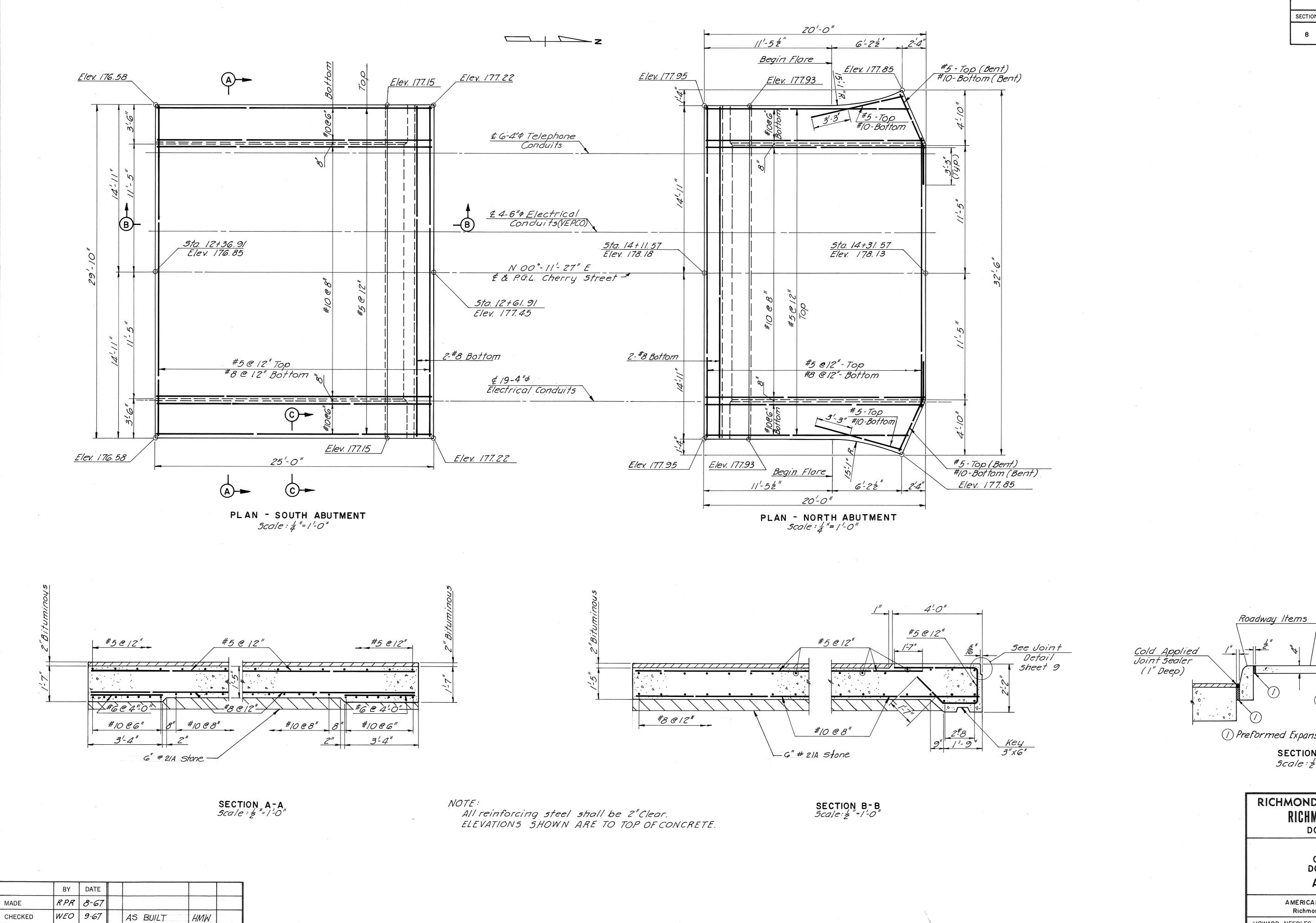
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BY DATE

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IN CHARGE

RICHMOND EXPRESSWAY SYSTEM SECTION PROJECT DOWNTOWN EXPRESSWAY

1) Preformed Expansion Joint Material

SECTION C-C

5cale: 2 = 1-0"

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM DOWNTOWN EXPRESSWAY

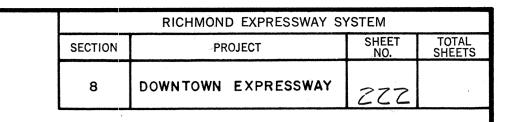
STRUCTURE B 49 CHERRY STREET OVER DOWNTOWN EXPRESSWAY APPROACH SLABS

AMERICAN ENGINEERS Richmond , Virginia

HOWARD, NEEDLES, TAMMEN & BERGENDOFF SHEET NO. 10 OF 11 General Consultants

SCALE: AS NOTED

(South Laurel Street Over Downtown Expressway {Rte. 195})



GENERAL NOTES:

ROADWAY: CAPACITY:

One 30'0" Clear roadway. Two 8'-0" sidewalks.

Dead Load - Includes 15 lbs. per sq. ft. for future wearing

Live Loads - H520-44 loading and B.P.R. modified for

military vehicles.

SPECIFICATIONS:

GENERAL-Virginia Department of Highway Road and Bridge

DESIGN -A.A.S.H.O. Standard Specifications for Highway Bridges, 1961 modified by Special Design Provisions.

Specifications and Contract Special Provisions referred to above are necessary to make these plans complete.

CITY OF RICHMOND DATUM:

TEMPERATURE : The normal temperature referred to on the plan is 60 °F. The temperature range for movement is 0°F, to 120°F.

DIMENSIONS: All dimensions are measured horizontally and vertically

EXCAVATION: Excavation below subgrade and cut slope template shall be classified as Structure Excavation All excavation above these limits shall be classified as Regular

FOUNDATIONS: Footings shall rest on firm material. Foundation material 401.05 of the General Specifications, and to the Contract

Concrete in superstructure shall be Class A4. All other concrete shall be Class A3. All exposed edges and corners shall have a 3 chamfer or fillet unless otherwise noted. Care in the method of vibration, the use of lowprevent downgrade movement of newly placed slab 'concrete. (When gradient is over 2%)

All reinforcing steel shall conform to ASTM A615 Grade 40 All reinforcing bar dimensions on the detailed drawings are to centers of bars unless otherwise noted. Clear distance between reinforcing steel and face of concrete shall be as noted on the plans. All bar laps shall be 30 diameters of the smaller diameter bar unless otherwise noted.

except as noted.

bolts. High strength bolts shall be & diameter unless

BENCH MARK: C-38. Monument located in walk N.E. Corner Cumberland and

Specifications 1970

WELDING - 1969 Standard Specifications for welded Highway and Railway Bridges of the American Welding Society.

CONTRACT SPECIAL PROVISIONS:

unless otherwise noted.

Excavation and is not included in the Structural Quantities.

shall be kept dry and special attention is called to Section Special Provisions, concerning preparation of foundations for footings.

CONCRETE NOTES

slump concrete and for other means shall be employed to Finishing concrete surfaces : See the Standard Architectural Detail Sheets and the Contract Special Provisions for types and details.

STEEL NOTES: Structural steel shall conform to A.S.T.M. Specification A36 All field connections shall be made with high strength

otherwise noted and shall conform to A.S.T.M. Specification

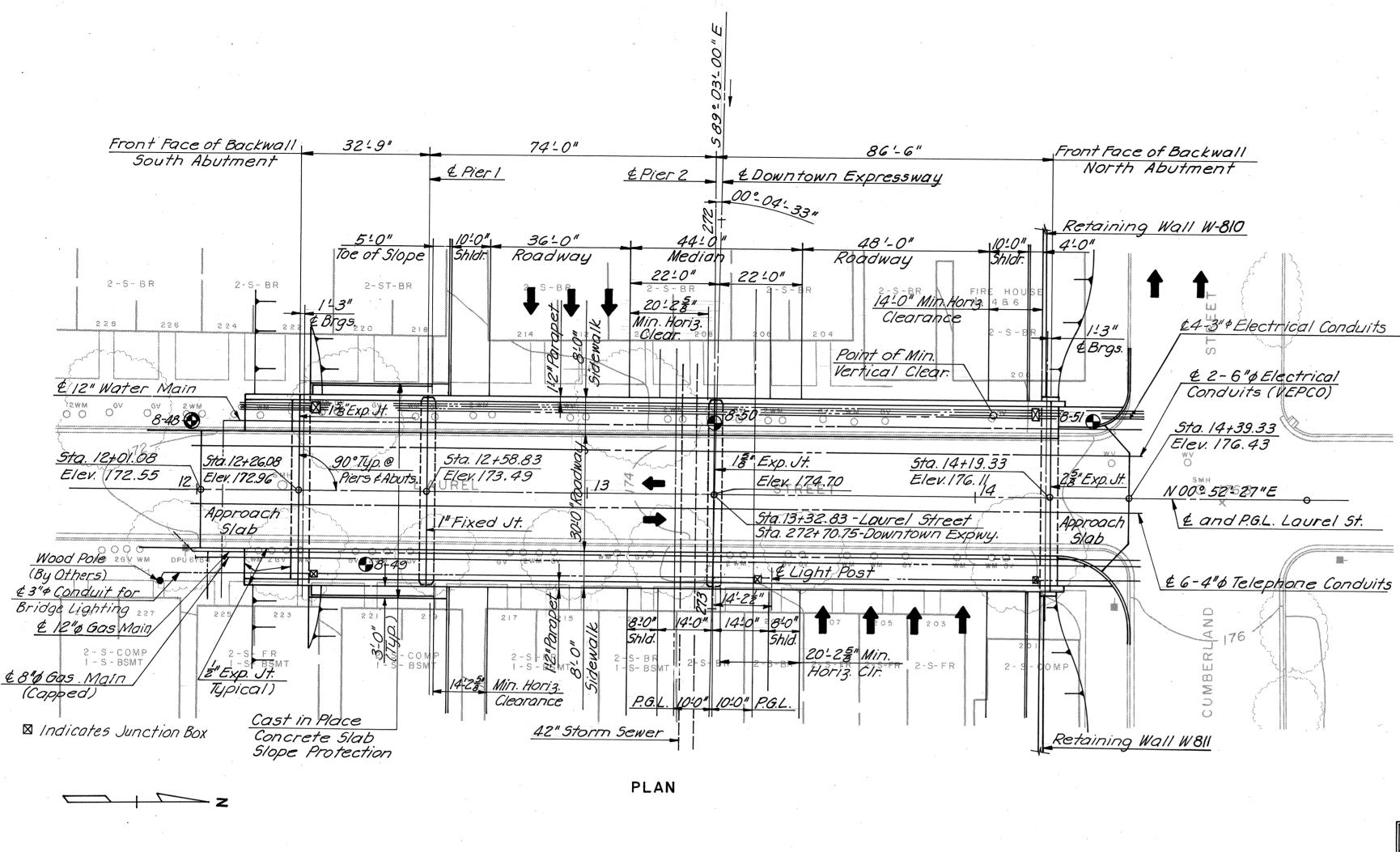
South Laurel. Elev. 176.83.

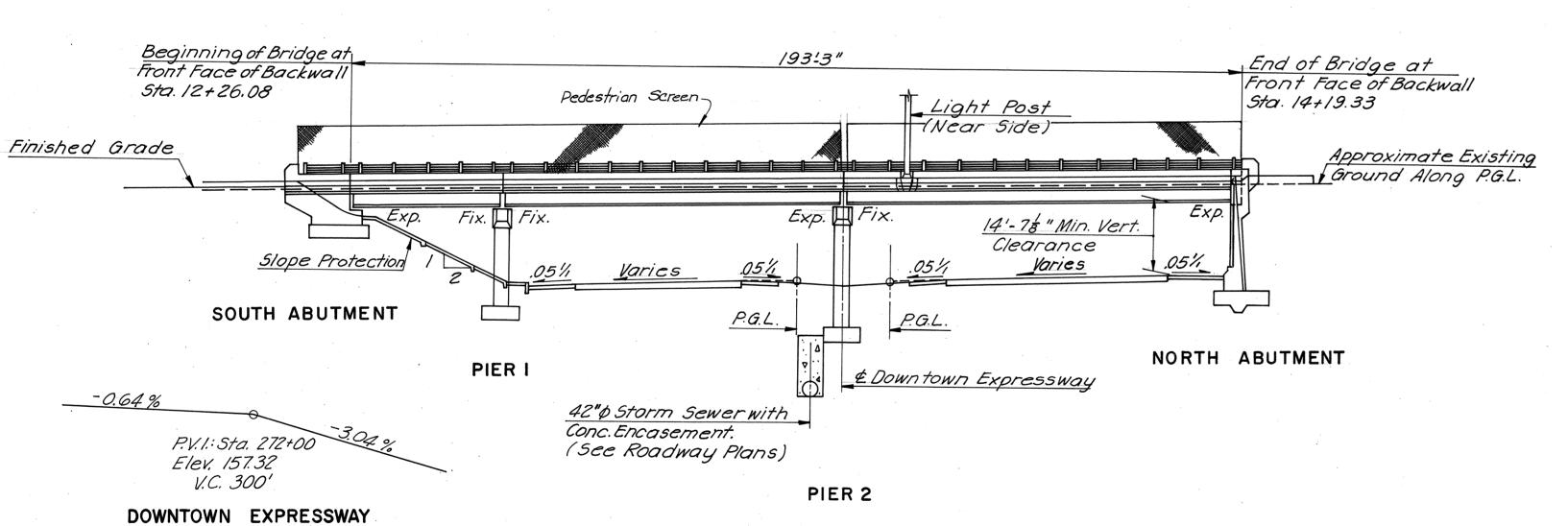
RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM DOWNTOWN EXPRESSWAY

> STRUCTURE B 50 LAUREL STREET OVER DOWNTOWN EXPRESSWAY

GENERAL PLAN AND ELEVATION SCALE: | | "= 20"

AMERICAN ENGINEERS Richmond, Virginia CONTRACT NO.: _ 8 HOWARD, NEEDLES, TAMMEN & BERGENDOFF SHEET NO. 1 OF 9 General Consultants





ELEVATION

53 Standard Aluminum Railing Details 54 Standard Electrical Details 5/58 Standard Architectural Details SIN Standard Elect. and Tele. Cond. Details SII Standard Utility Support details @ Abut. Top of Pavement Elevations at ends of Deck along P.G.L. are given on Plan, Remaining Pavement Elevations are given on Sheet 7.

226 217.3 208

481

1442 960.5

● Indicates 2 % Cased Hole Boring. 1 Indicates 4" D Cased Hole Boring

INDEX

General Plan and Elevation

DESCRIPTION

6 Cross Section & Utility Details

1 Deck Plan & Voint Details

51 Standard Shoe Details

South Abutment

3 North Abutment

4 Pier Details

5 Framing Plan

8 Approach Slabs

9 Boring Logs

ESTIMATED QUANTITIES WATER CONDUITS CONDUITS 3" PVC 2" METAL Pedestrian Screen 12" From 12" Pedestrian Screen STRUCTURE CONCRETE CONCRETE REINFORCING STRUCTURAL EXCAVATION CLASS CLASS STEEL STEEL-A3G E ASPHALT GINCH DAMP- PIPE PROOFING UNDERDRAIN ALUMINUM POROUS CONC. SLAB GAS MAIN GAS MAIN
BACKELLI SLOPE 12" 6 8" 6 STEEL-A36 BRIDGE RAILING BACKFILL 12"\$ 8"4 PROTECTION S.Y. L.F. C.Y. L.F. L.F. C.Y.LBS. LBS. L.F. 420 1442 960.5 382,078.4 217.3 208 481 408 Superstructure 364.21 68,195 226 134 4.873 232.6 43 57 14 South Abutment 88.24 92.65 16,300 241 272 97.54 19,411 242.29 12,341 268.12 55 72 119

112

86

232.6

PROFILE DATA Pier 1 Pier 2 North Abutment BY DATE Approach Slabs 76.60 17,263 W.D.U. 8-67 915.12 364.21 382,078.4 Total 597.32 /38,383 408 162 W.E.O. 10-67 HM.W. 4-16 AS BUILT BY DATE W.E.O. REVISION

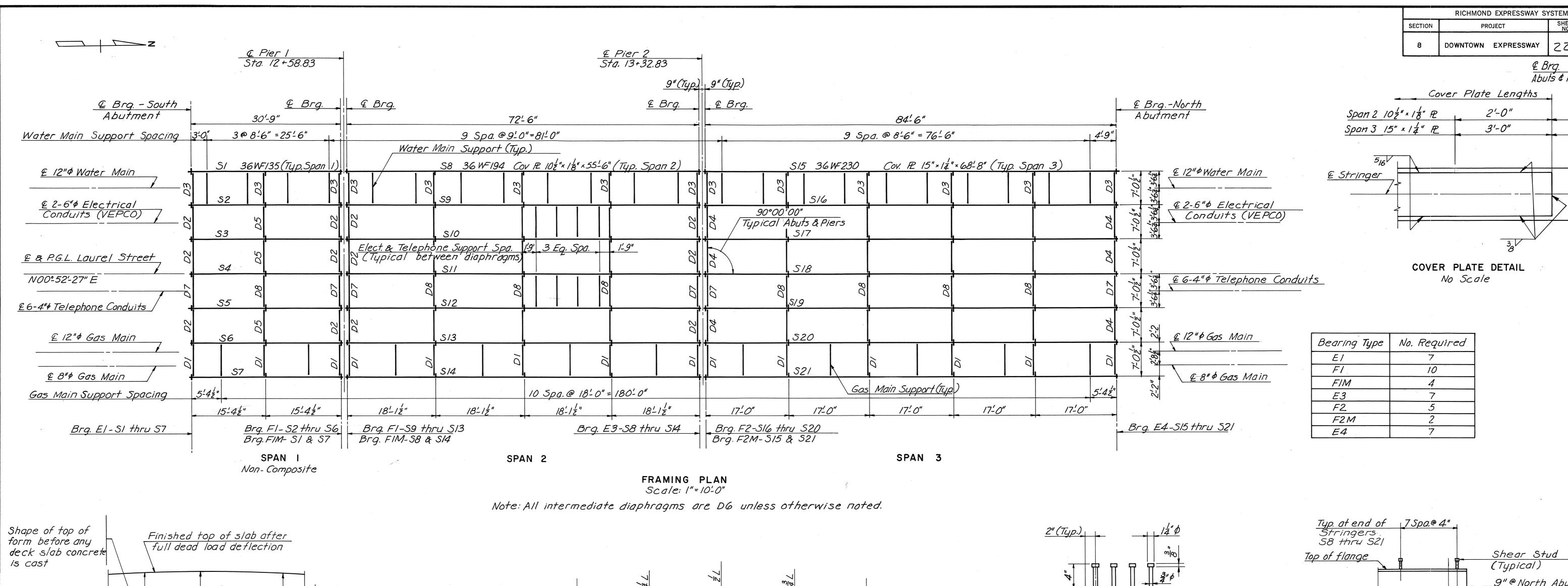
+1.63%

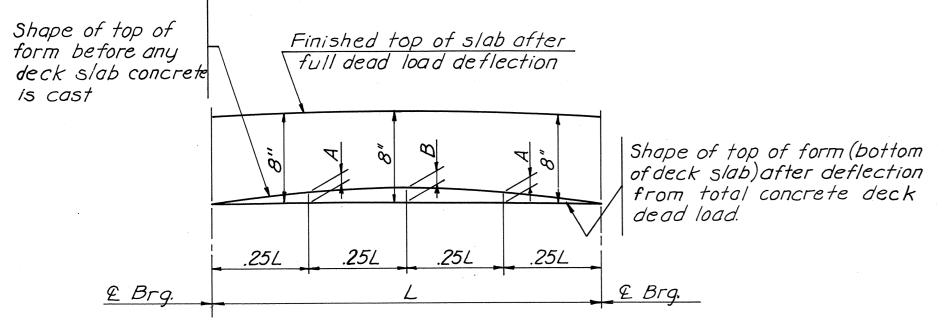
MADE

CHECKED

IN CHARGE

LAUREL STREET





DEAD LOAD DEFLECTION DIAGRAM

1		
Stringers	A	В
SI thru S7	16"	8"
<i>S8</i> , <i>S14</i>	<i>I</i> ⁿ .	17/16
59 thru 5/3	7 "	13/6"
S/5, S2/	/ ½"	2 16"
S16 thru 520	1/4"	$/\frac{3}{4}''$

NOTE TO CONTRACTOR:

BY DATE

F.B.C. 7-67

W.E.O. 9-67

REVISION

BY

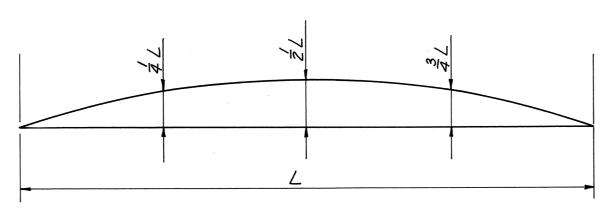
DATE

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IN CHARGE W.E.O.

The above deflections are those anticipated to occur in the stringer upon placement of the total concrete deck dead load. In practice the stringers in place are not likely to have the exact camber to compensate for these deflections during construction. The residual amounts shall be provided for by adjusting forms to vary the thickness of the concrete bolster between the bottom of the slab and the top of the stringer, without alteration of the slab thickness.

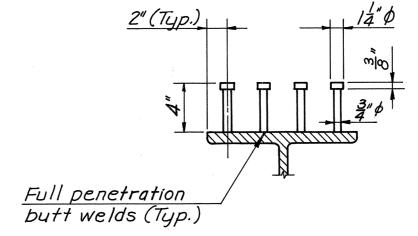


CAMBER DIAGRAM

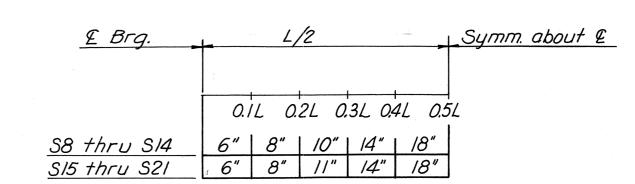
Stringers	$\frac{1}{4}L, \frac{3}{4}L$	<u> </u>
S8, S/4	15"	/ 7 "
S9 thru S13	3/9	1#6"
S/5, S2/	2"	2/3"
S16 thru S20	/ /3 "	2 9 "

NOTE TO FABRICATOR:

The above stringers shall be fabricated with an upward camber amounting to (see table). This will provide approximate compensation for deflection under full dead load and for conformity with finished grade. Stringers which are not required to be shop cambered shall be turned so that any mill tolerance deviation from straightness will be in the direction shown by the camber diagram above.



SHEAR STUD DETAIL Scale: 12"=1-0"

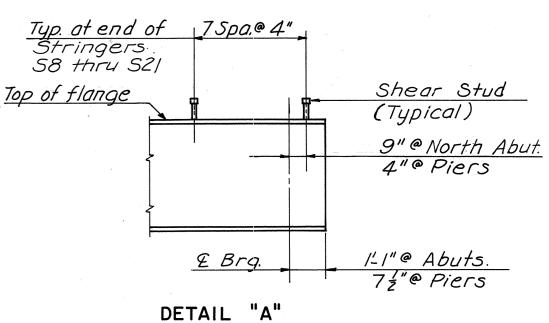


SHEAR STUD SPACING

No Scale

Shear Stud spacing shown is maximum

Shear Stud Notes: Capacity = 3400 lbs. per stud. Three # diameter studs may be substituted at the same longitudinal spacing as shown for four $\frac{3}{4}$ " diameter studs. Stud rows to be placed perpendicular to & stringer. For end condition see Detail "A".



No Scale

Structural steel shall conform to ASTM Specifications A36 (latest revision).

For General Notes see Sheet 1. For Superstructure Cross Section see Sheet 6.

For Diaphragm Details see Sheet 6. For Utility Support Details see Sheet 6.

For Bearing Shoe Dimensions see Standard Sheet Sl.

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM DOWNTOWN EXPRESSWAY

> STRUCTURE B 50 LAUREL STREET OVER

DOWNTOWN EXPRESSWAY FRAMING PLAN

AMERICAN ENGINEERS Richmond, Virginia

CONTRACT NO.: 8 HOWARD, NEEDLES, TAMMEN & BERGENDOFF SHEET NO. 5 OF 9 General Consultants

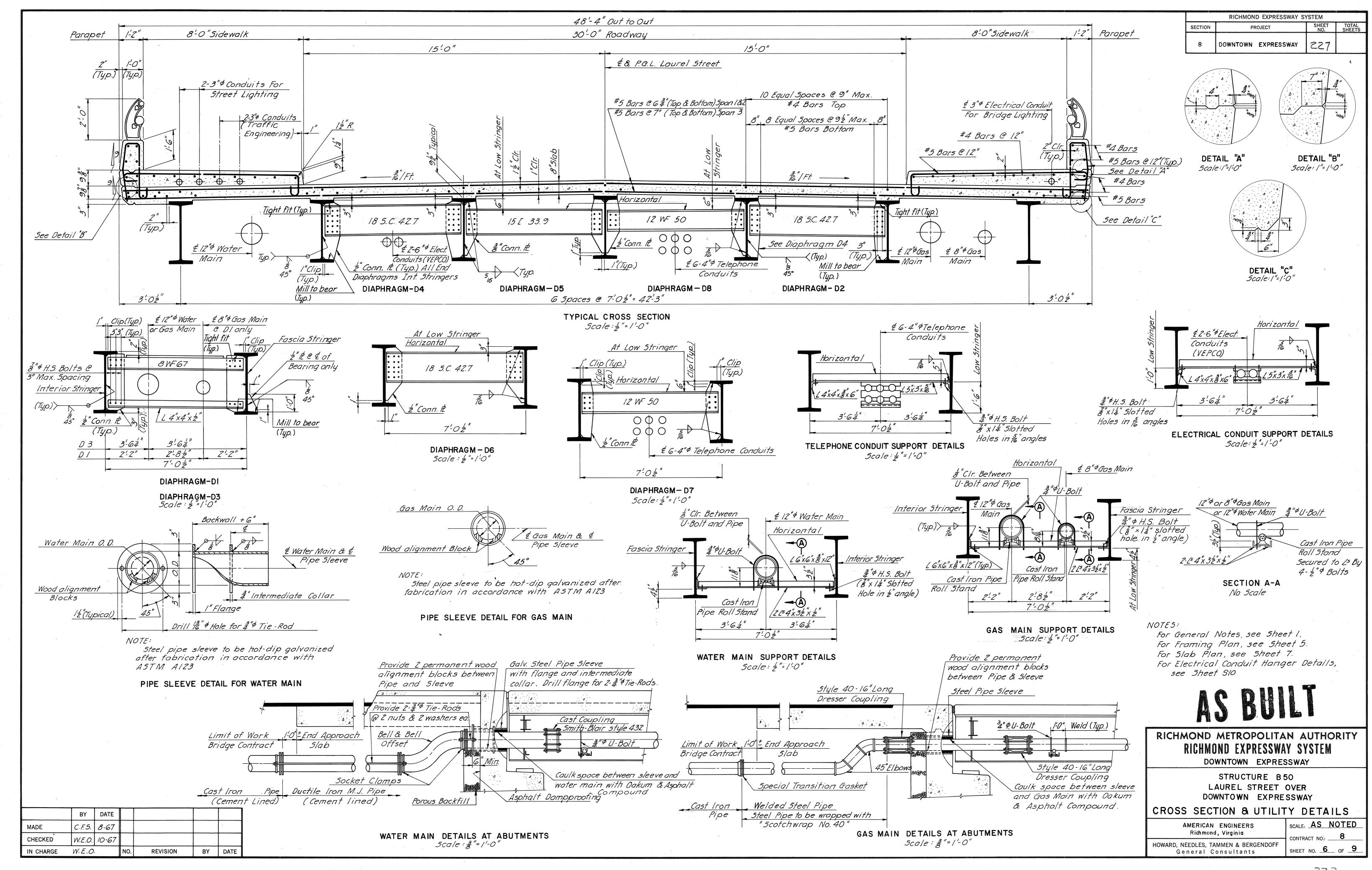
226

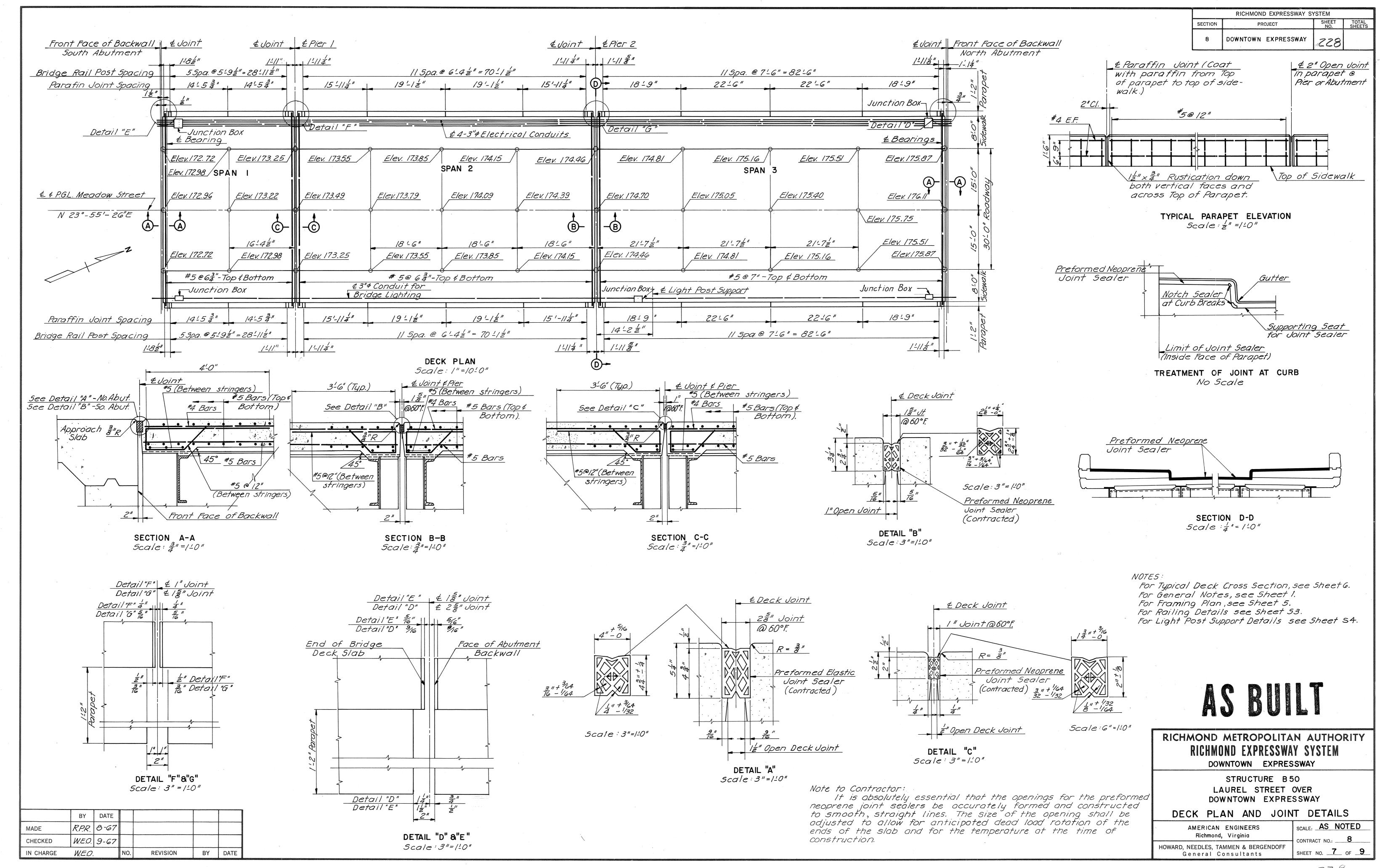
SCALE: AS NOTED

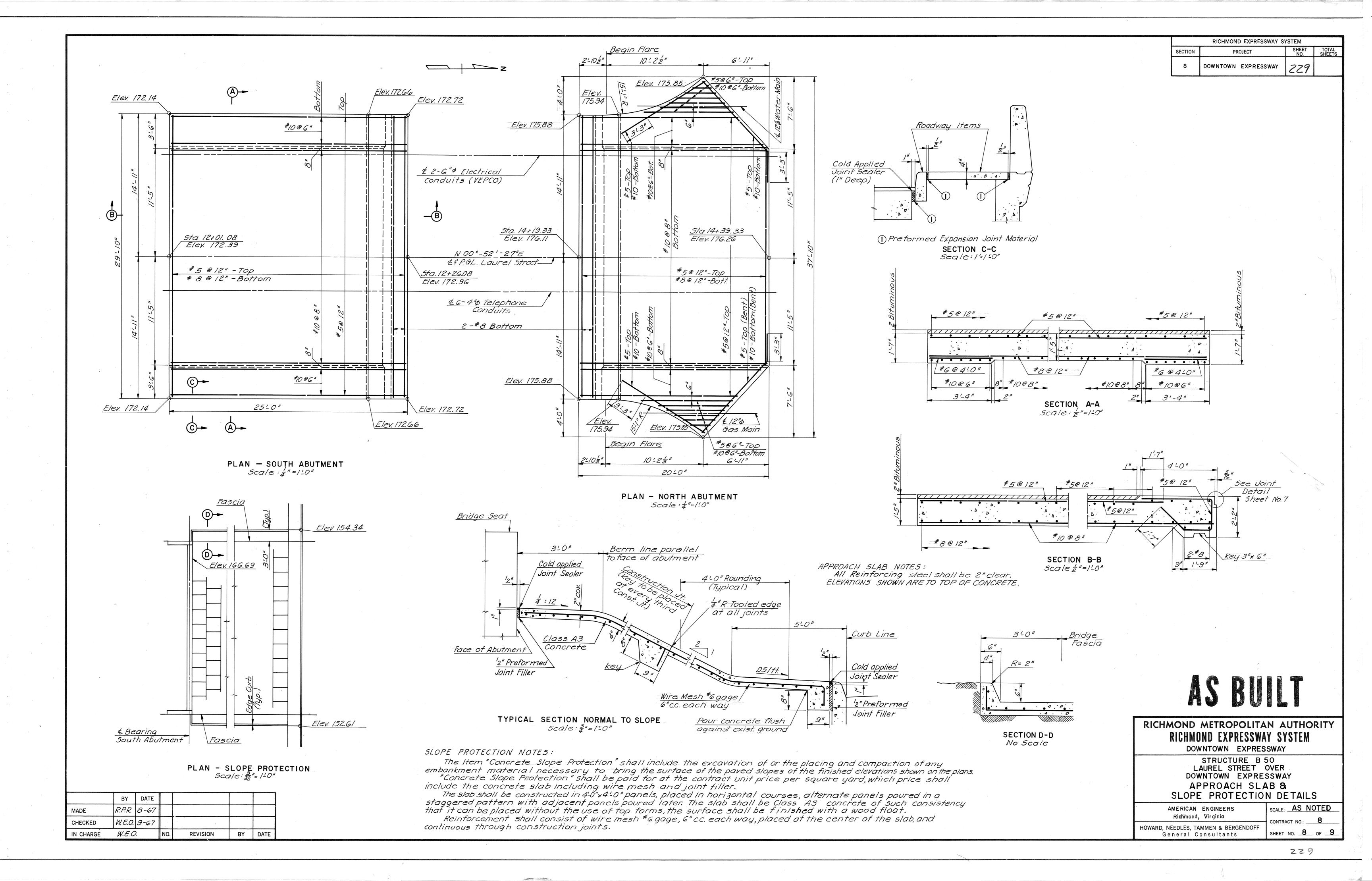
£ Brg. Abuts & Piers

8-6"

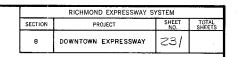
7-//"

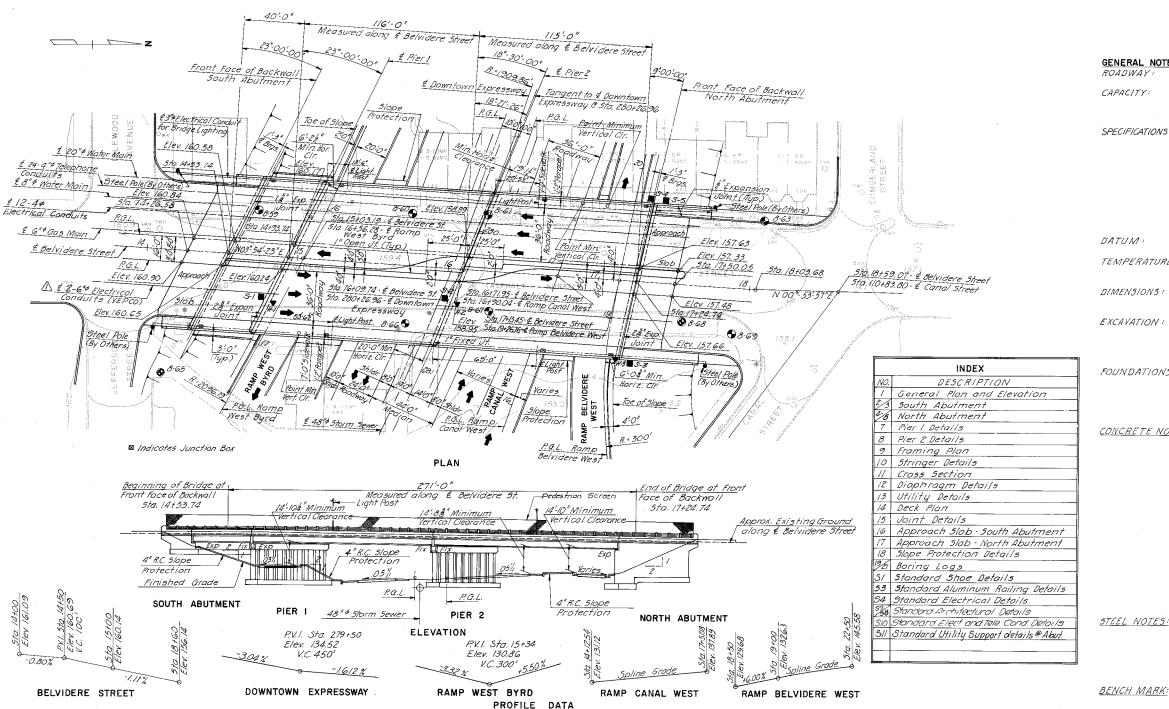






(South Belvidere Street Over Downtown Expressway {Rte. 195})





GENERAL NOTES:

ROADWAY. Two 36'-0" Clear roadways. Two 7'0" Sidewalks. One

variable median.

CAPACITY: Dead Lood - includes 15 lbs. per sq. ft. for future wearing

Live Loads - HS20-44 loading and B.P.R. modified for militaru vehicles.

SPECIFICATIONS: GENERAL - Virginia Department of Highway Road and Bridge Specifications 1970

DESIGN - A.A.S.H.O. Standard Specifications for Highway

Bridges, 1961 modified by Special Design Provisions.
WELDING 1969 Standard Specifications for welded Highway and Railway Bridges of the American Welding Society.

CONTRACT SPECIAL PROVISIONS:

Specifications and Contract Special Provisions referred to above are necessary to make these Plans complete.

DATUM:

CITY OF RICHMOND

TEMPERATURE: The normal temperature referred to on the plan is 60°F. The temperature range for movement is 0°F to 120°F.

All dimensions are measured horizontally and vertically

unless otherwise noted.

Excavation below subgrade and cut slope template shall be classified as Structure Excavation. All excavation above these limits shall be classified as Regular Excavation

and is not included in the Structural Quantities.

FOUNDATIONS: Footings shall rest on firm material. Foundation material shall be kept dry and special attention is called to Section 401.05 of the General Specifications, and to the Contract Special Provisions, concerning preparation of foundations

for footings.

CONCRETE NOTES:

Concrete in superstructure shall be Class A4. All other concrete shall be Class A3. All exposed edges and corners shall have a a chamfer or fillet unless otherwise noted. Care in the method of vibration, the use of low-slump concrete and for other means shall be employed to prevent downgrade movement of newly placed slab concrete. (When gradient is over 2%).

Finishing concrete surfaces : See the Standard Architectural Detail Sheets and the Contract Special Provisions for types

and details.

All reinforcing steel shall conform to ASTM A615 Grade 40. All reinforcing bar dimensions on the detailed drawings are to centers of bors unless otherwise noted. Clear distance between reinforcing steel and face of concrete shall be as noted on the plans. All bar laps shall be 30 diameters of the smaller diameter bar unless otherwise noted.

STEEL NOTES:

Structural steel shall conform to A.S.T.M. Specification A36 except as noted.

All field connections shall be made with high strength bolts. High Strength bolts shall be & " o unless otherwise noted and shall conform to A.S.T.M. Specification A.325.

BENCH MARK: C-40 Monument located at S.E. corner Idlewood Avenue and Belvidere Street. Elev. 160.76.

									ESTIMATED	QUANTITIES											
	STRUCTURE EXCAVATION	CONCRETE CLASS A4	CONCRETE CL A55 A 3	REIN FORCING STEEL	5TRUCTURAL 5TEEL A36	ALUMINUM BRIDGE RAILING	SLAB SLOPE	ASPHALT DAMP- PROOFING	6" INCH PIPE UNDERDRAIN	POROUS BACKFILL	GAS MAIN 6" \$	WATER MAIN	WATER MAIN 8" \$		CONDUIT 4" # TELE.				2"\$ METAL CONDUIT		
	C.Y.	C. Y.	C. Y.	L B5.	LB5	L.F.	5. Y.	5 Y.	L.F.	C.Y.	L. F.	L. F.	L.F.	4. F.	L.F	L.F.	L. F.	4. F.	L.F.		
Superstructure		962.16		224,/4/	1,324,511.8	658					323	247	3.13	664 1	6,504	665	3,252	570	16		
South Abutment	475		234.47	11,589			564.9	228	126	76							-,				
Pier /	598		258.89	50,865			295.1			····						—	-		 		†
Pier 2	3/8		258.71	48,366					T										 	-	+
North Abutment	1,194		891.55	75,607			/3/.2	468	170	289			-			1					
Approach Slabs			25 8.0 8	58,162						200											t
Total	2,585	962.16	1,901.70	468,730	1, 324, 511.8	658 ·	.991.2	696	296	365	323	247	3/3	664 /\	6504	665	3252	570	16		+

NOTES: Top of Pavement Elevations at ends of Deck along P.G.L.'s are given on Plan; Remaining Payement Elevations are given on Sheet 14

8 Indicates 22 4 Cased Hole Boring

⊕ Indicates 4" Cased Hole Boring
▼ Indicates Geonor Heave Points

BY DATE

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■ Indicates Settlement Points

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

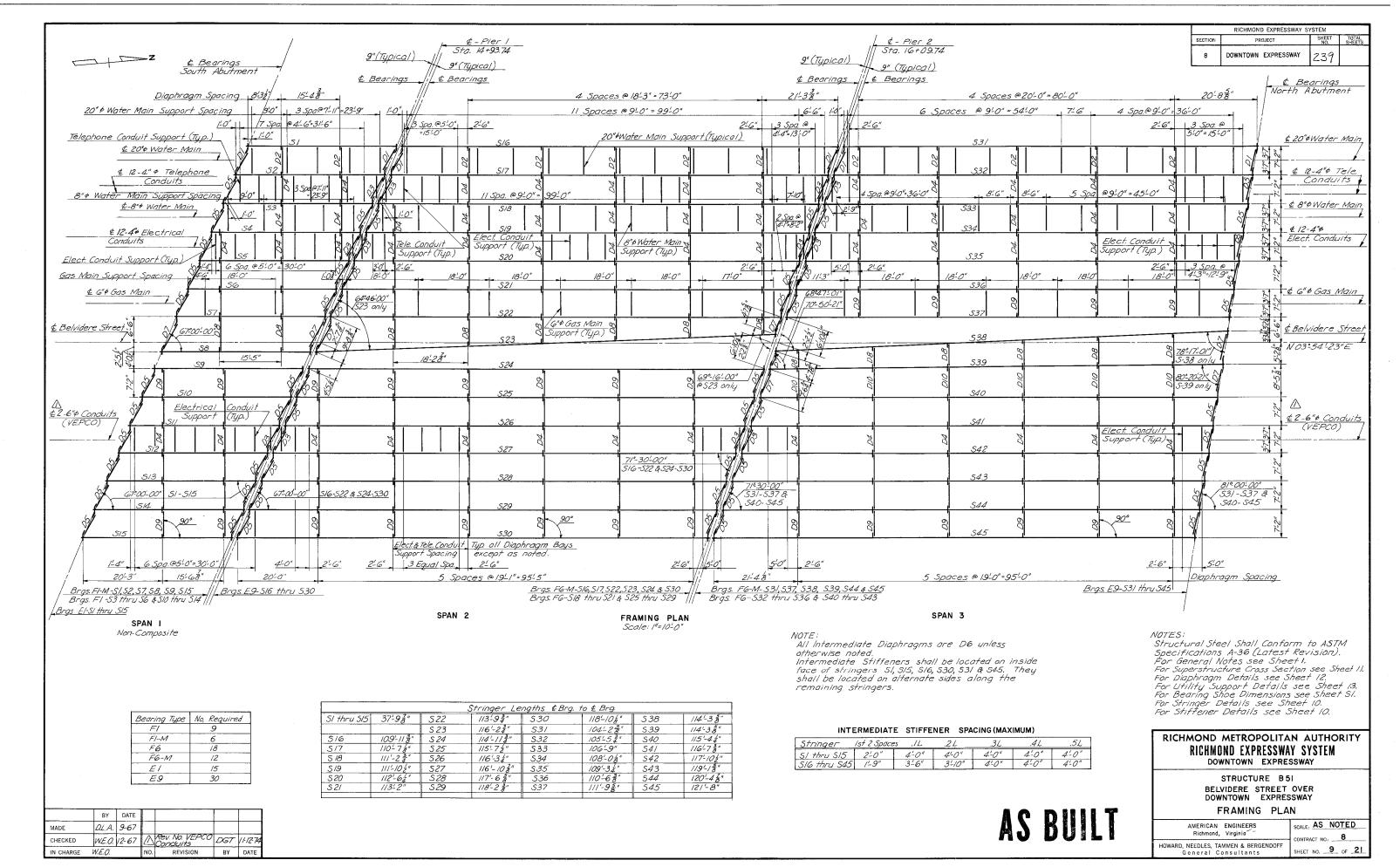
DOWNTOWN EXPRESSWAY

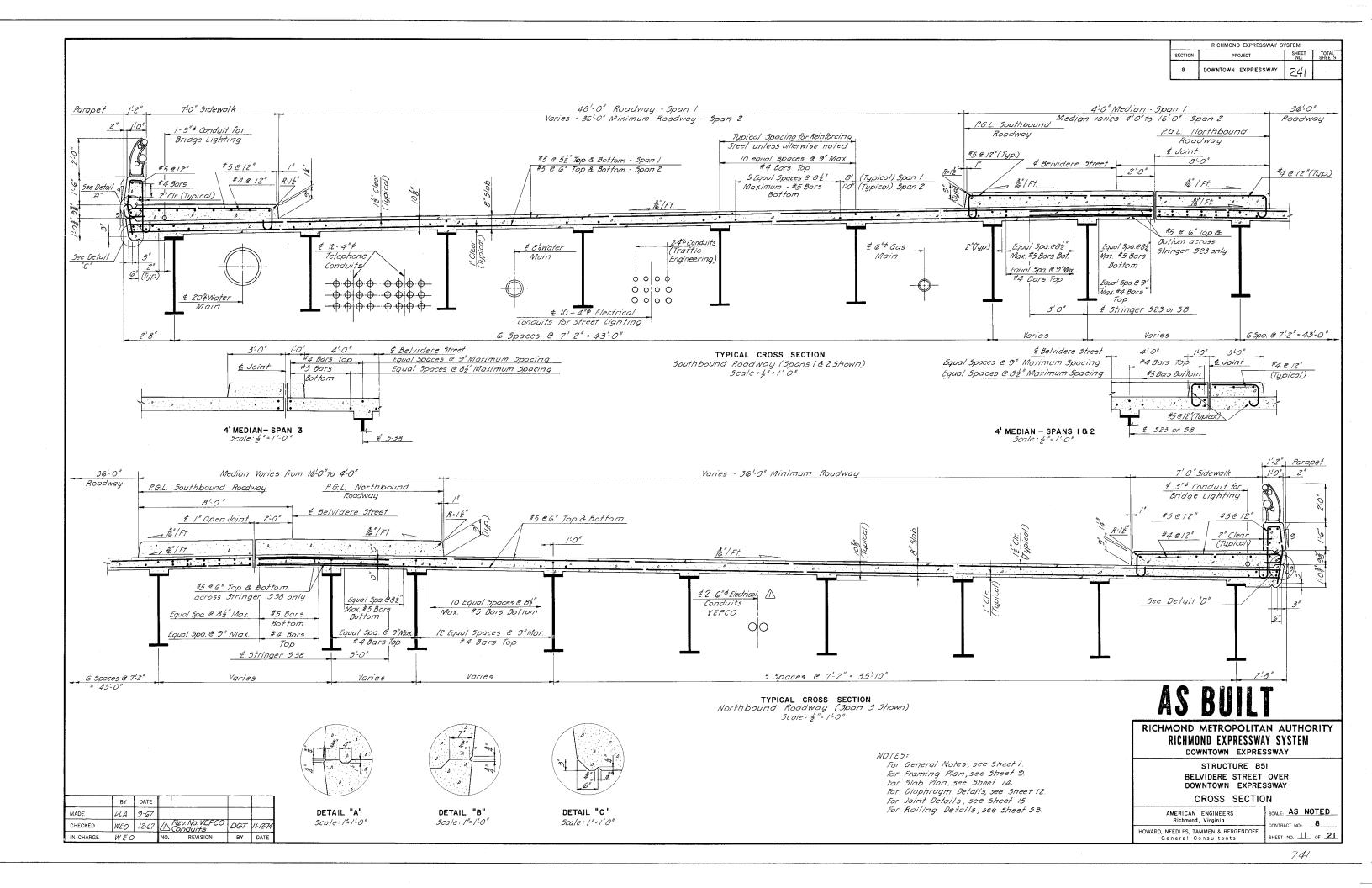
STRUCTURE B 51 BELVIDERE STREET OVER DOWNTOWN EXPRESSWAY

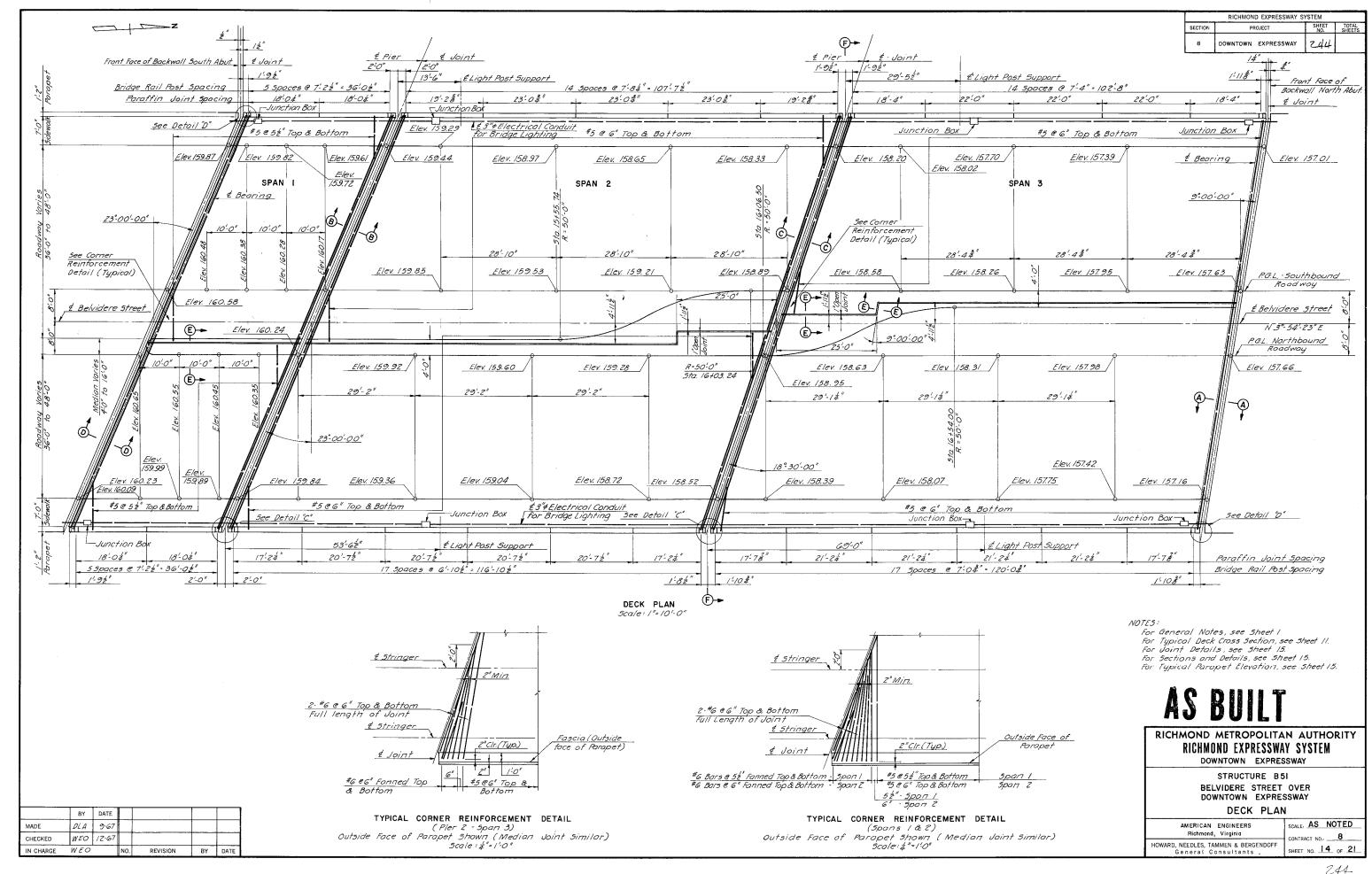
GENERAL PLAN AND ELEVATION SCALE: 1"=30'-0"

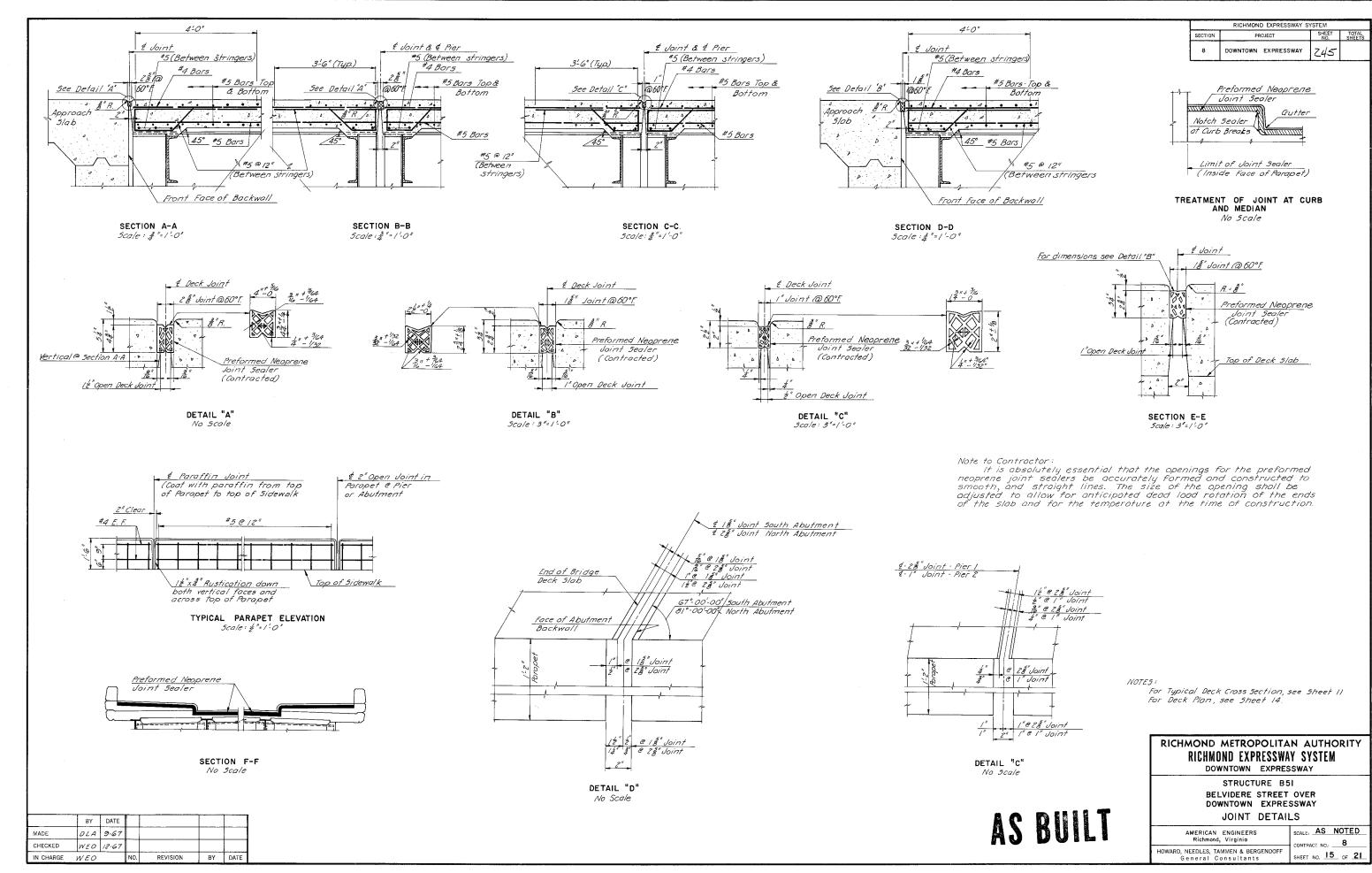
AMERICAN ENGINEERS

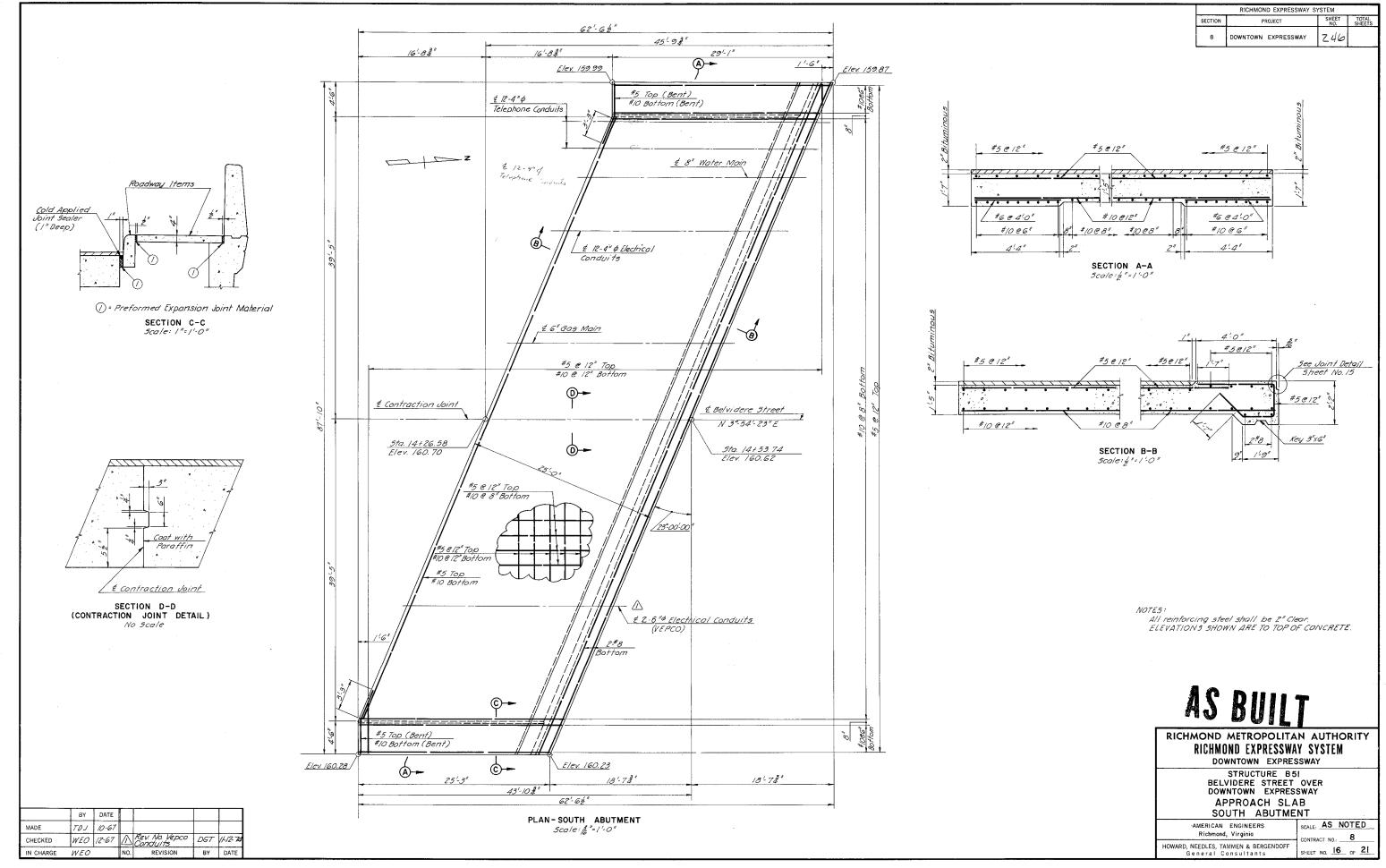
ONTRACT NO: HOWARD, NEEDLES, TAMMEN & BERGENDOFF General Consultants SHEET NO. 1 OF 21

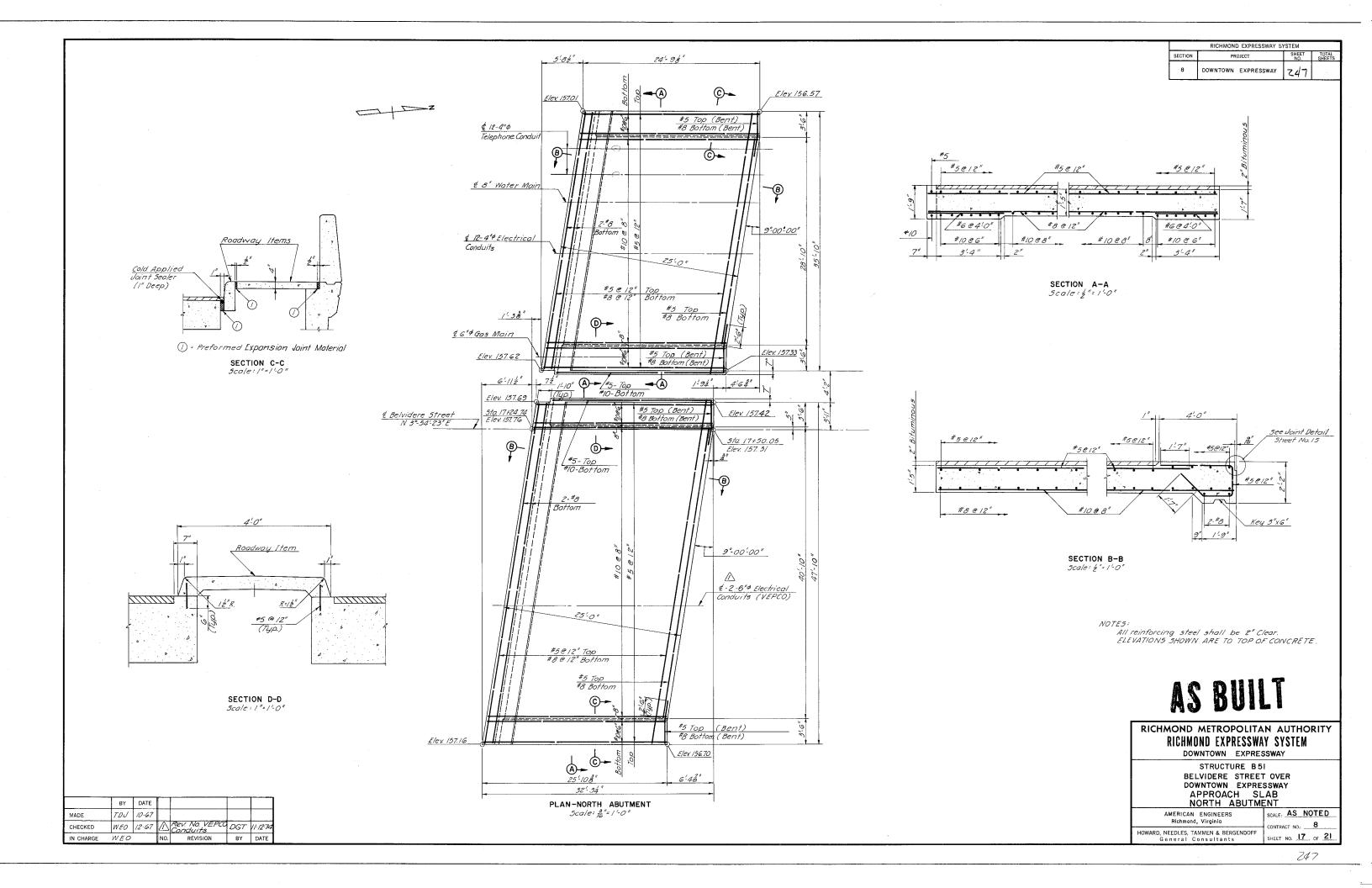




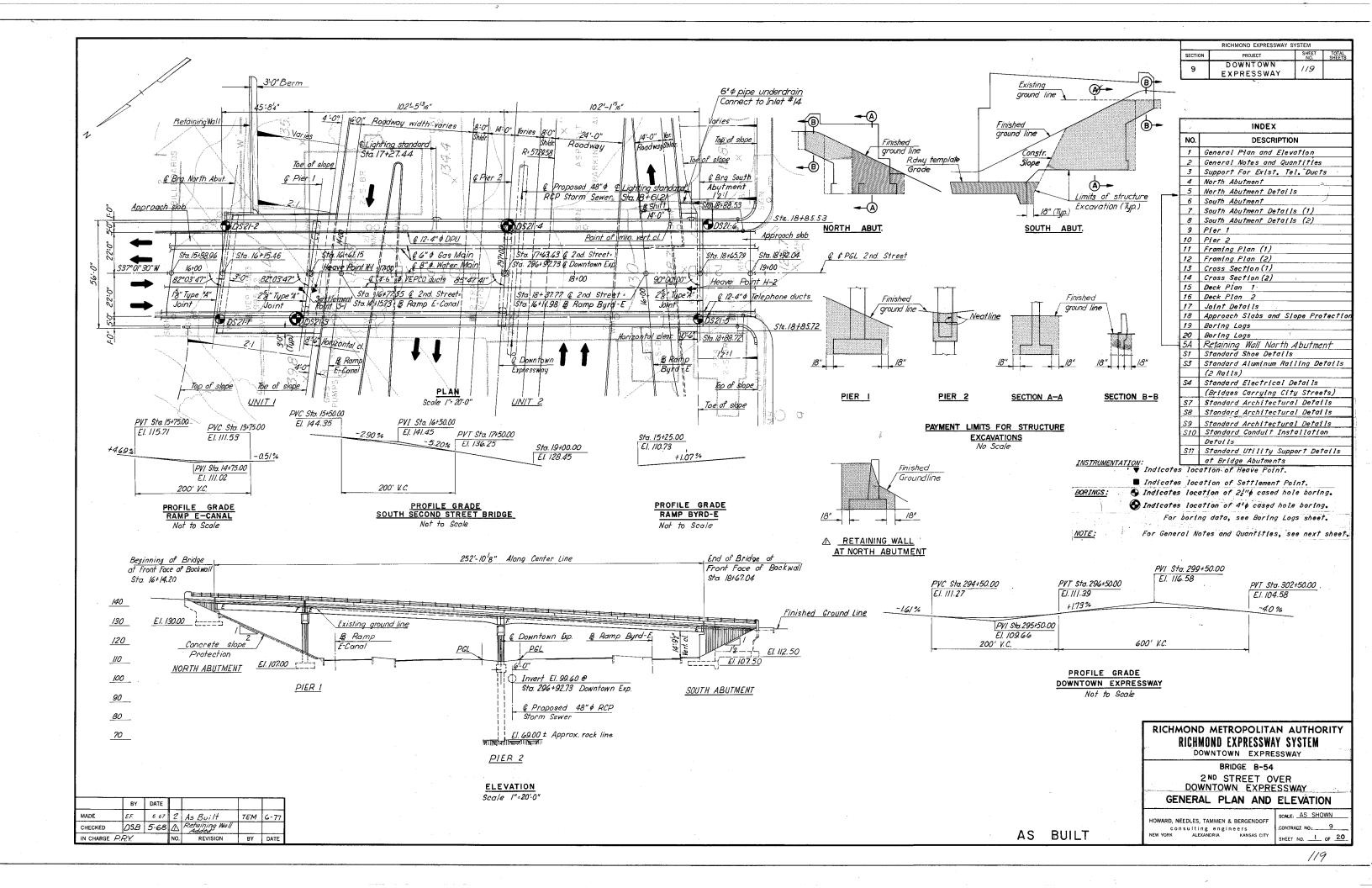


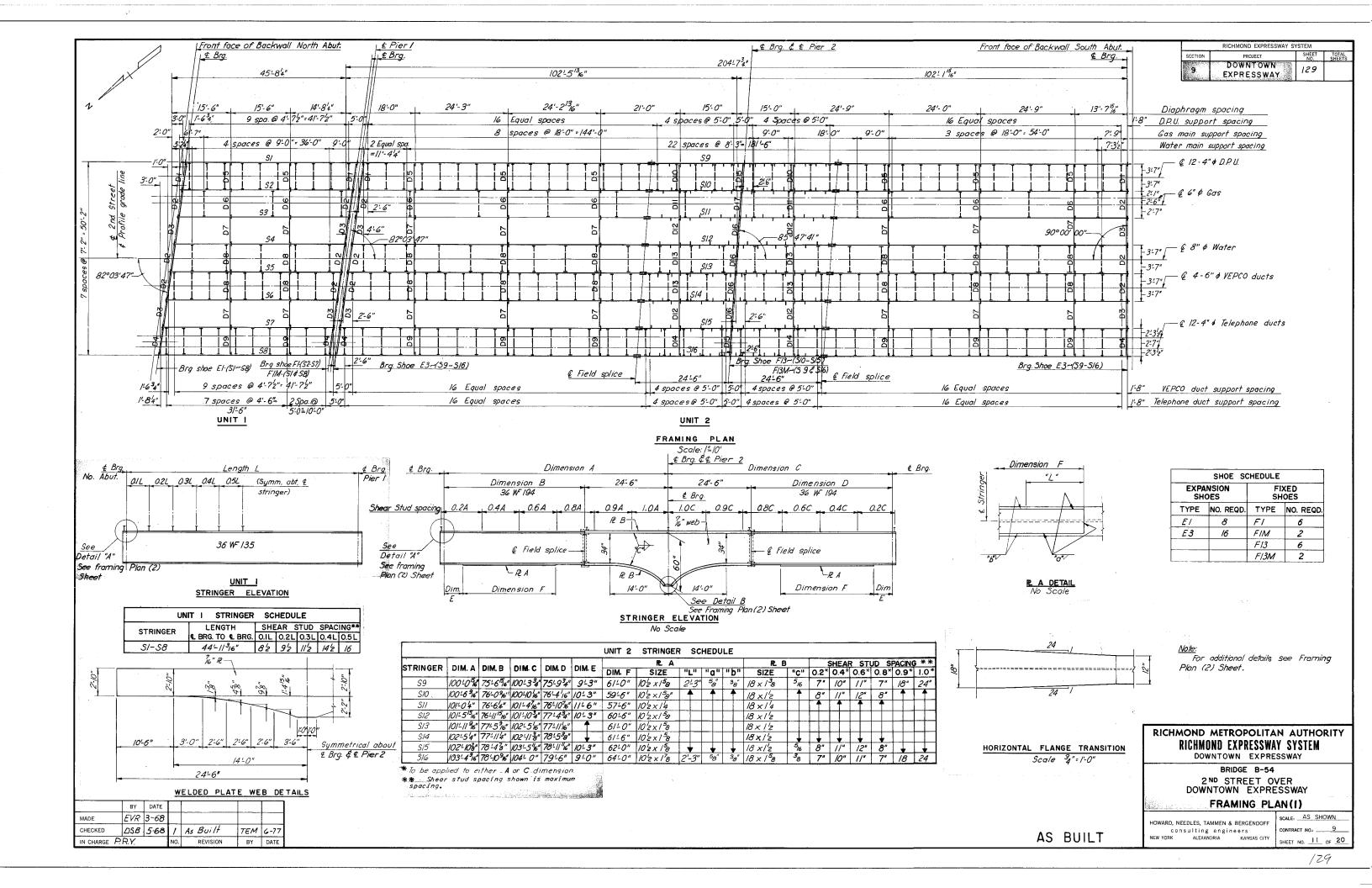


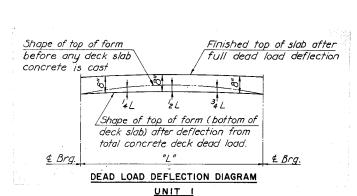




(South 2nd Street Over Downtown Expressway {Rte. 195})







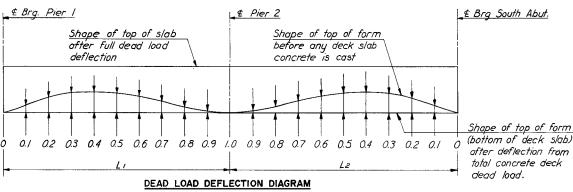
NOTE TO CONTRACTOR:

DEFLECTION SCHEDULE 3_{9"} 5/-58 4"

NOTE TO CONIRACION:

The above deflections are those anticipated to occur in the stringer upon placement of the total concrete deck dead load.

In practice, the stringers in place are not likely to have the exact comber to compensate for these deflections during construction. The residual amounts shall be provided for by adjusting forms to vary the thickness of the concrete bolster between the bottom of the slab and the top of stringer, without alteration of the slab thickness.



UNIT 2

			DE	FLECTIC	N SCH	EDULE		٠		
STR.	0.IL ₁	0.2L ₁ 0.2L ₂					0.7L ₁	0.8L ₁ 0.8L	0.9L ₁ 0.9L ₂	1.0
59	1/16"	14"	/ ⁵ 8"	1/3/6"	11/16"	1716"	/"	12"	[/] 8"	0"
510-512	58"	//8"	1/2"	158"	1/2"	14"	78"	716"	<i>lg"</i>	0"
5/3-5/5	34"	14"	1"16"	1/3/6"	1"16"	1716"	/"	2"	/s"	0"
5/6	34"	/ ³ 8"	136"	2"	/8"	1916"	1'16"	916"	3/6"	0"

\$ Brg. Pier I £ Pier 2 £ Brg. South Abut. 0/ 02 03 04 05 06 07 08 09 10 09 08 07 06 05 04 03 02 0/

CAMBER DIAGRAM UNIT 2

CAMBER SCHEDULE STR. 4L 2L 3L S8 58" /" 58"

CAMBER DIAGRAM

UNIT I

NOTE TO FABRICATOR

⊈ Brg.

NOTE TO FABRICATOR:

The above stringers shall be fabricated with an upward camber amounting to the tabulated value.

This will provide approximate compensation for deflection under full dead load and for conformity with finished grade.

Stringers which are not required to be shop cambered shall be turned so that any mill tolerance deviation from straightness will be in the direction shown by the camber diagram above.

Dimensions	~	10	inchas
Dimensions	0, 6	.,,	11161163

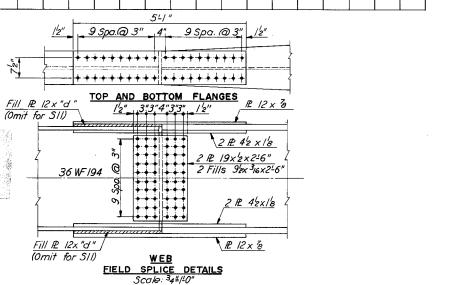
								CAMB	ER SCI	HEDUL	-E								
STR.	0.IL _I	0.2L ₁	0.3L ₁	0.4L ₁	0.5L ₁	0.6L ₁	0.7L ₁	0.8L ₁	0.9L ₁	1.0	0.91ء	مـ8.0	0.7L ₂	0.6L ₂	0.5L2	0.4L ₂	0.3L ₂	0.2ل	O.IL
59	1/2"	21/6"	32"	3%"	3/3/6"	33 ₈ "	296"	/ ⁵ 8"	1/16"	0"	3/6"	58"	14"	113/6"	23/6"	24"	216"	158	15/6
SIO-SI2	1/2"	25g"	3%"	334	31/6"	34"	2 2"	/ ⁹ /6"	1/16"	0"	3/6"	9/6"	18"	/ ⁵ 8"	2"	28"	2"	1/2"	79'
S/3-S/5	1916"	2 13/6"	3"/6"	416"	4"	32"	234"	1116"	34"	0"	³ /6"	5 _{8"}	14"	136"	23/6"	25/6"	2 3/6"	/"16"	15/16
<i>SI6</i>	1116"	3"	313/6"	44"	43/6"	31/16"	278"	//3/6"	34"	0"	3/6"	13/6"	/ ³ 8"	2"	2 ³ 8"	22"	2 ⁵ /6"	1/3/1	1
																			Г

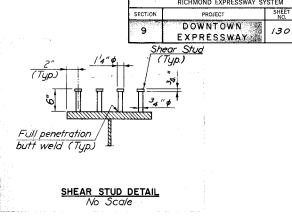
FILL PLATE THICKNESS

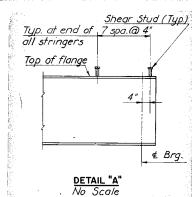
STR.	"d "
5 9	8"
510	/ ₄ "
512-515	4
5/6	38"

STRINGER SPLICE NOTE

Web splice plates shall have horizontal rolled or mill sheared edges. Vertical edges may be shop sheared or flame cut.
For \$9\$ and \$16\$ place heads of H.S. bolts on outside face of web splices.
For all stringers place heads down for flange splices.

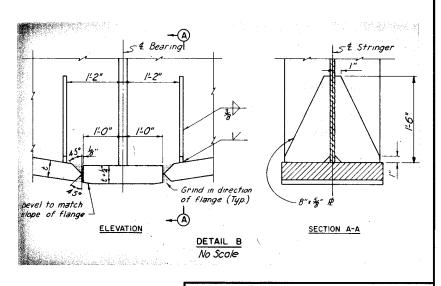






SHEAR STUD NOTE:

Capacity = 3,400 lbs. per stud.
The contractor may, if he elects, use
three i'' diameter studs at the same longitudinal spacing in lieu of the four i'' diameter
studs shown.
Stud rows shall be placed parallel to the
main deck reinforcing.



RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM DOWNTOWN EXPRESSWAY

BRIDGE B-54 2ND STREET OVER DOWNTOWN EXPRESSWAY

FRAMING PLAN (2)

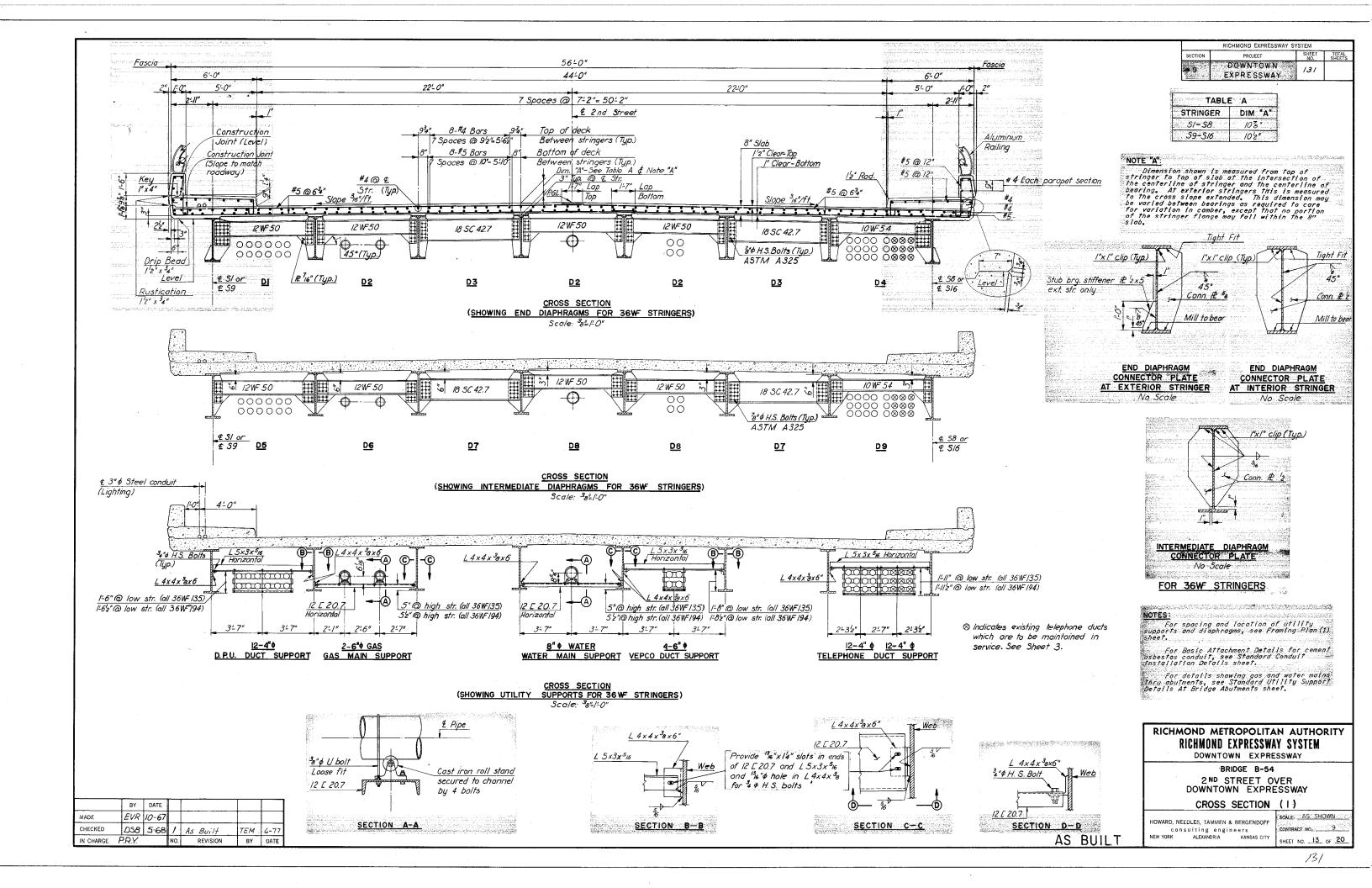
AS BUILT

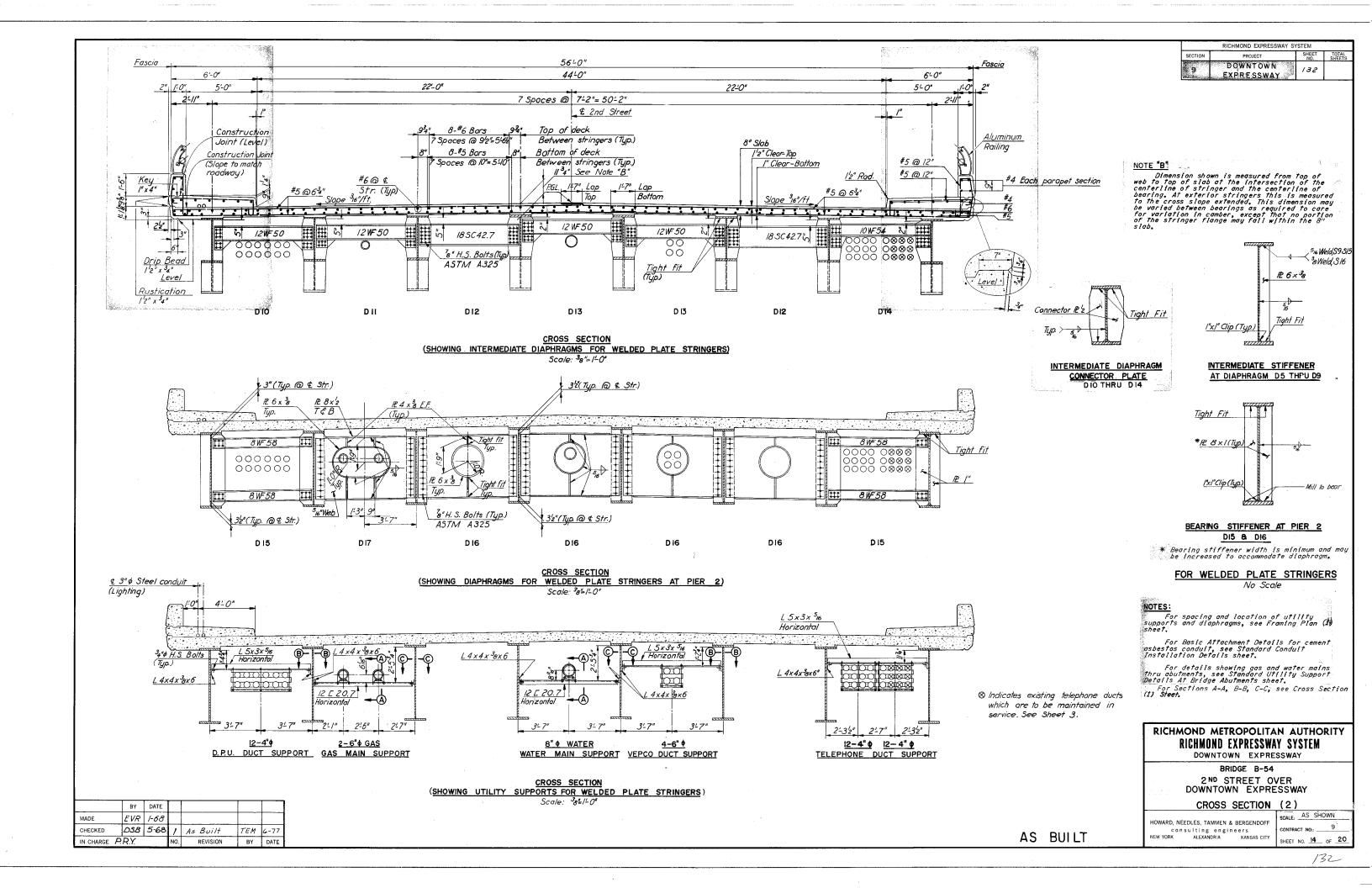
ني HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers
NEW YORK ALEXANDRIA KANSAS CITY

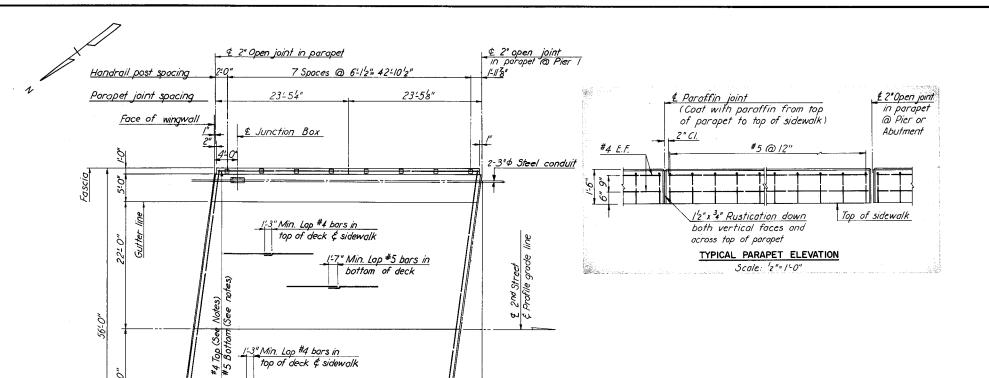
SCALE: AS SHOWN SHEET NO. 12 OF 20

EVR 3-68 MADE CHECKED A.J.S. 3-68 1 As Built IN CHARGE PRY BY DATE REVISION

BY DATE







7-63"

PLAN-UNIT I Scale: 18"=1-0"

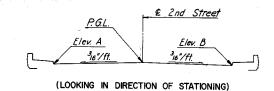
1-7" Min. Lop #5 bors in bottom of deck

1-3" Min. Lop #4 bors in top of deck \$ sidewalk

#5@ 634" Top & Bottom

46-118"

DOWNTOWN /33 EXPRESSWAY



		P/	VEMENT	ELEVATION	VS		
STATION	ELEV. A	€ a. P.G.L.	ELEV. B	STATION	ELEV. A	€ a. P.G.L.	ELEV. B
		*143.10	*142.76	<i>†40.00</i>	136.42	136.76	136.42
16†00.00	*142.42	*142.76	*142.42	17+50.00	135.91	/36.25	135.91
<i>+10.00</i>	*/42.06	*142.40	142.06	+60.00	/35.39	135.73	/35.39
<i>†/1./3</i>			142.02	<i>+62.01</i>			135.28
t/4.20		142.25		<i>†63.63</i>		/35.54	
<i>t</i> /7.26	141.80			<i>†65.25</i>	/35.//		
<i>†20.00</i>	141.70	142.04	141.70	<i>+70.00</i>	134.87	/35.2/	134.87
+30.00	141.32	141.66	141.32	<i>†80.00</i>	/34.35	134.69	134.35
<i>†40.00</i>	140.93	141.27	140.93	+90.00	/33.83	134.17	133.83
16+50.00	/40 .53	/40.87	140.53	18+00.00	/33.3/	133.65	133,37
+58.08			140.20	+ 10.00	132.79	/33./3	132.79
+60.00	140.12	140.46	140.12	<i>†20.00</i>	132.27	132.61	132.27
<i>†61.15</i>		140.42		+30.00	/3/.75	132.09	/3/.75
<i>†64.21</i>	/39.94			<i>†40.00</i>	131.23	/3/.57	/3/ .23
<i>+70.00</i>	139.70	140.04	139.70	18+50.00	/30.7/	131.05	/30.7/
<i>†80.00</i>	139.27	139.61	139.27	+60.00	130.19	130.53	130.19
+90.00	138.82	139.16	138.82	† 6 7.04	129.82	/30./6	129.82
7+00.00	/38.37	/38.7/	/38.37	<i>†</i> 70.00	129.67	/30.0/	129.67
<i>+10.00</i>	137.90	138.24	/37.90	+80.00	* <i>129.15</i>	*129.49	*129.15
<i>†20.00</i>	/37.42	137.76	/37.42	190.00		* <i> 28.97</i>	
+30.00	136.93	137.27	136.93	19+00.00		*128.45	207.42

* Elevations shown ore given to top of bituminous surfacing.

NOTES:

For location and spacing of deck, parapet and sidewalk reinforcing, see Cross Section (1) sheet.

For location and spacing of reinforcing in haunch over end diaphragms, see Joint Details sheet.

For lighting standard base, junction box details and additional reinforcing, see Standard Electrical Details sheet S4.

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM DOWNTOWN EXPRESSWAY

> BRIDGE B-54 2 ND STREET OVER DOWNTOWN EXPRESSWAY

DECK PLAN

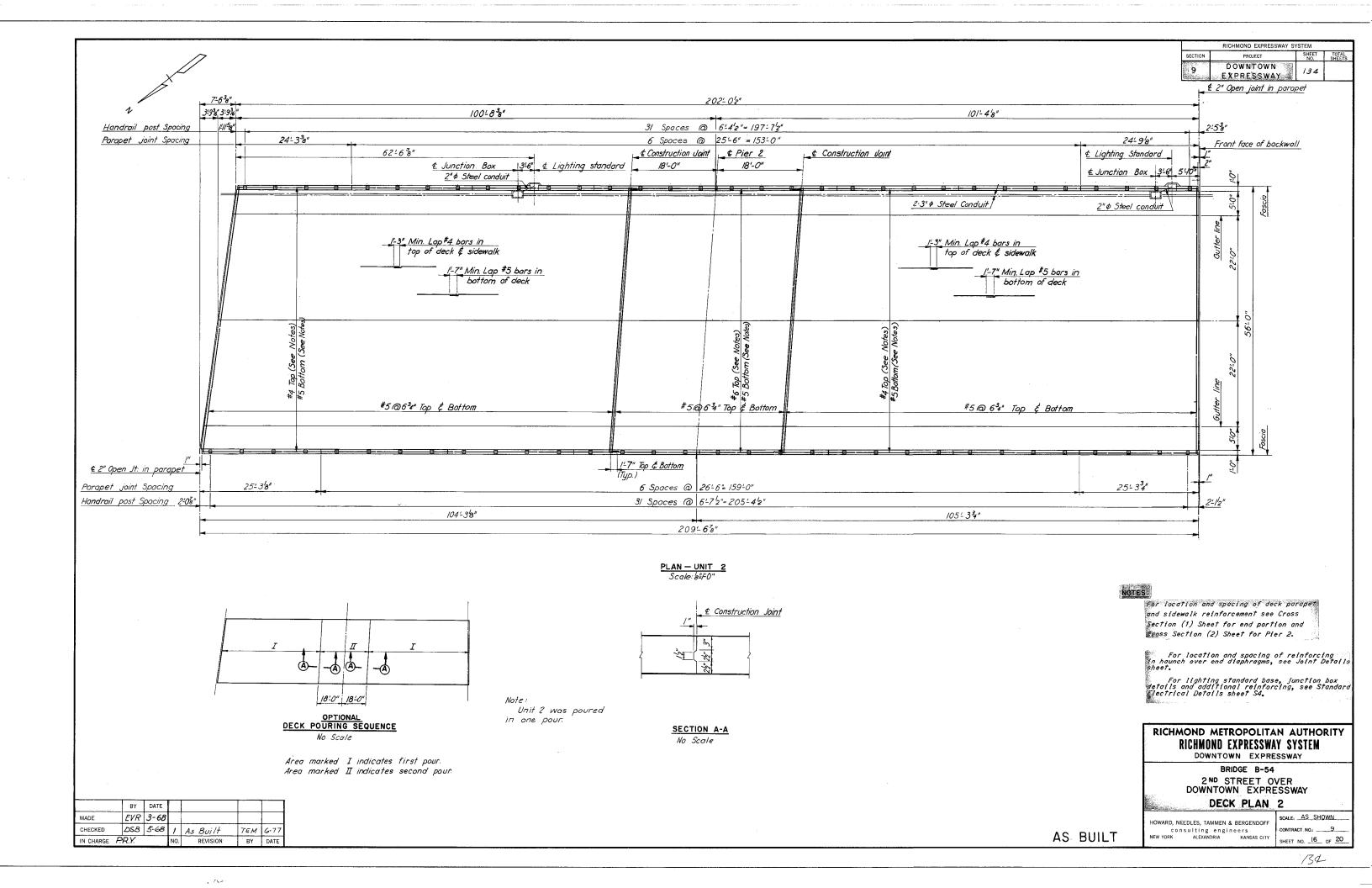
HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers
NEW YORK ALEXANDRIA KANSAS CITY

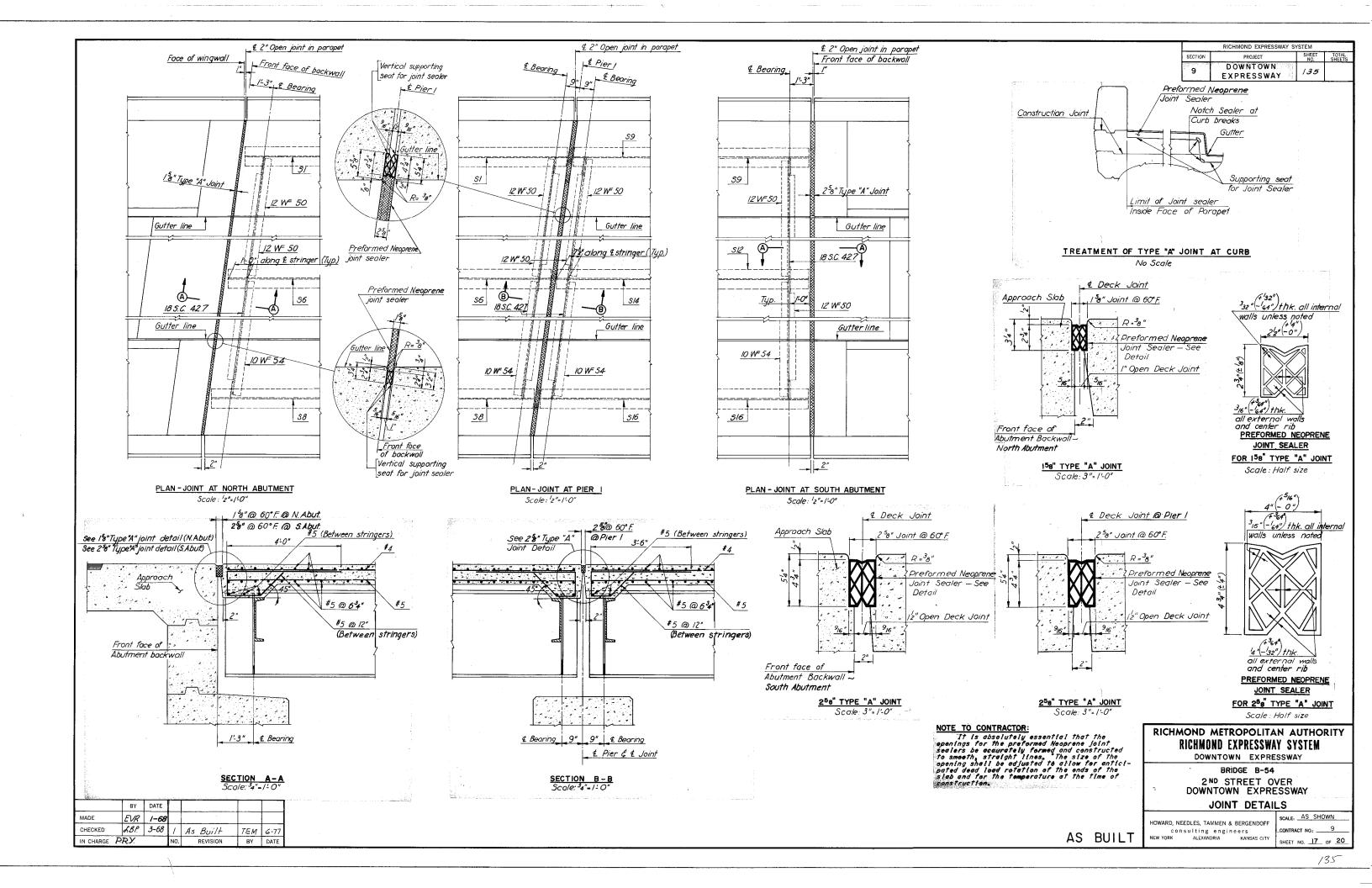
SCALE: AS SHOWN SHEET NO. 15 OF 20

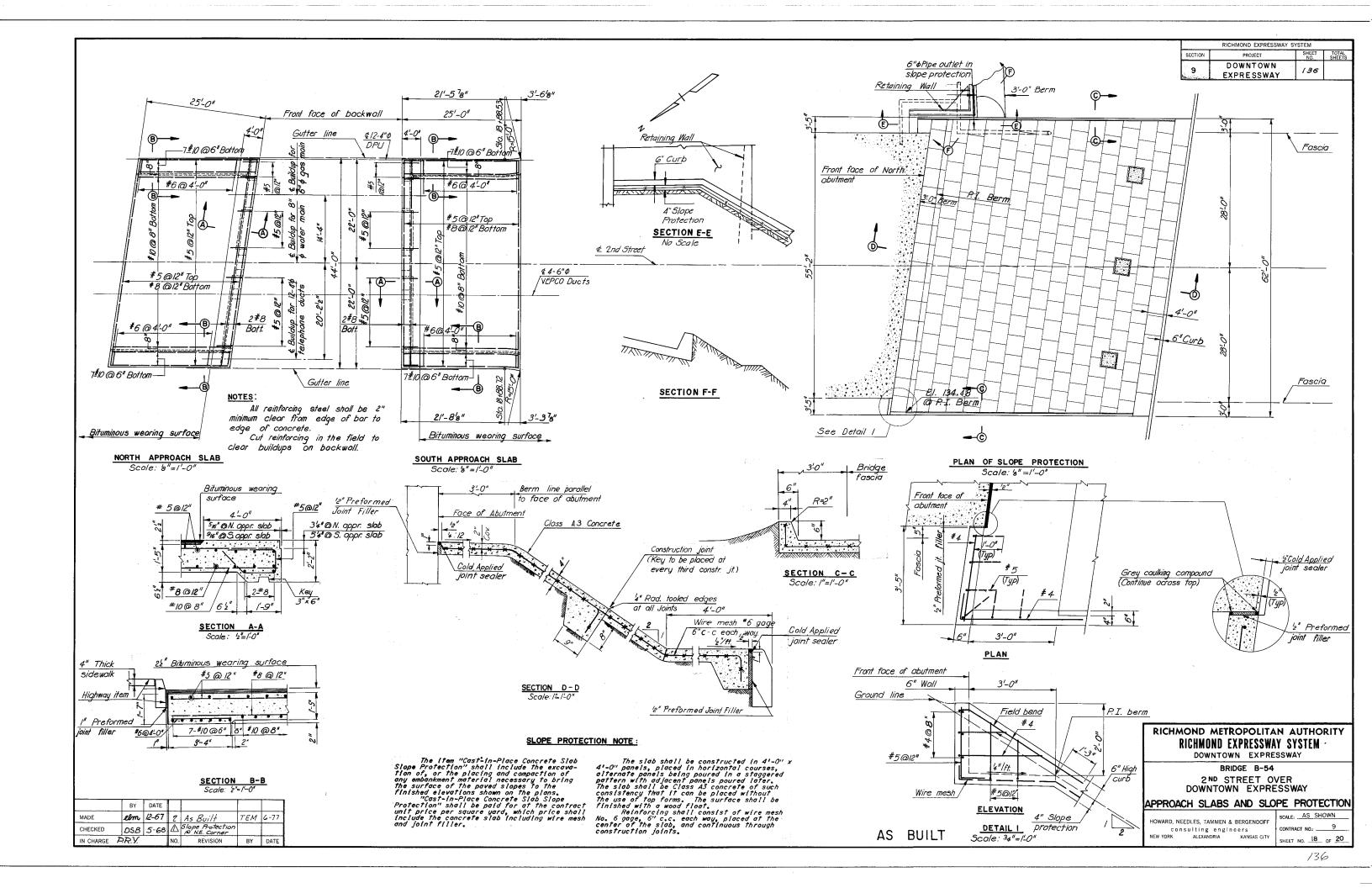
ı		BY	DATE				
	MADE	EVR	3-68				
Ì	CHECKED	ු යනු	5-68	/	As Built	TEM	6-77
Į	IN CHARGE PA	₹.У.		NO.	REVISION	BY	DATE

Face of wingwall

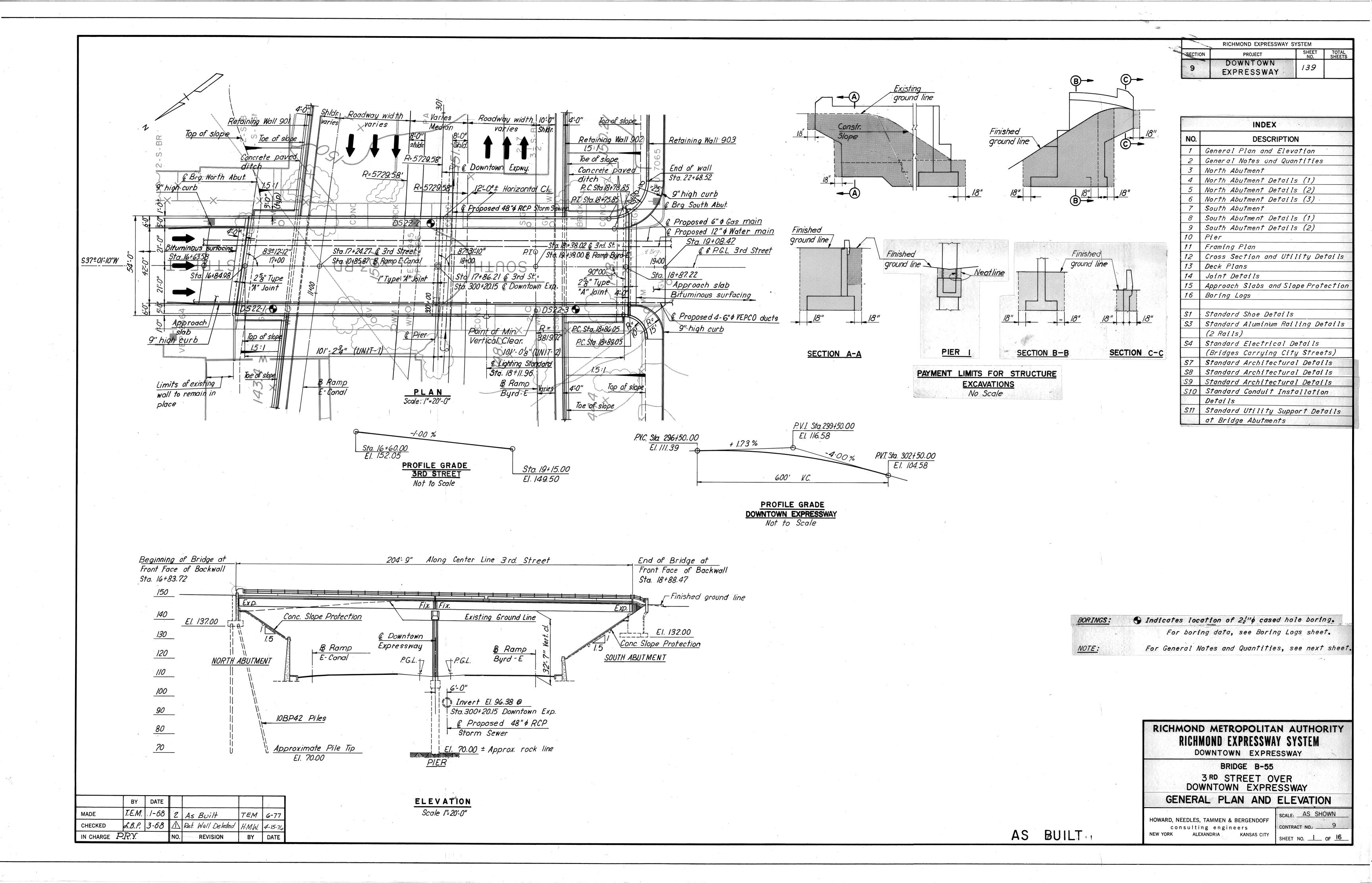
AS BUILT

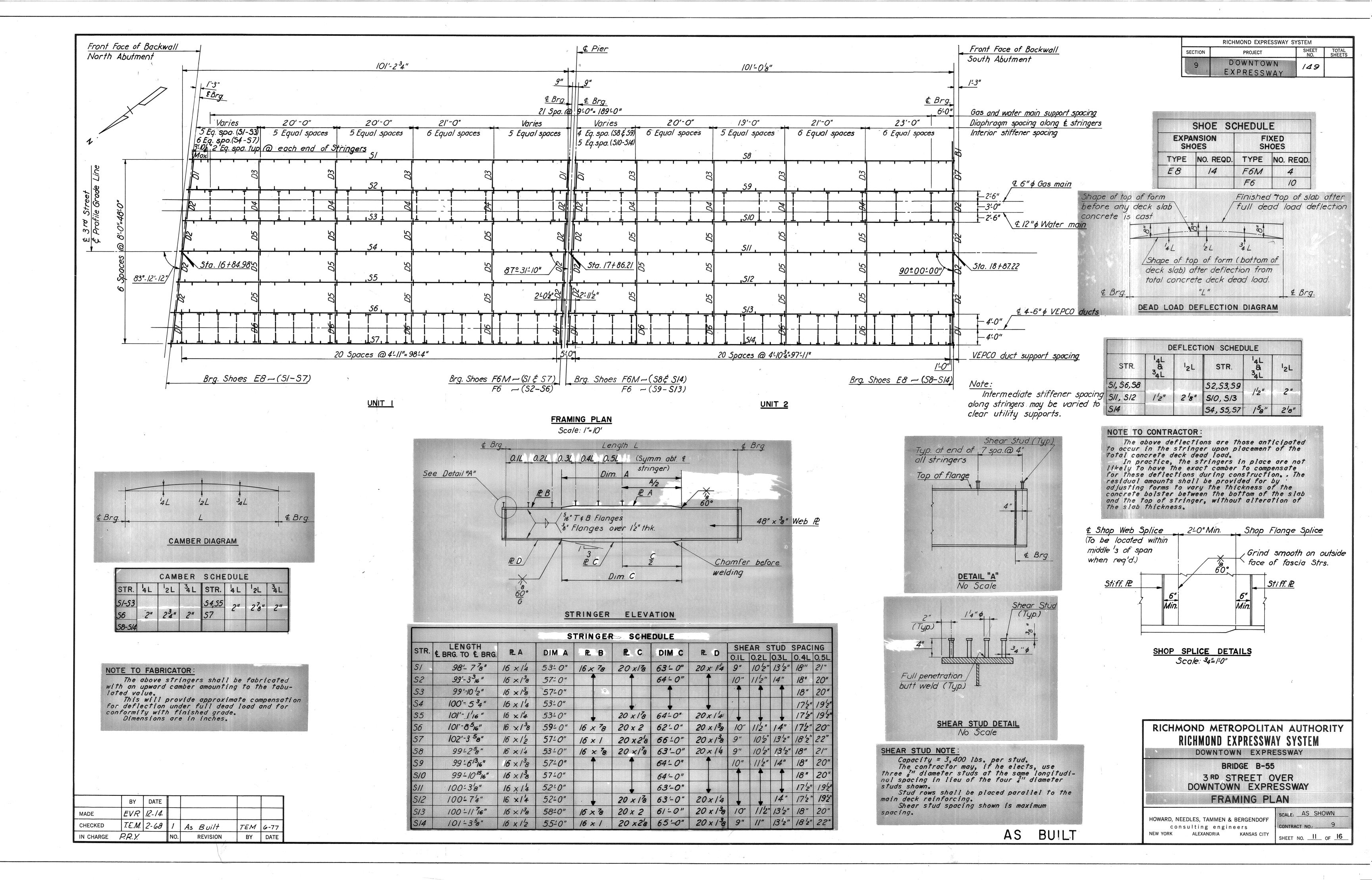


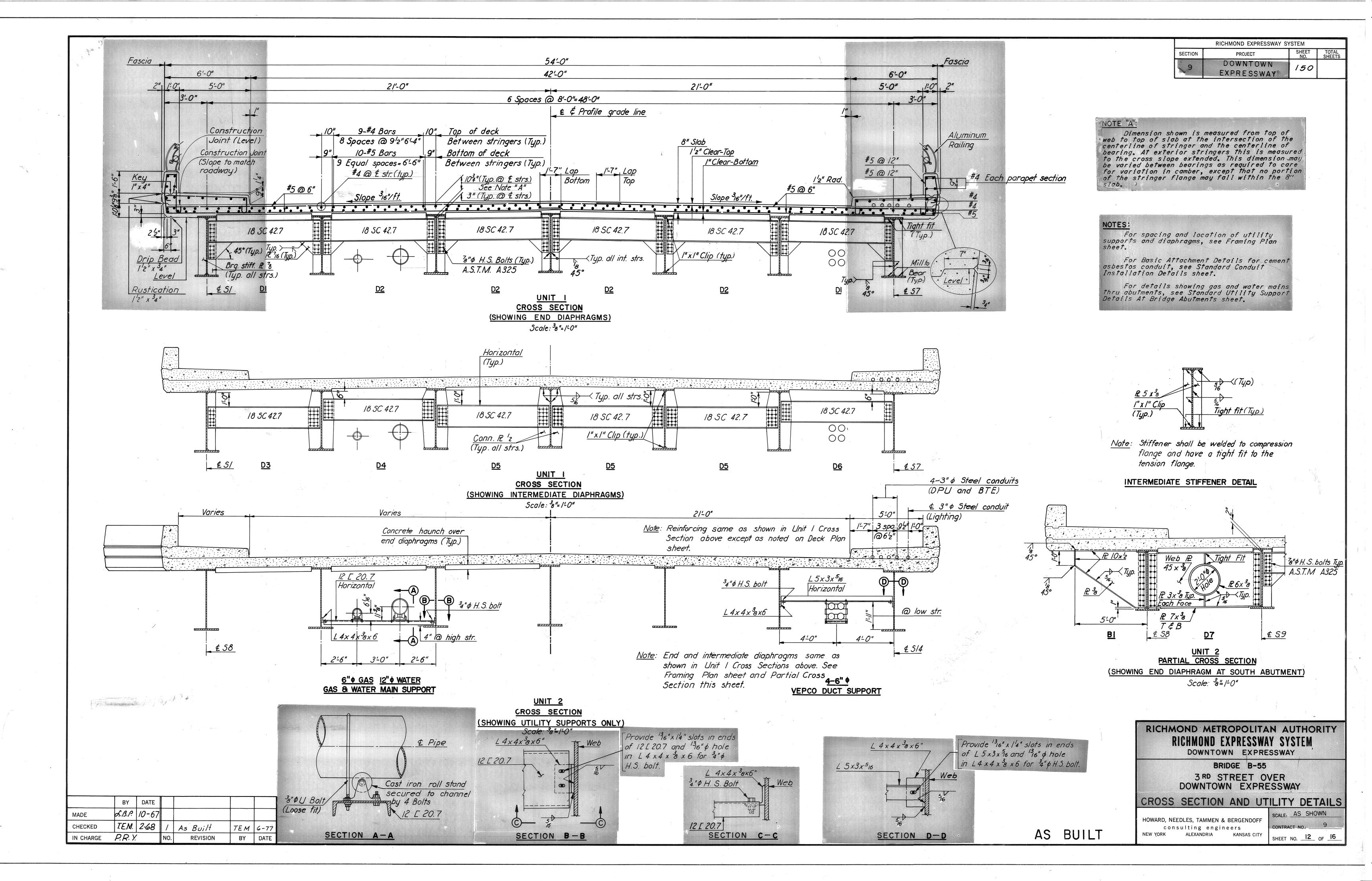


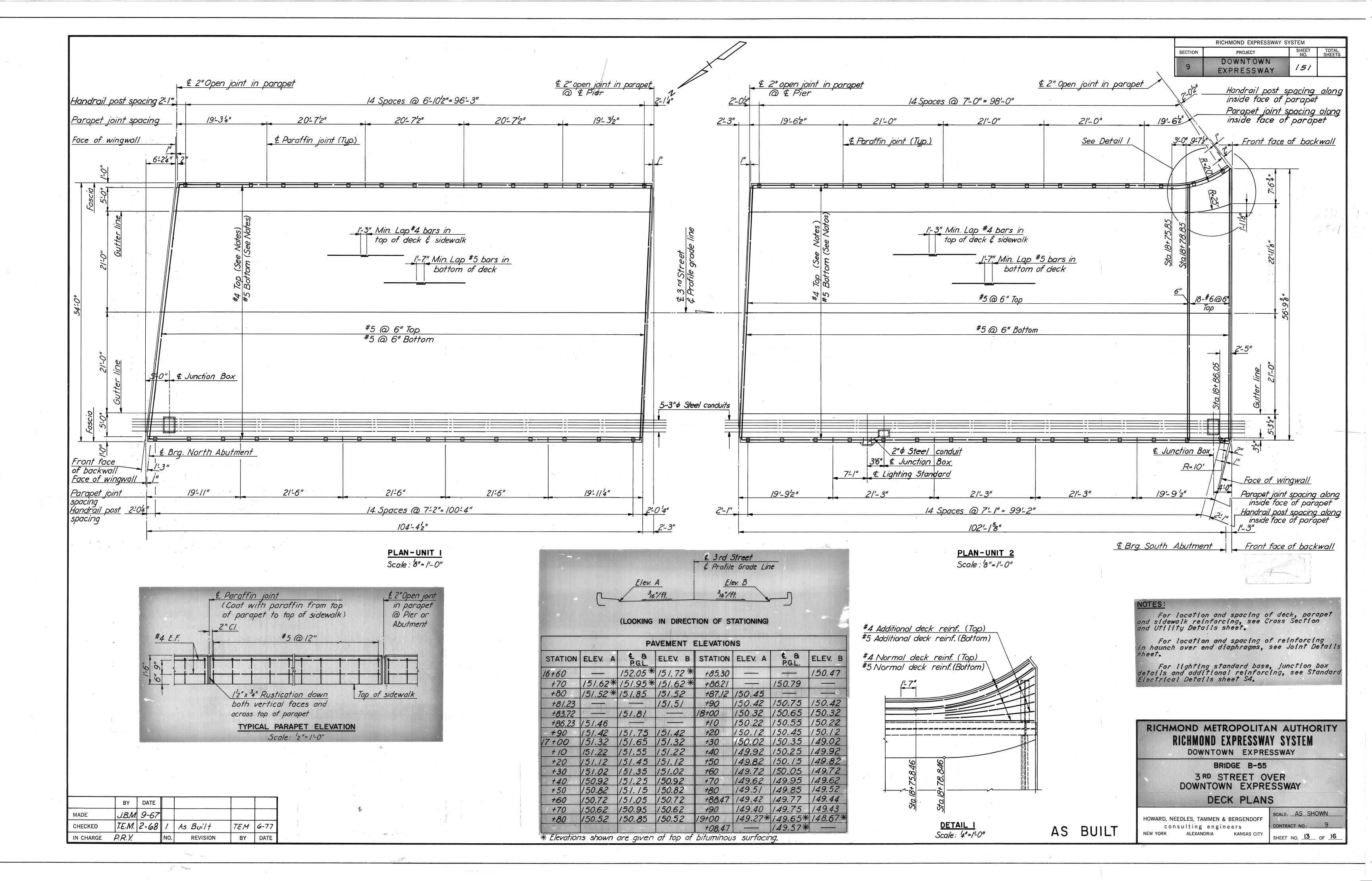


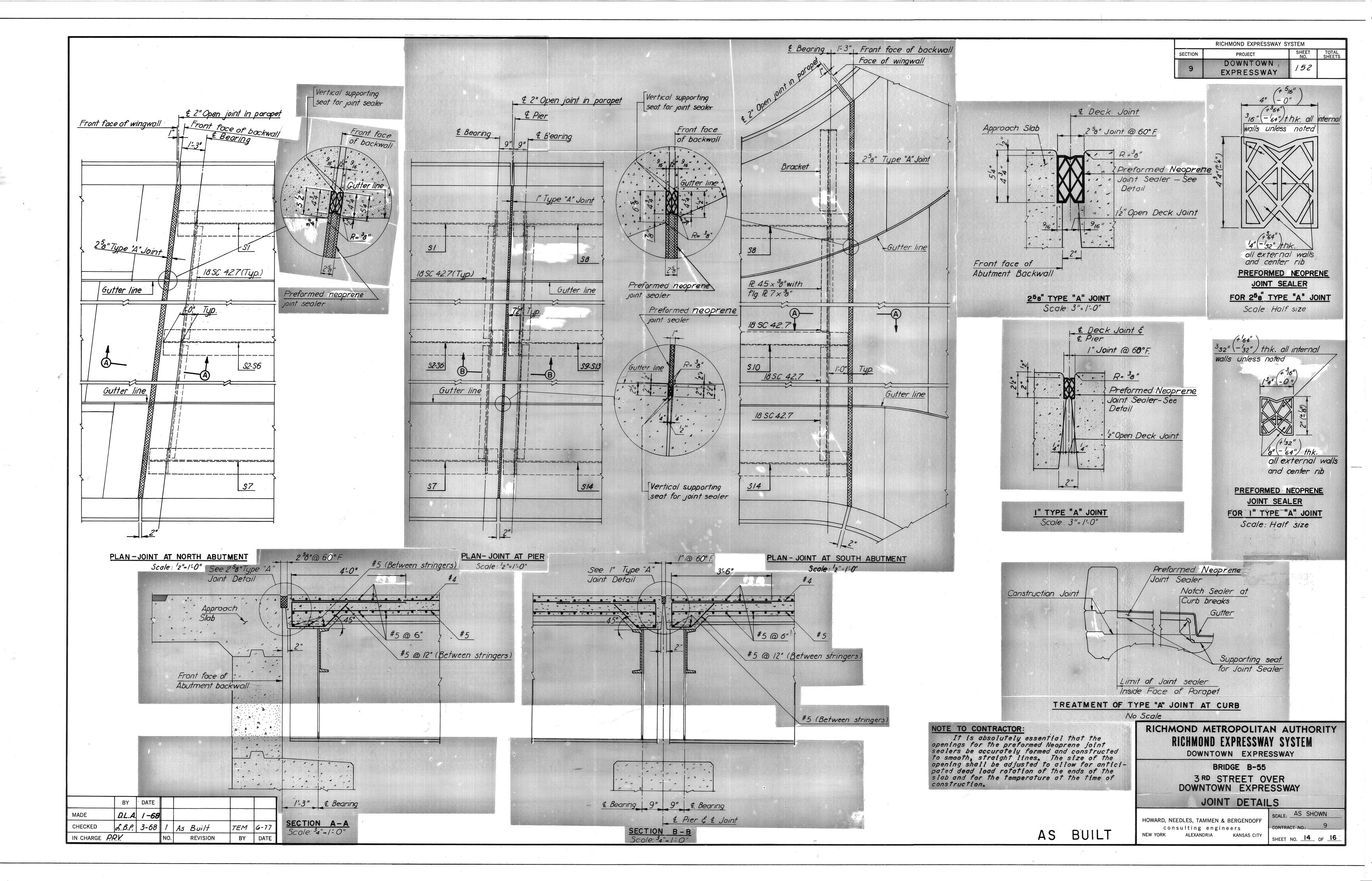
(South 3rd Street Over Downtown Expressway {Rte. 195})

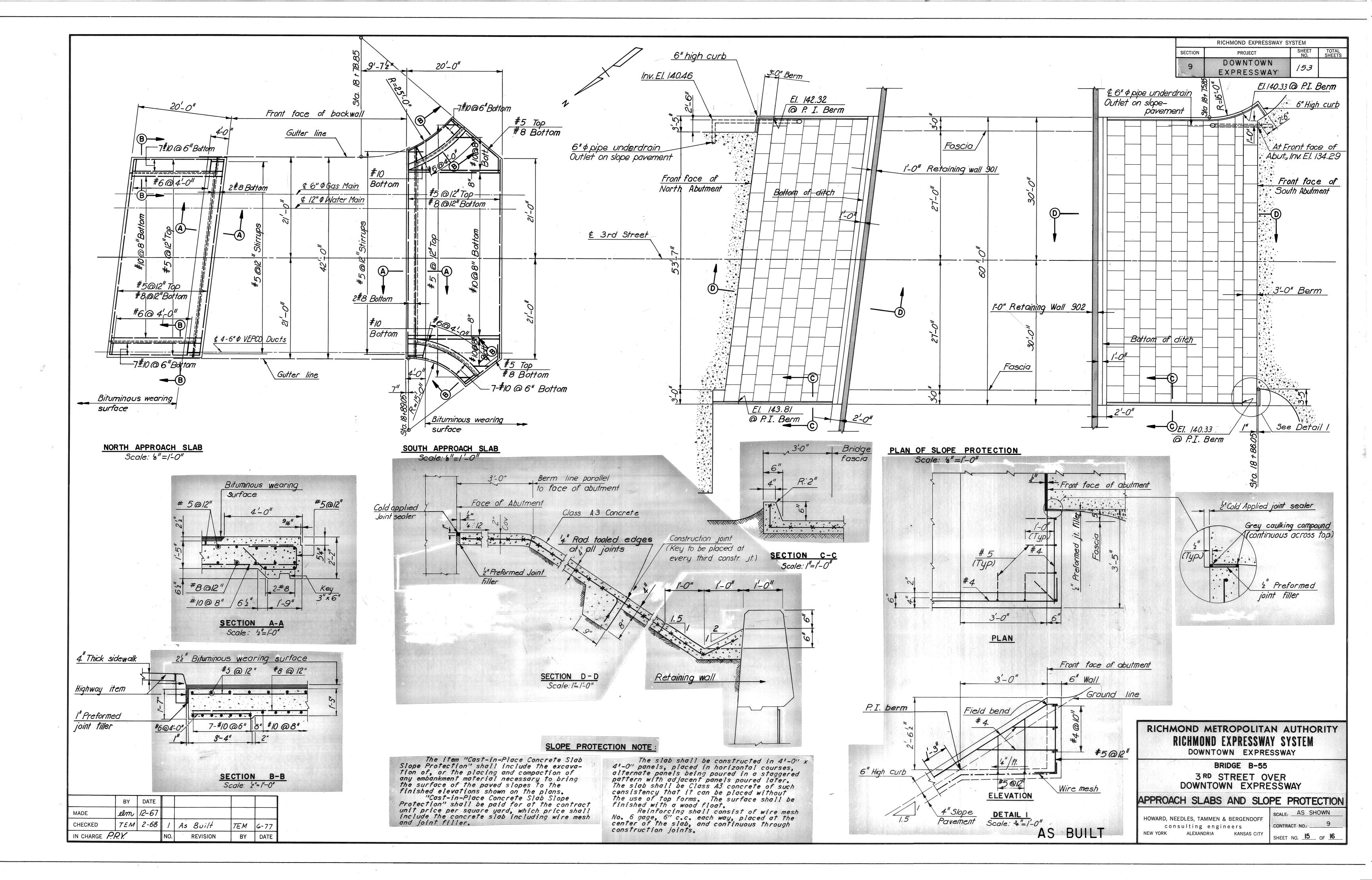




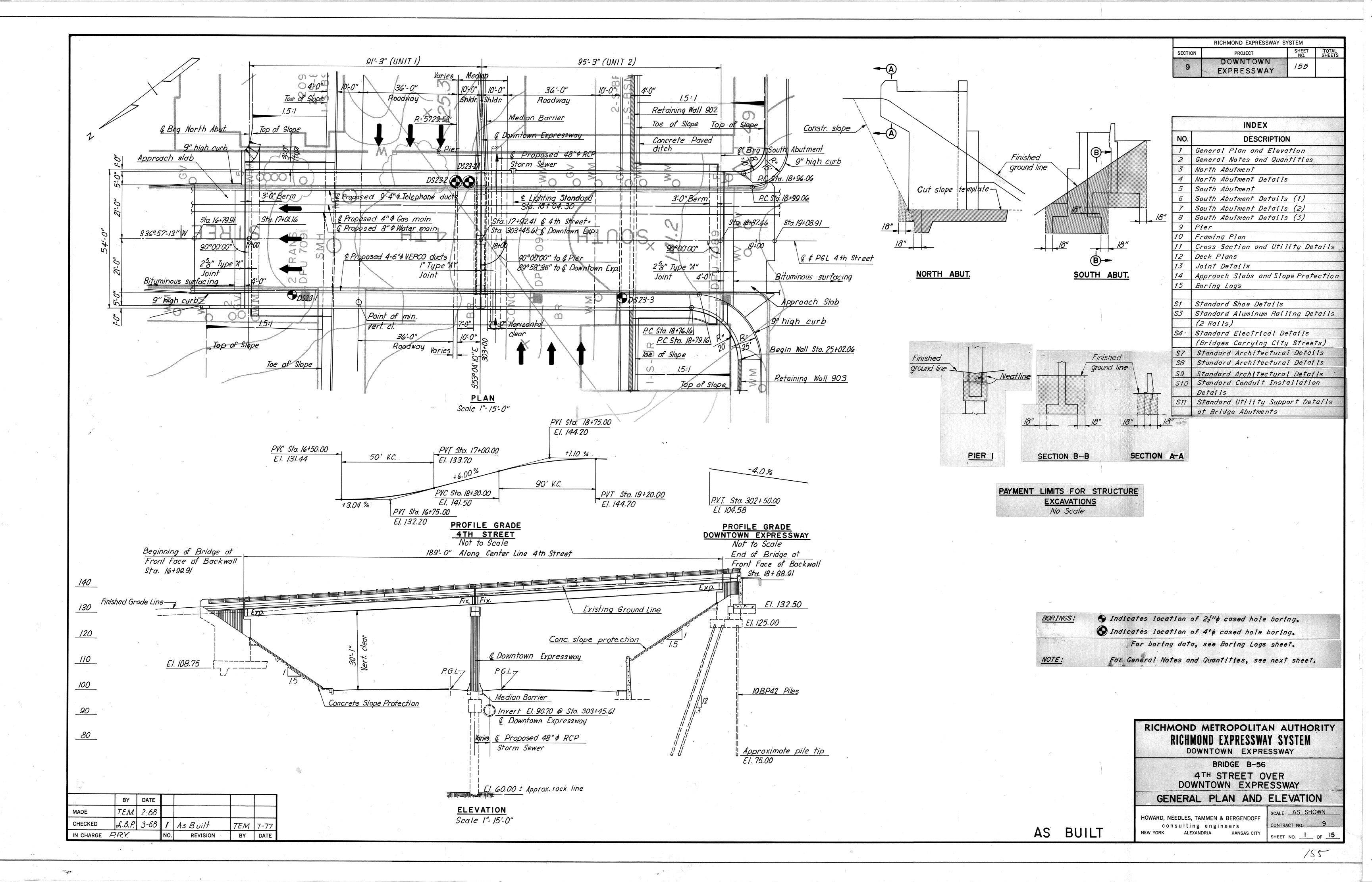


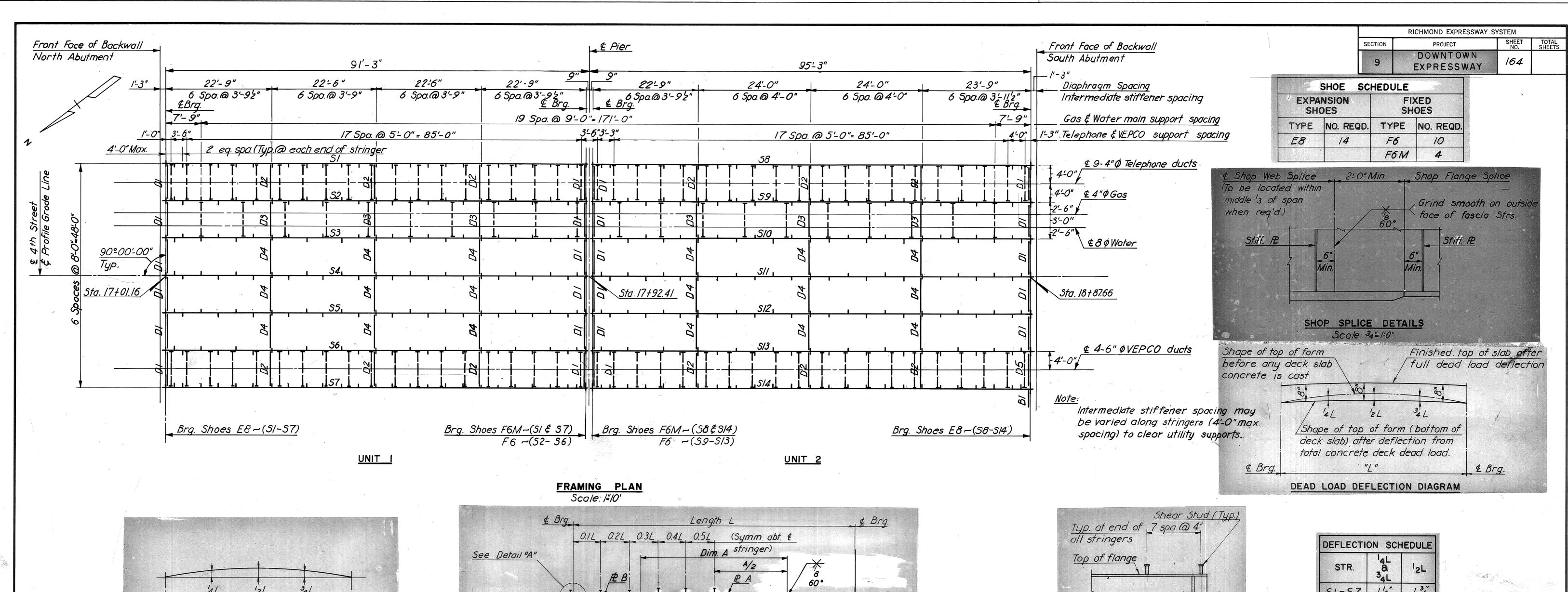


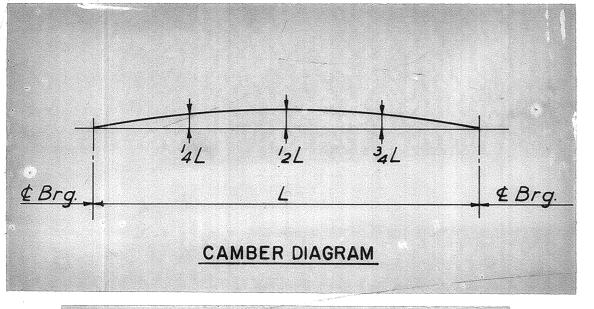




(South 4th Street Over Downtown Expressway {Rte. 195})







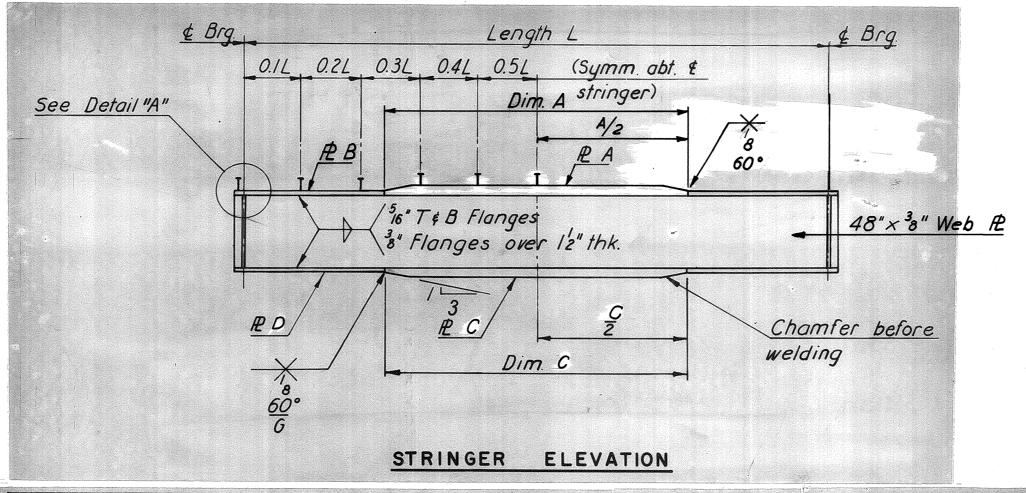
		CAMI	BER	SCHE	DUL	Ξ	
STR.	4L	^l 2L	34 L	STR.	4 L	1 ₂ L	34L
51,52	15"	21,11	, 5 11	58	4/2"	75/8"	68"
54-57	1 8	2 4	18	58 511,512	42	18	00
53	158"	238"		A PARKET MATERIAL STREET, STRE	COMPARED TO SECURE A SECURE OF SECURITIES.	7/2"	6/2"
	·			59,510 513,514	42	12	08

NOTE TO FABRICATOR:

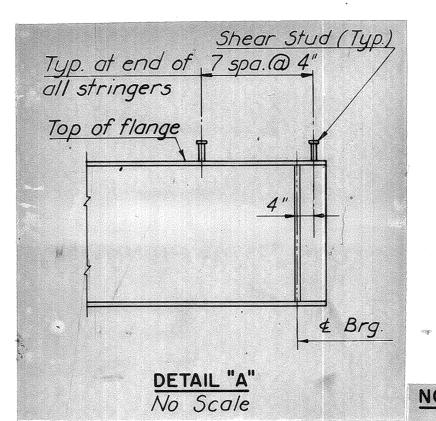
The above stringers shall be fabricated with an upward camber amounting to the tabulated value.

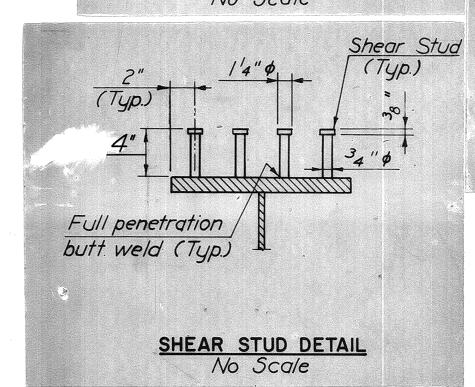
ure		will	provide	approximo	te compe	nsation
for	defle	ction	under	full dead	load and	for
				hed grade.		
	Dimen	sions	ore in	inches.		

THE RESIDENCE OF THE PERSON NAMED IN COLUMN 2 IS NOT THE PERSON NA	angenous management and a second	COLUMN TO THE PARTY OF THE PART				
	ВҮ	DATE				à
MADE	EVR	10-20				ŕ
CHECKED	T.E.M.	2-68	1	As Built	TEM	7-77
IN CHARGE	P.R.	!	NO.	REVISION	BY	DATE



				STRINGER	SCHED	ULE -						
STR.	LENGTH	R A	DIM A	R.B.	B C	DIM C	B D	SHE	AR S	TUD	SPAC	NG
	& BRG. TO & BRG.	r. A	DIW A	ILD /	PL C	DIM C	RD.	0.IL	0.2L	0.3L	0.4L	0.51
51	90':6"	16 x 1	42'-0"	$16 \times \frac{3}{4}$	20×15	55'-0"	20x18	9"	102"	132"	172"	20
s2		16 x1	40'-0"		20x158	53'-0"	20x18	10"	1/2"	132"	/7"	119
S 3		16× 8	34-0"		20x158	53'-0"	20×1/8	10"	1	132%	162"	182
54	a a	1	29'-0"		20x1/2	57'-0"	20×1	9 2"		14"	1	4
S <i>5</i>		· ·	29'-0"	-	20x1/2	57'-0"	20×1	92"	1/2"	14"		I
56		16' × 78	34'-0"		20x 158	53'-0"	20 x 1/8	9 4"	1/"	132"	162"	18
<i>5</i> 7	90' 6"	16 × 1	42'-0"		20x 158	55'-0"	1	9"	102"	13 2"	172	20/2
58	94-6"	16 × 1/8	53:0"		20x /34	62'-0"	1	9"	102"	13'2"		21"
59		16x18	52-0"		20 × /34	6/'-0"		92"	//"	14"		19/2
510		16×18	50'-0"		20x/34	60'-0"		10"	1/2"	1	172"	19/2
S//		16 × 1	44'-0"		20x 158	58'-0"		10"	1/2"		17"	19"
512		16×1	44'-0"		20x 158	58'-0"		10"	112"		17"	19"
S/3		16 × 1/8	50'-0"		20x 134	61'-0"	20×18	92"	//"	14"	17"	192
5/4	94'-6"	16 × 1/8	53'-0"	$16 \times ^34$	20x18	.58'-0".	20x14	9"	10'2"	13/2"	17/2"	2/"





SHEAR STUD NOTE: Capacity = 3,400 lbs. per stud. The contractor may, if he elects, use three "" diameter studs at the same longitudinal spacing in lieu of the four "diameter" studs shown. Stud rows shall be placed parallel to the main deck reinforcing. Shear stud spacing shown is maximum

BUILT

DEFLECTION	ON SCH	EDULE	
STR.	¹ 4L 8 3 ₄ L	l ₂ L	
51-57	14"	13"	
S8,511,512	18"	2"	
59, S10, S13, S14	13.	13"	

The above deflections are those anticipated to occur in the stringer upon placement of the shear Stud total concrete deck dead load.

(Typ.)

In practice, the stringer In practice, the stringers in place are not likely to have the exact camber to compensate for these deflections during construction. The residual amounts shall be provided for by adjusting forms to vary the thickness of the concrete bolster between the bottom of the slab and the top of stringer, without alteration of the slab thickness.

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

DOWNTOWN EXPRESSWAY

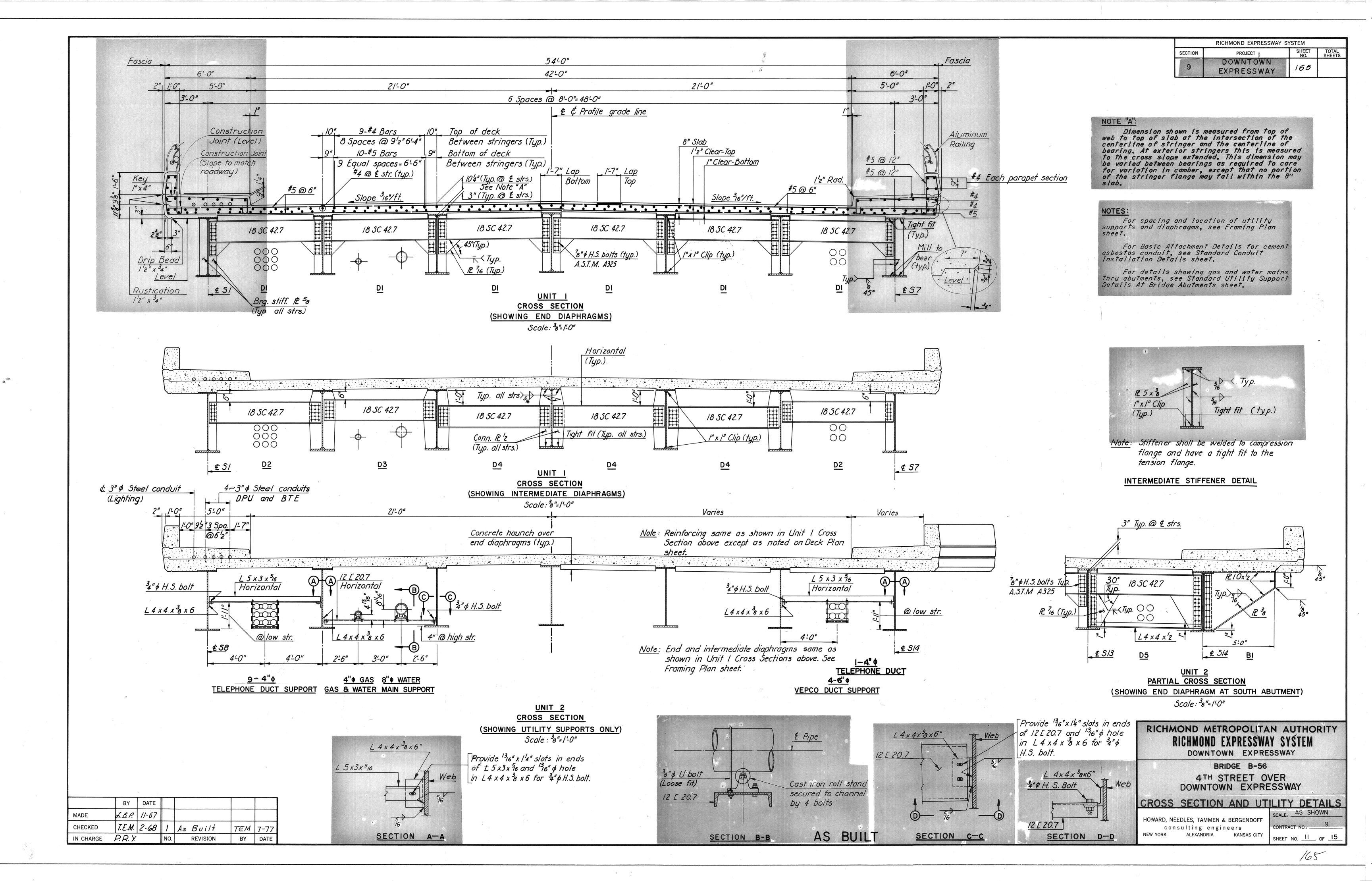
BRIDGE B-56 4TH STREET OVER DOWNTOWN EXPRESSWAY

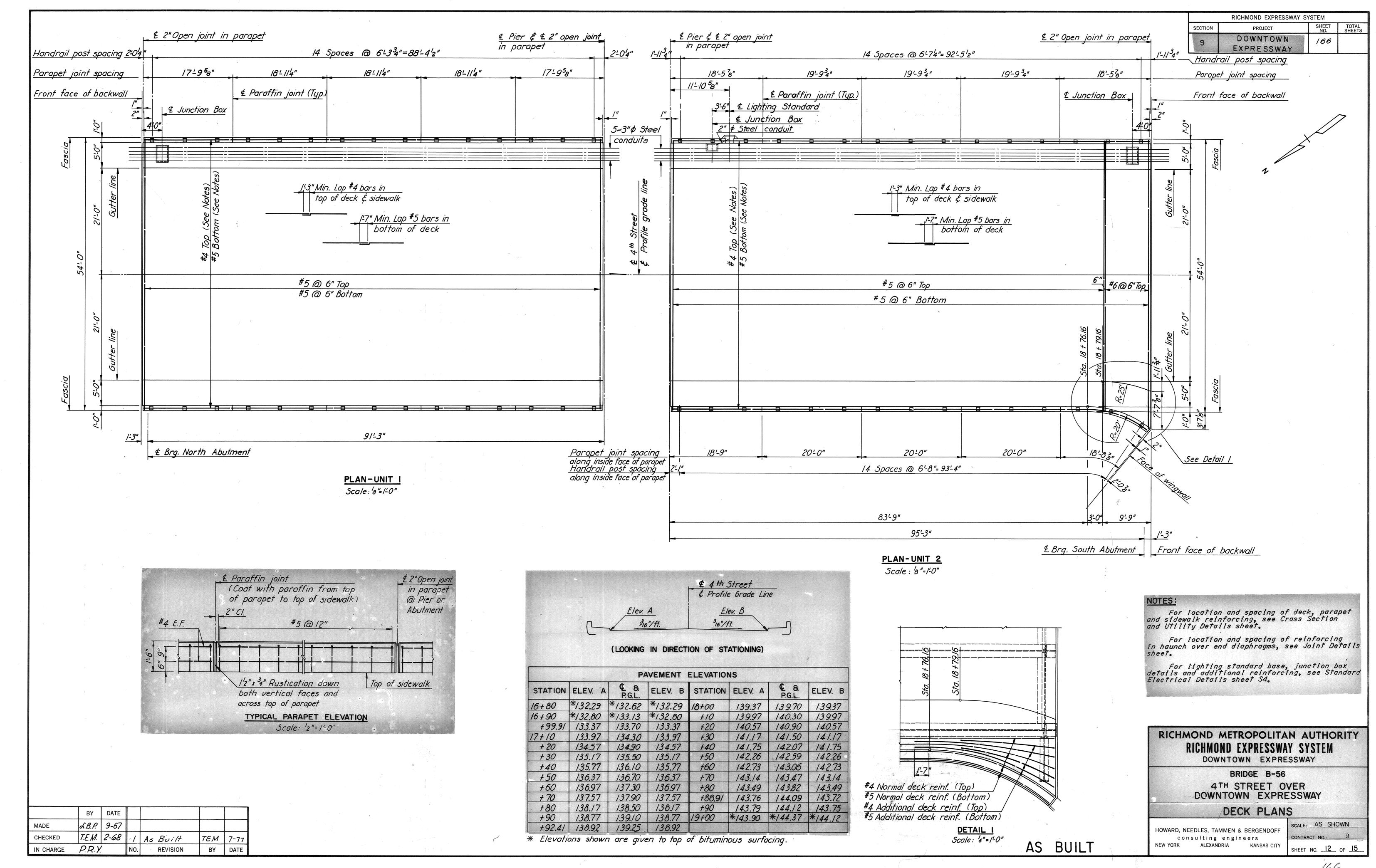
FRAMING PLAN

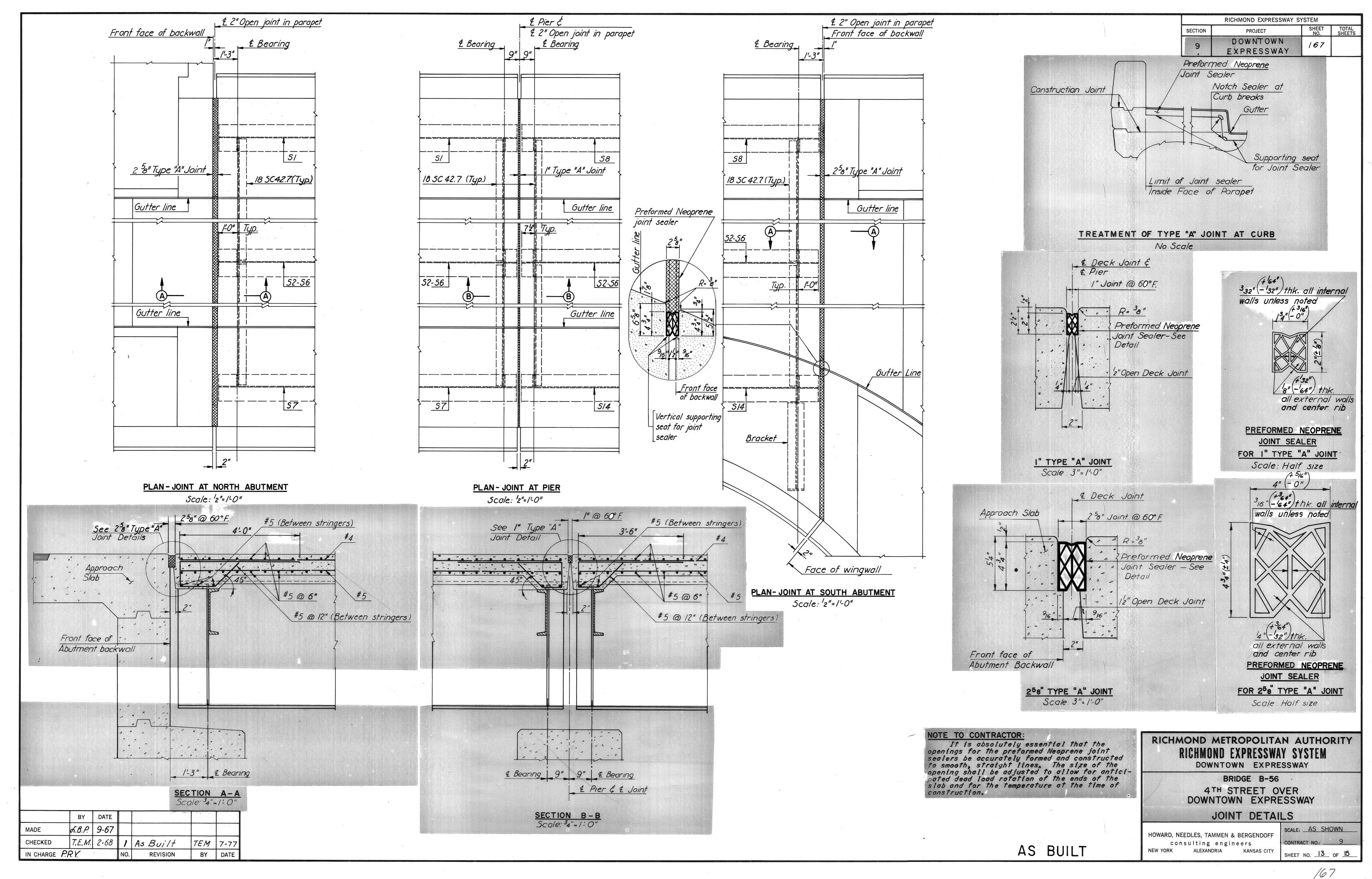
HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers NEW YORK ALEXANDRIA KANSAS CITY

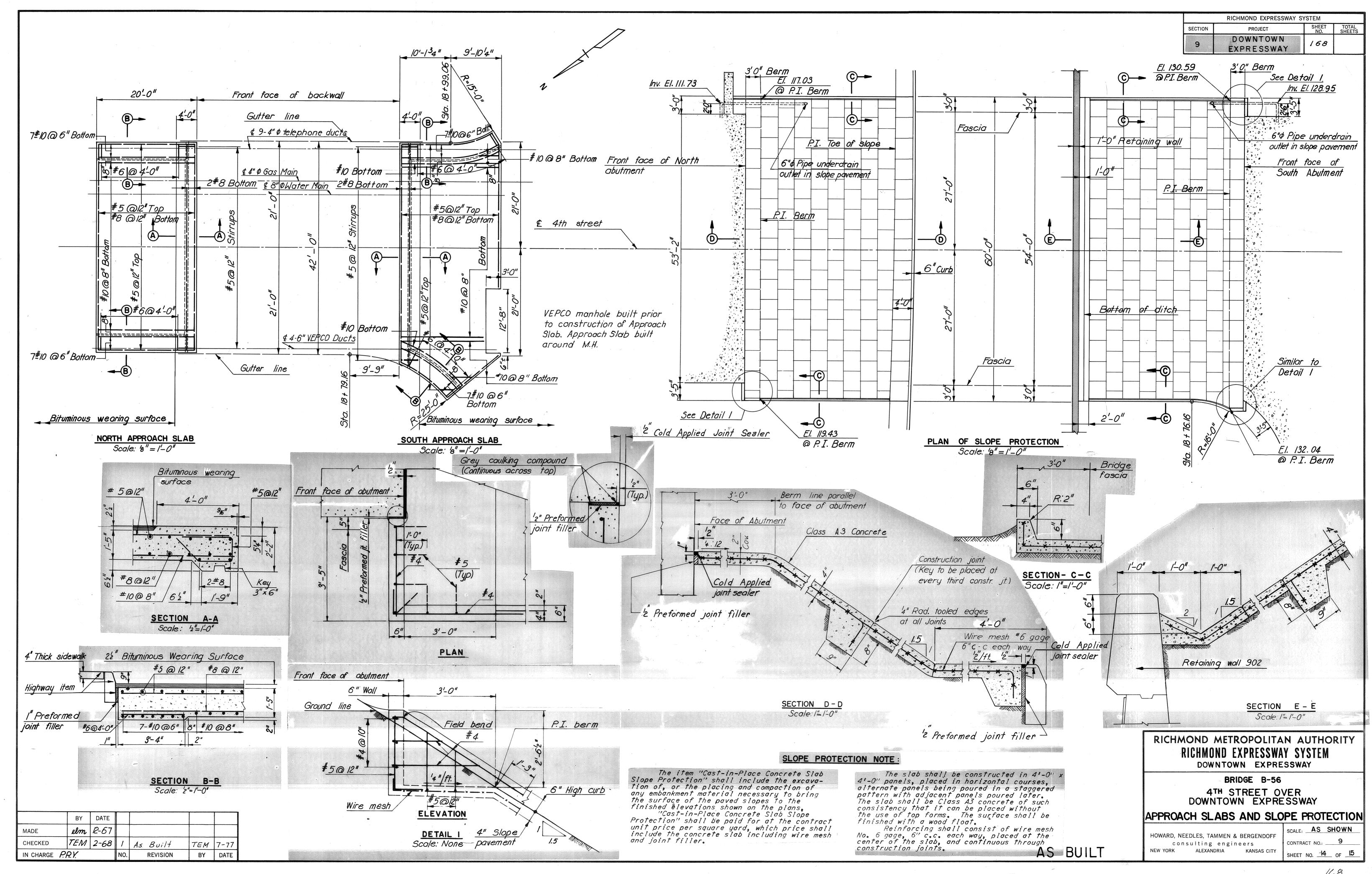
SCALE: AS SHOWN

SHEET NO. 10 OF 15

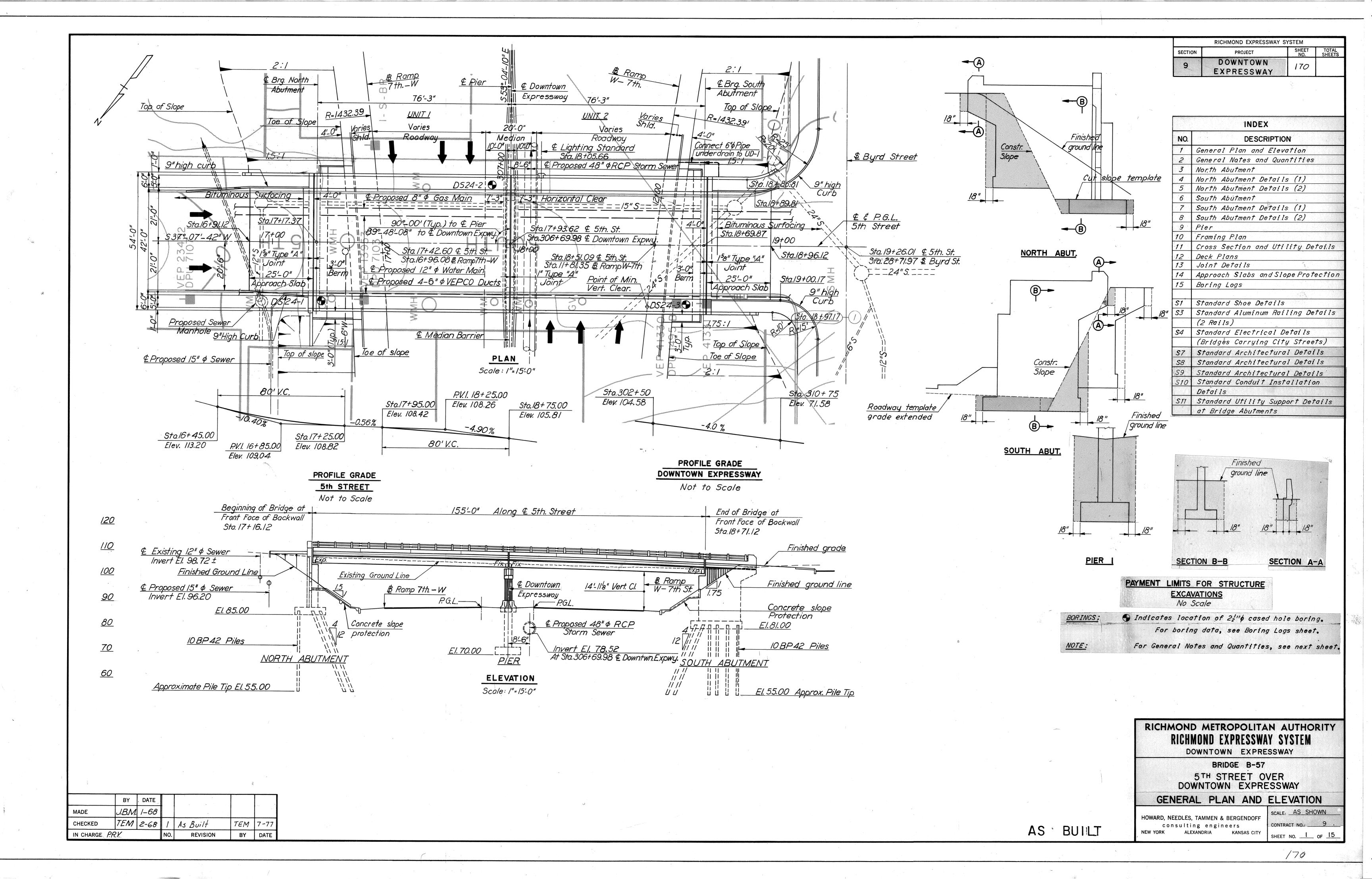


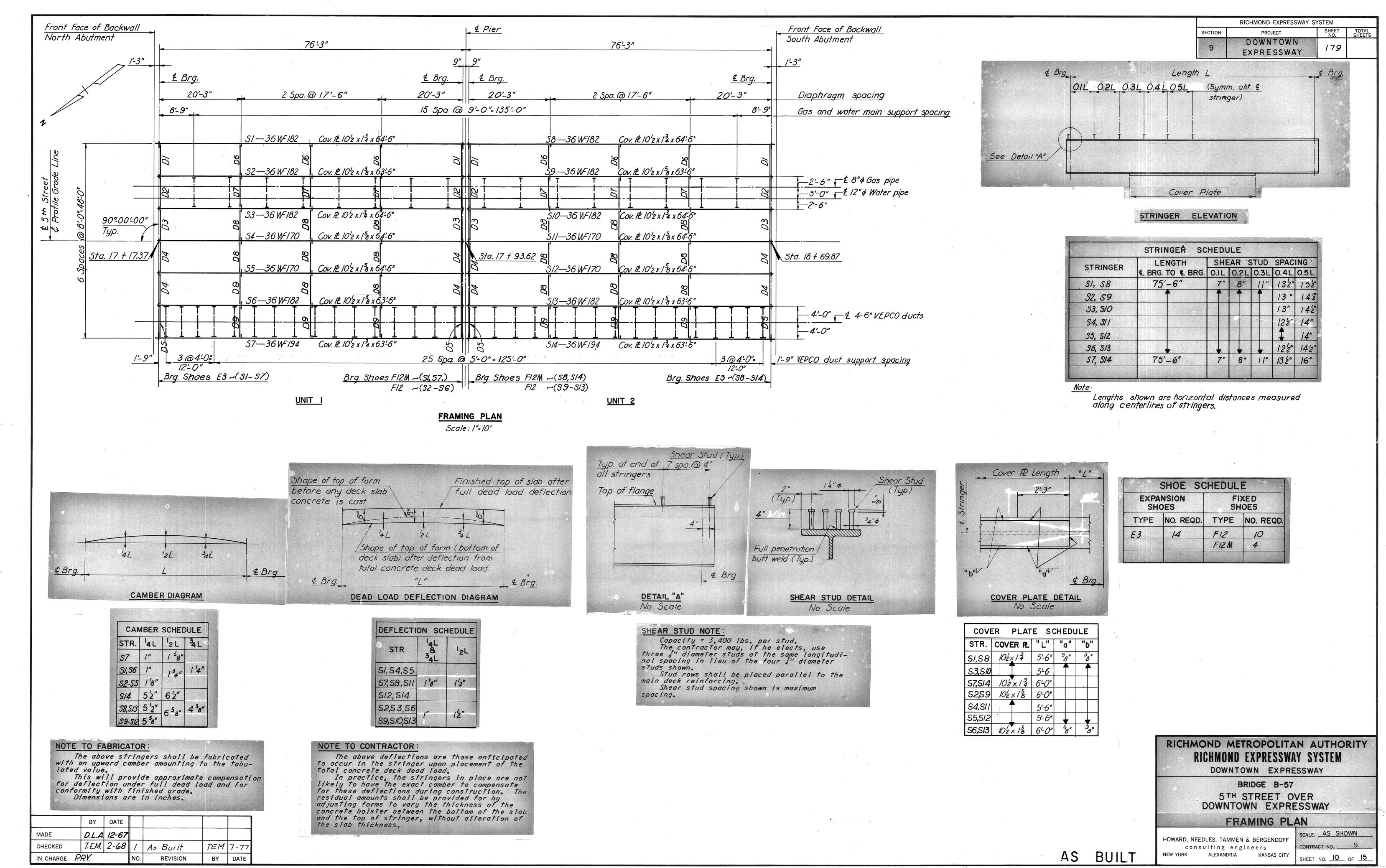


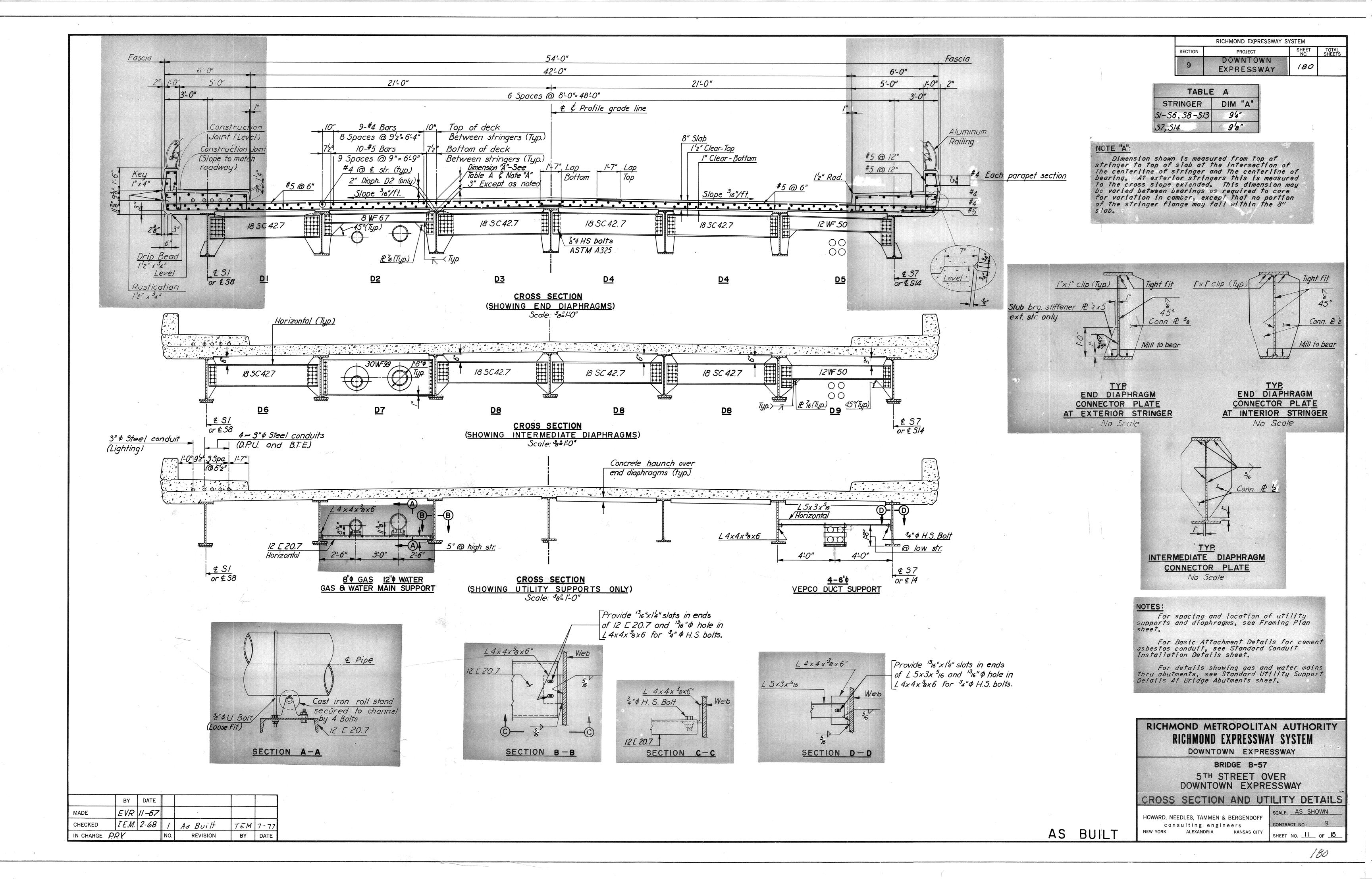


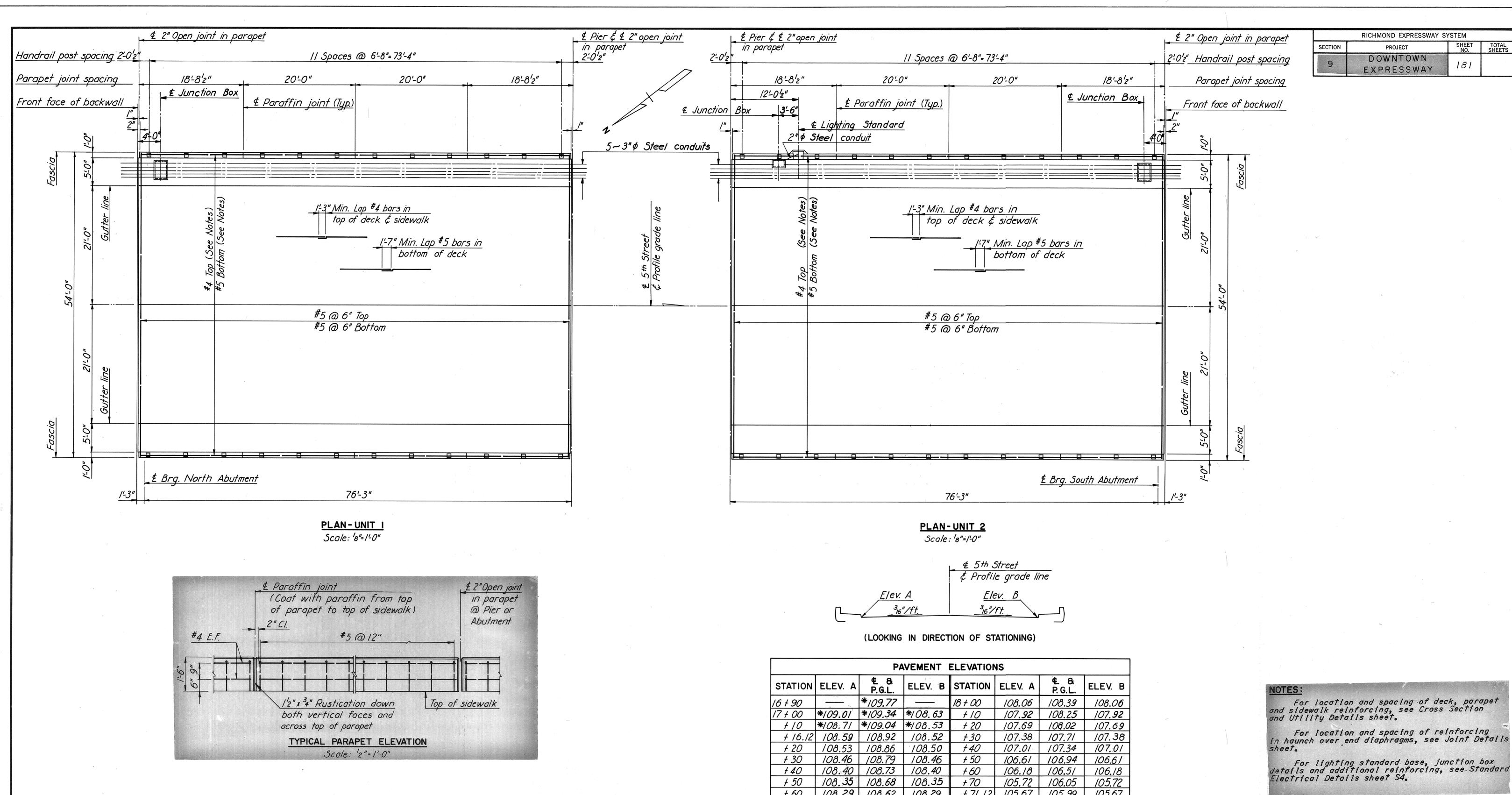


(South 5th Street Over Downtown Expressway {Rte. 195})









BY DATE

T.E.M 2-68

As Built

REVISION

TEM 1-77

BY DATE

S.B.P. 9-67

CHECKED

IN CHARGE PRV

108.29 108.62 108.29 105.67 | 105.99 +71.12 105.67 **105.23* **105.56* * *105.25* 108.23 | 108.56 | 108.23 *104.64 *105.07 * 104.72 108.18 | 108.51 | 108.18 +90 108.12 | 108.45 | 108.12 | 19 + 00 * 104.58 + 93.62 | 108.10 | 108.4**3** | 108.10

* Elevations shown are given to top of bituminous surfacing.

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

DOWNTOWN EXPRESSWAY

BRIDGE B-57 5TH STREET OVER DOWNTOWN EXPRESSWAY

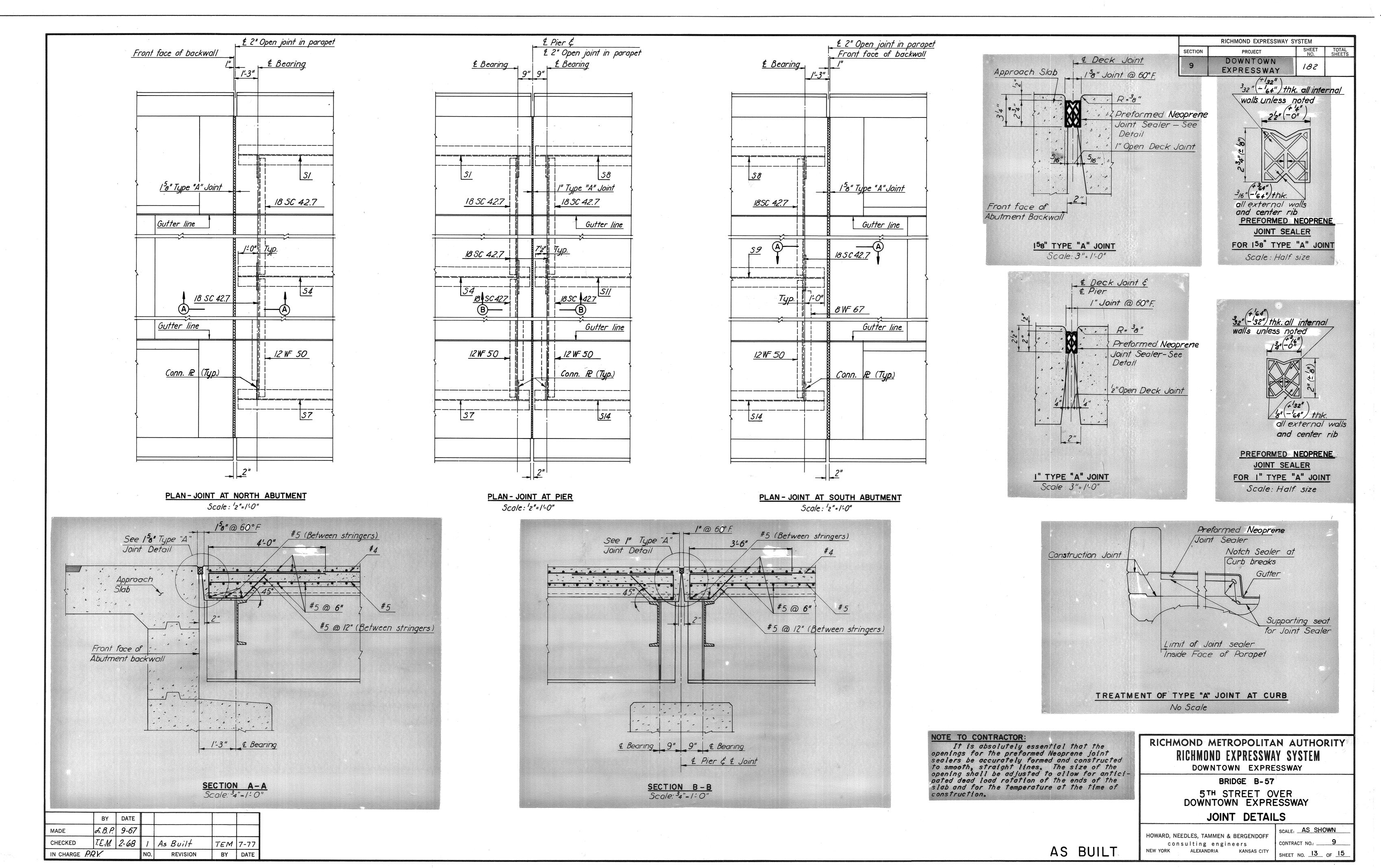
DECK PLANS

HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers

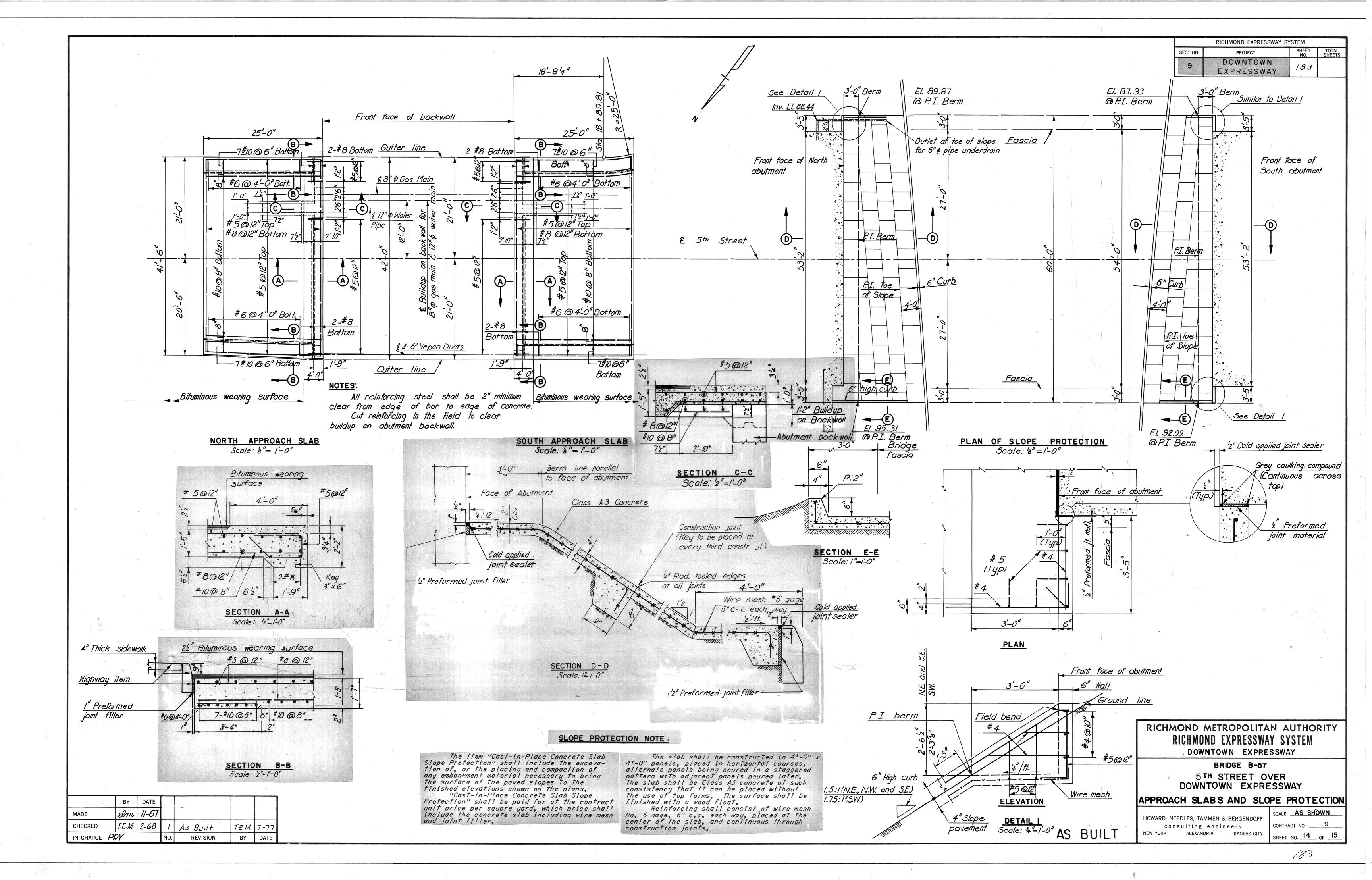
BUIL NEW YORK

AS

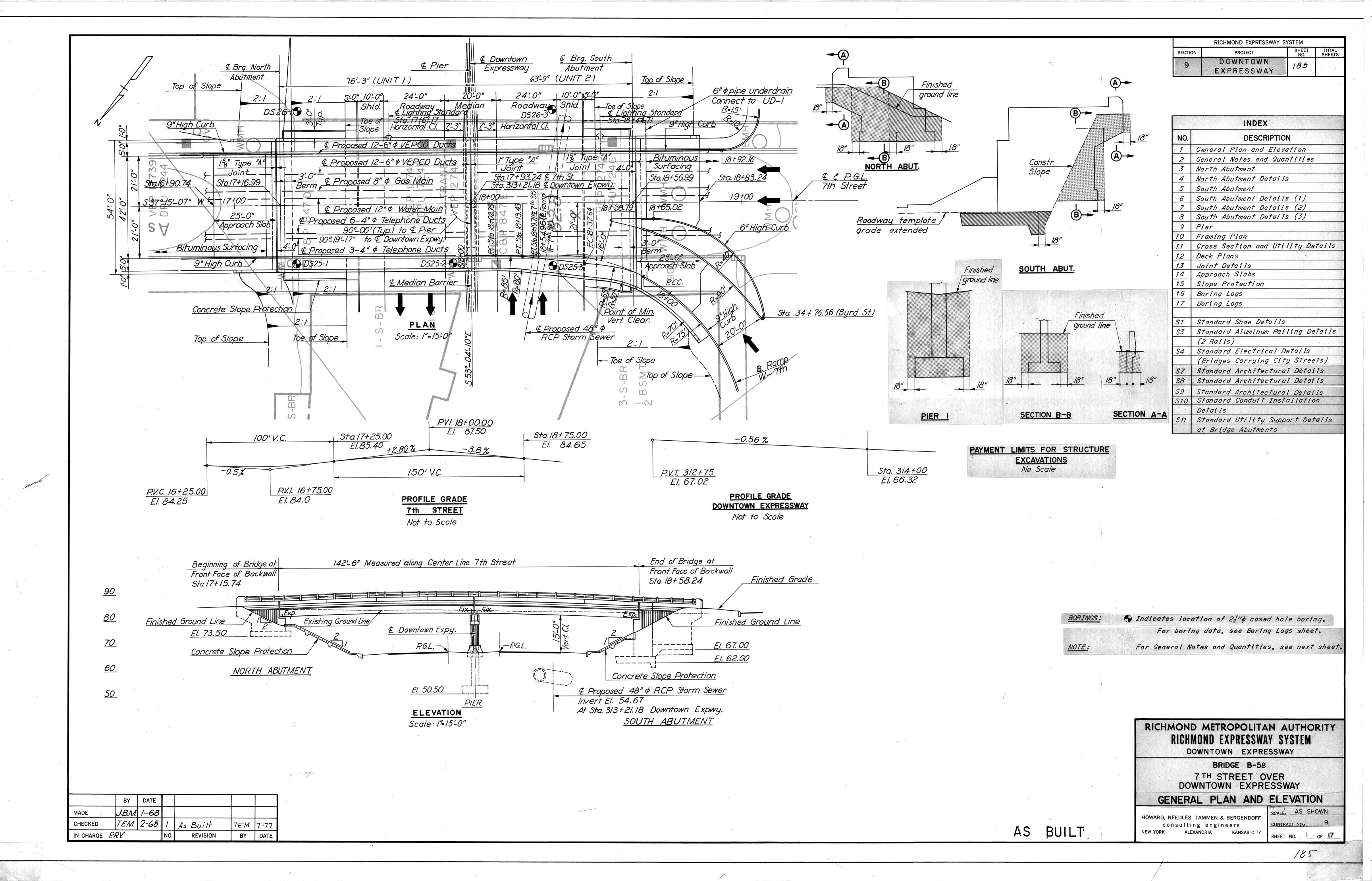
SCALE: AS SHOWN ALEXANDRIA KANSAS CITY SHEET NO. 12 OF 15

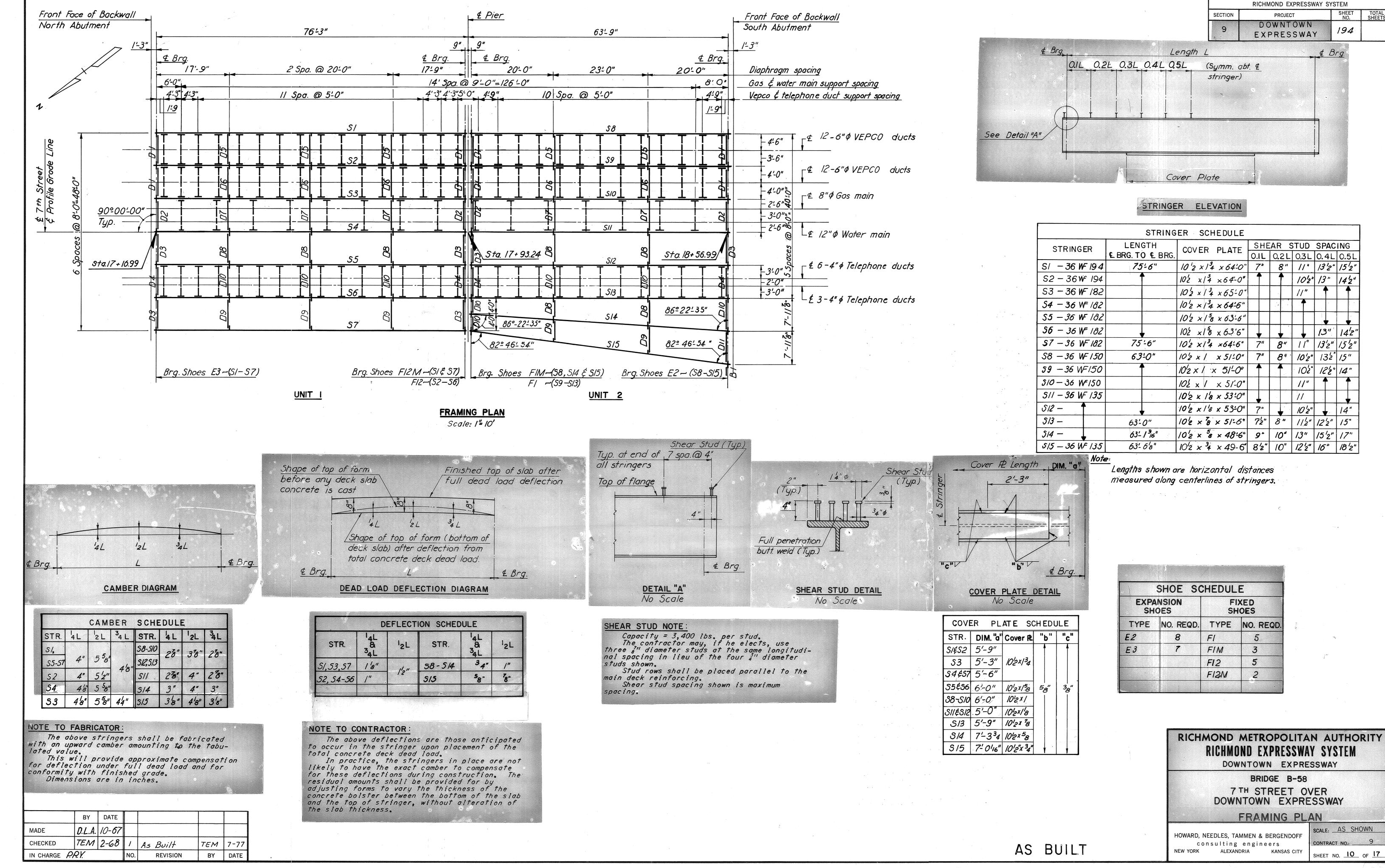


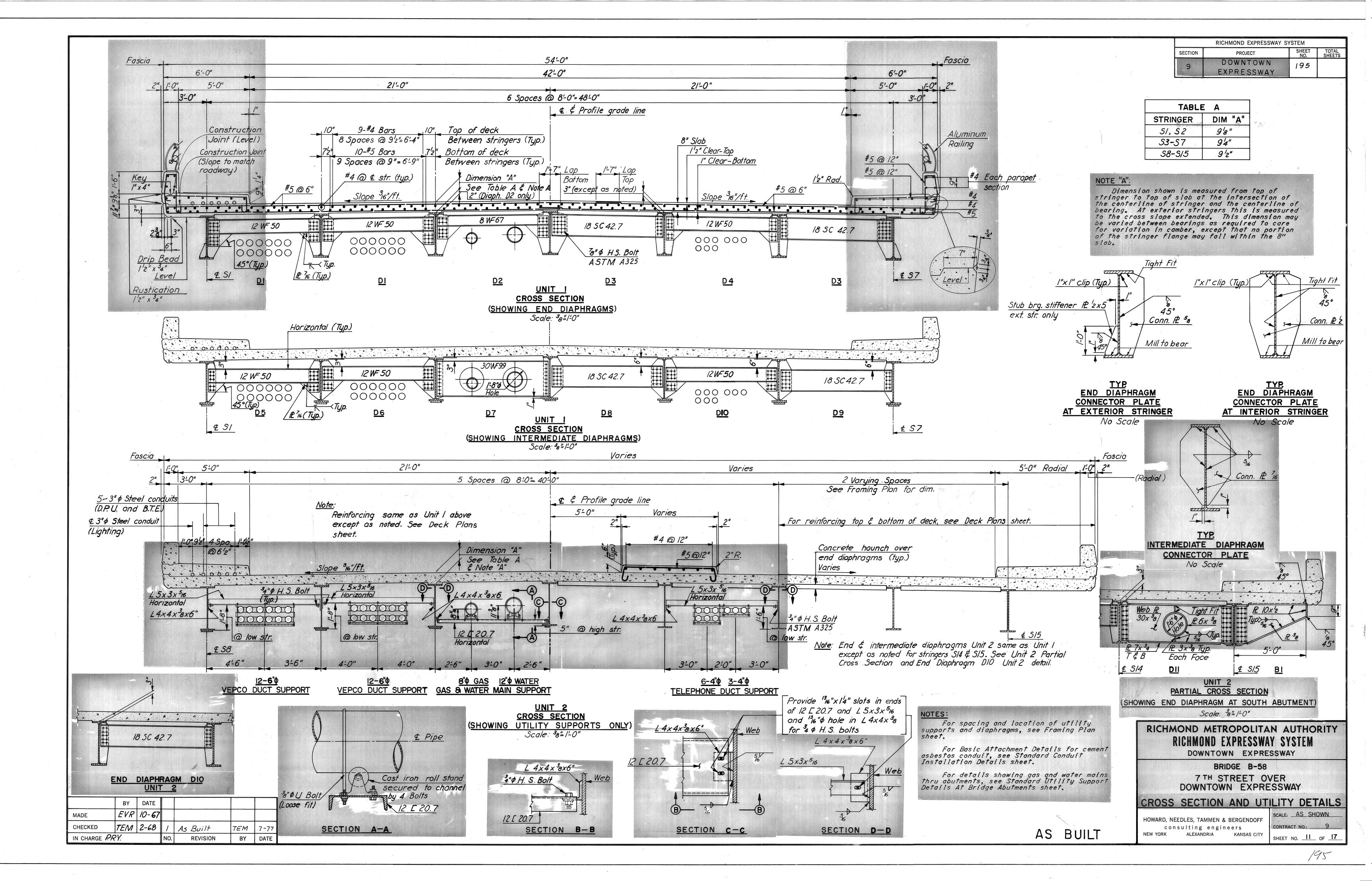
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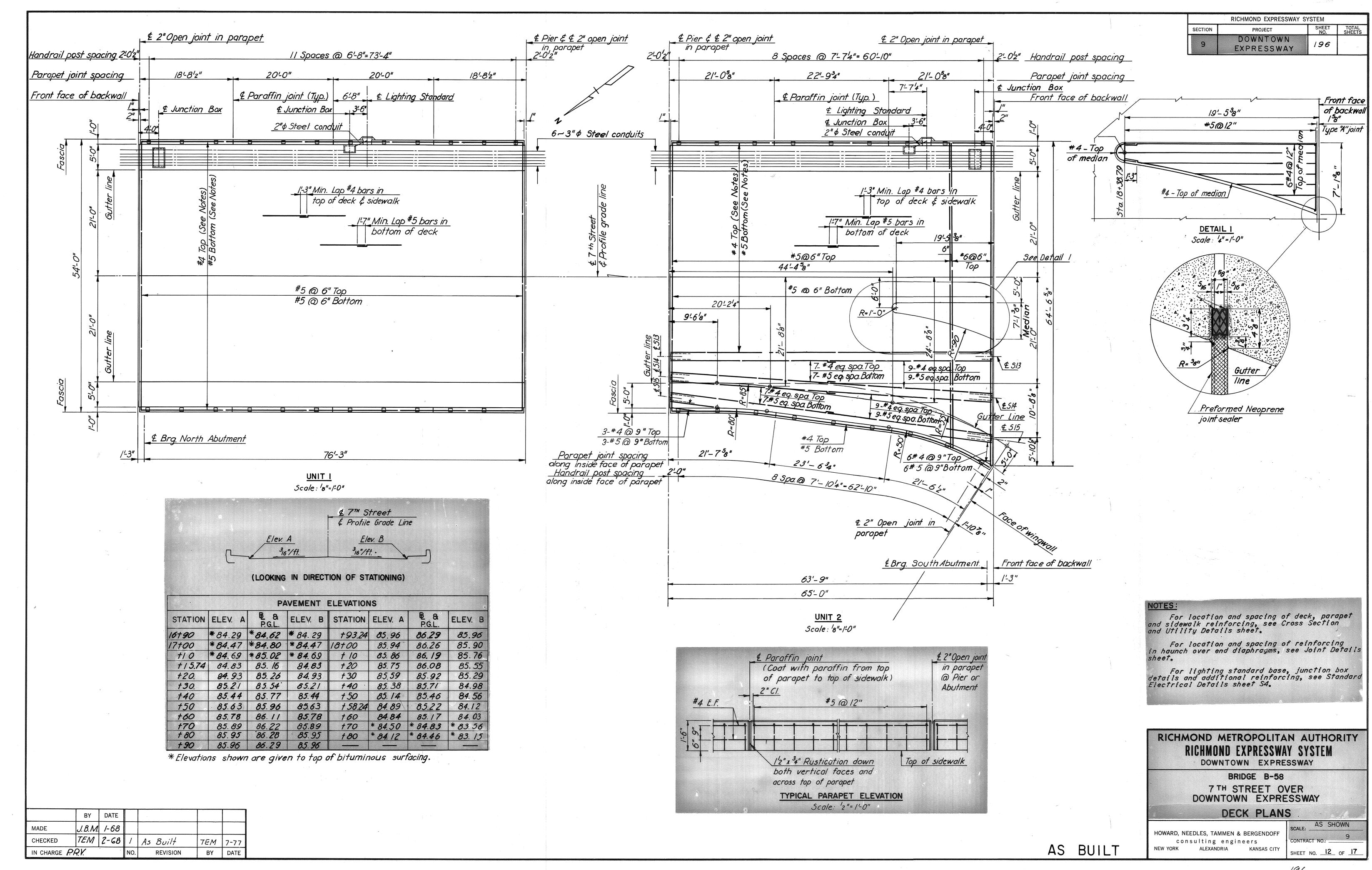


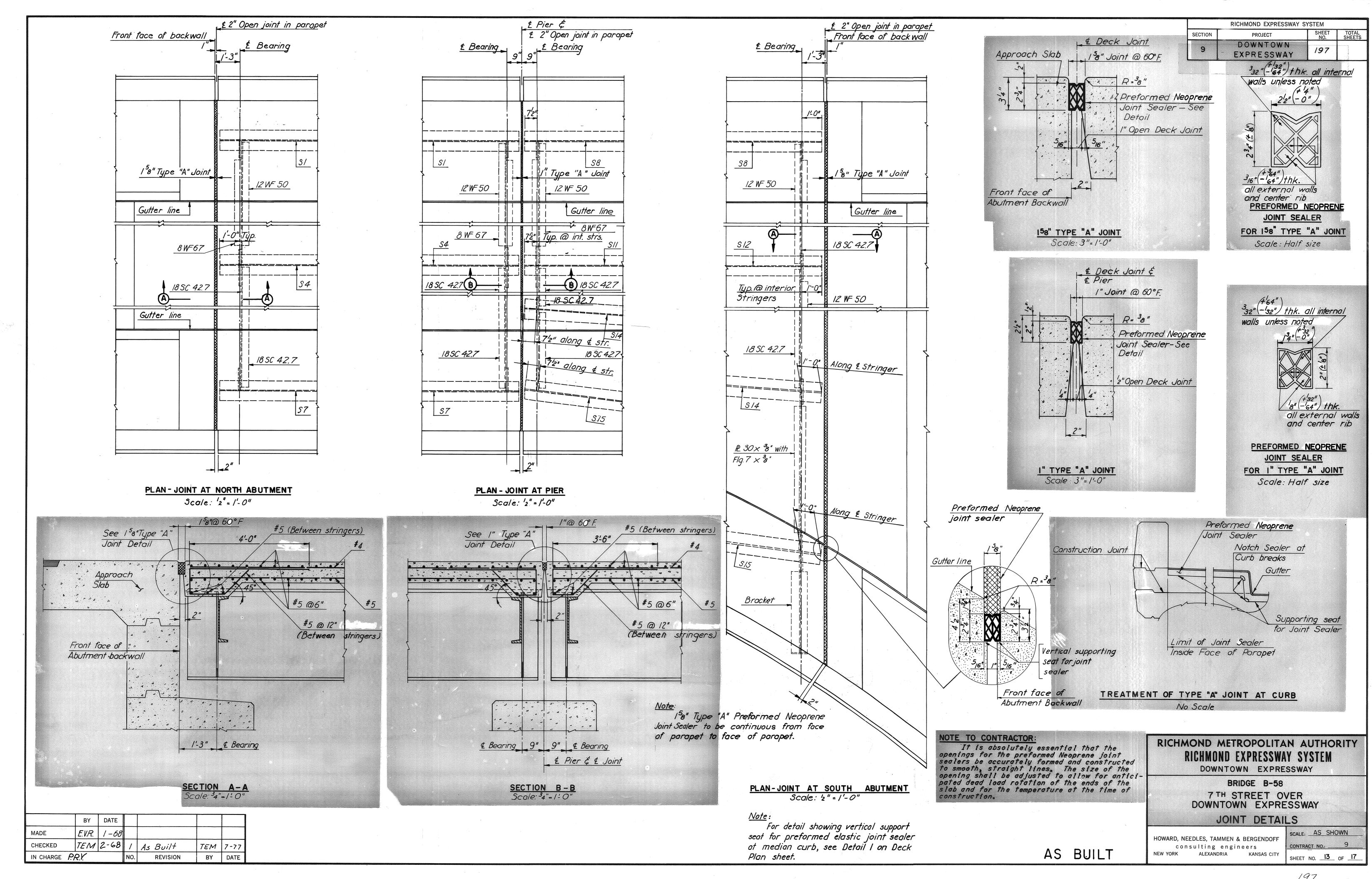
(South 7th Street Over Downtown Expressway {Rte. 195})

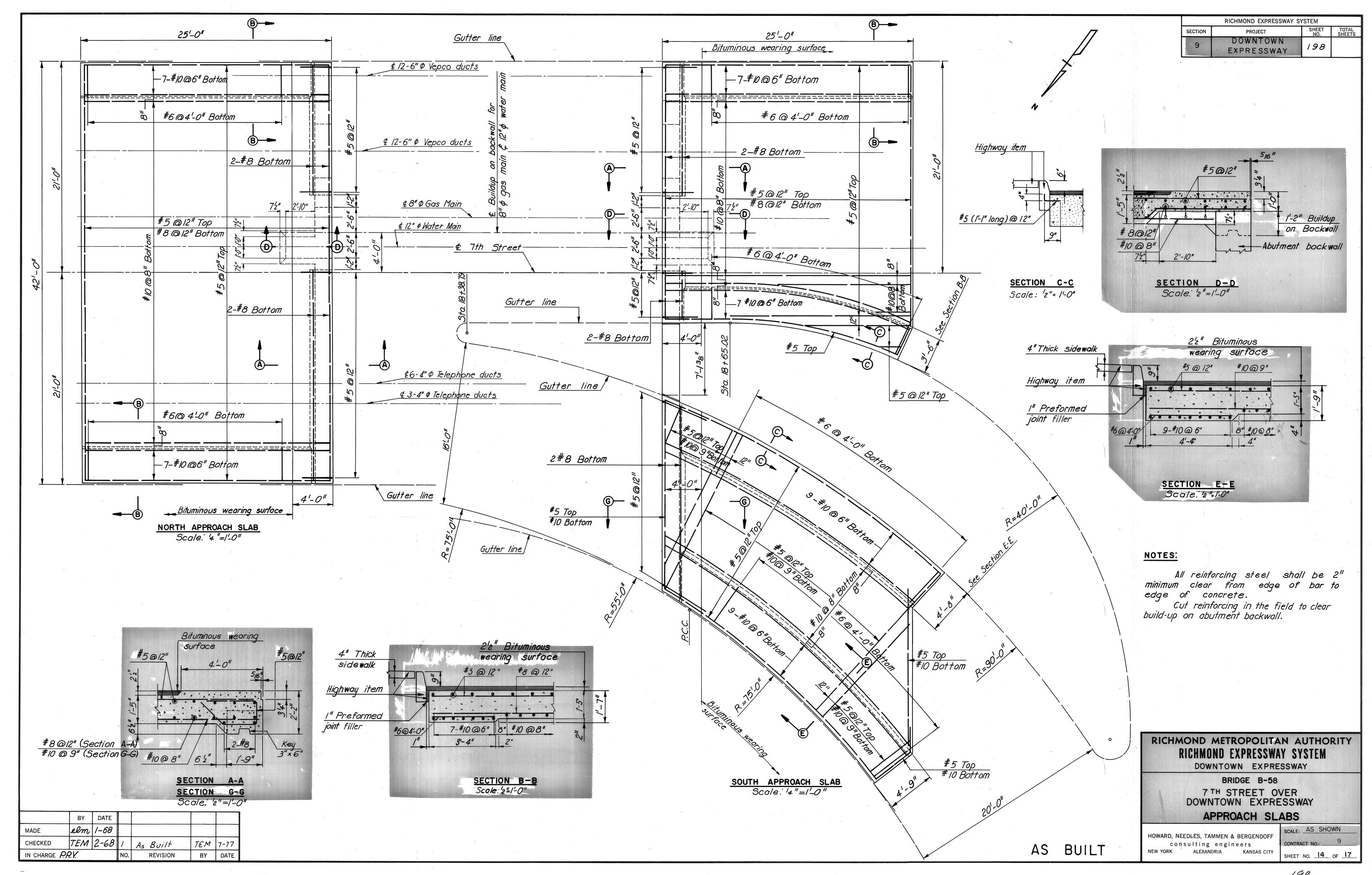




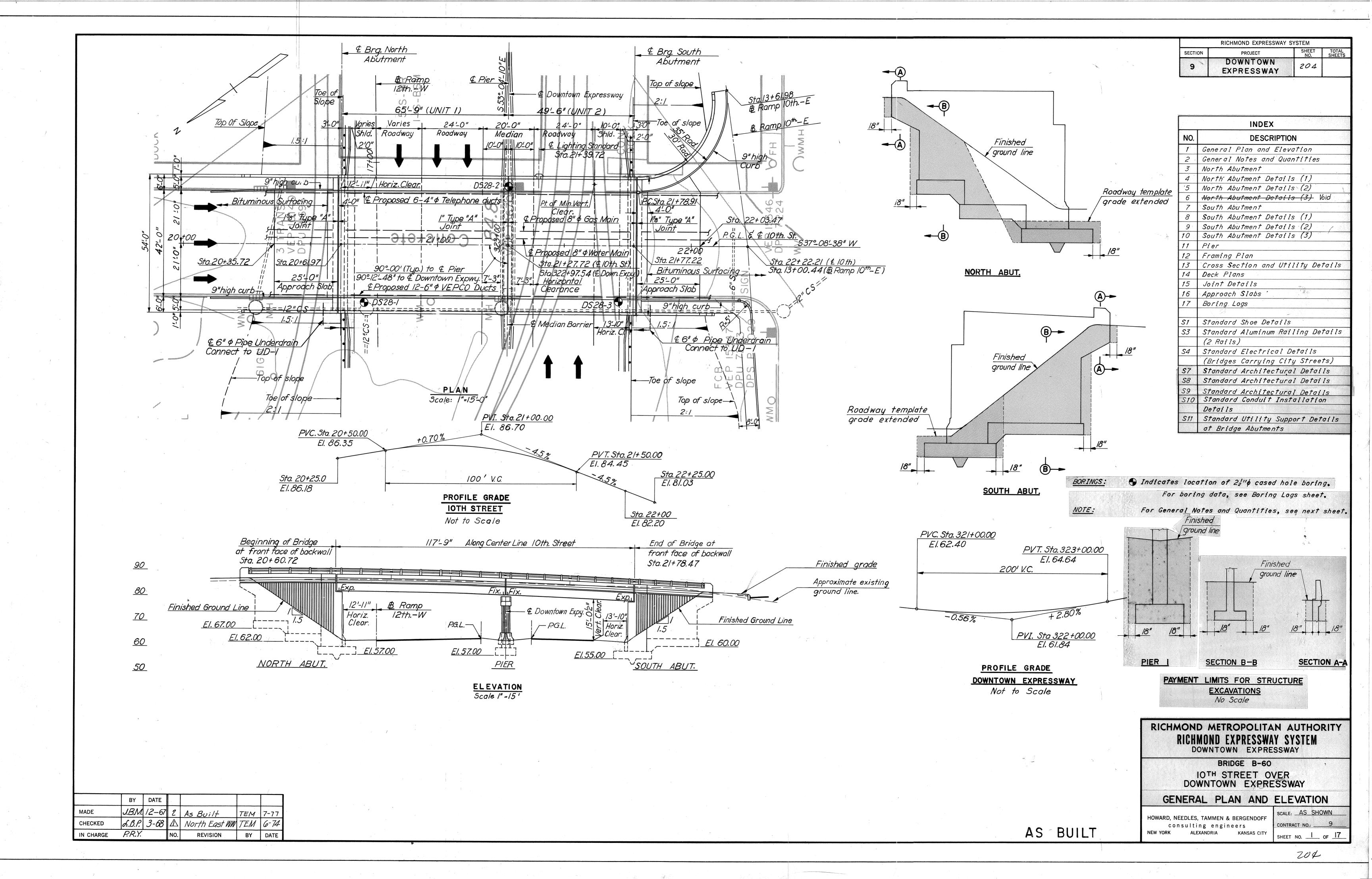


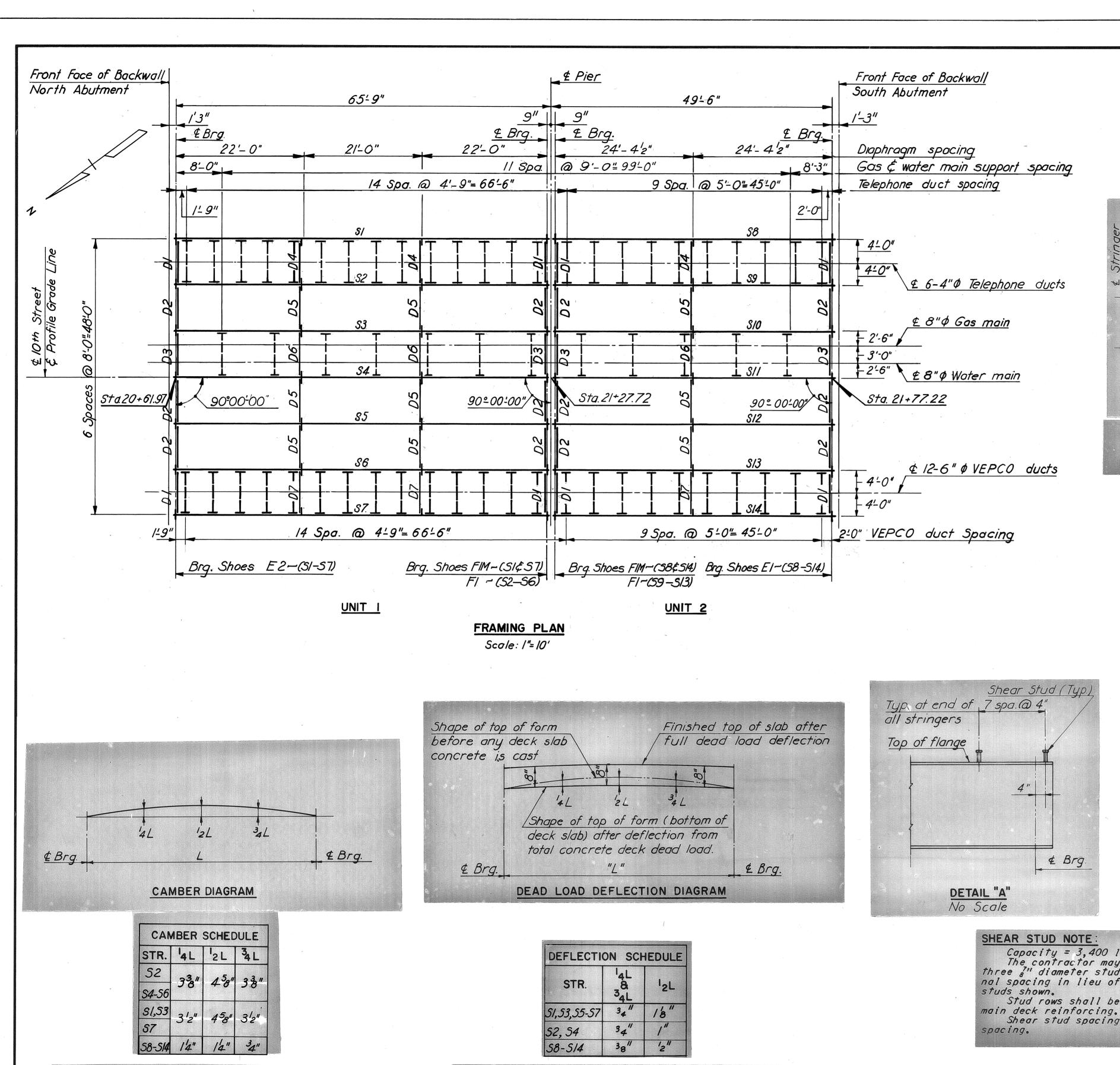






(South 10th Street Over Downtown Expressway {Rte. 195})





NOTE TO CONTRACTOR

The above deflections are those anticipated to occur in the stringer upon placement of the total concrete deck dead load.

In practice, the stringers in place are not likely to have the exact camber to compensate for these deflections during construction. The residual amounts shall be provided for by adjusting forms to vary the thickness of the concrete bolster between the bottom of the slab and the top of stringer, without alteration of the slab thickness.

NOTE TO FABRICATOR:

DATE

lated value.

BY

P.R.Y.

MADE

CHECKED

IN CHARGE

EVR 11-67

T.E.M. 2-68

The above stringers shall be fabricated with an upward camber amounting to the tabu-

This will provide approximate compensation for deflection under full dead load and for conformity with finished grade.

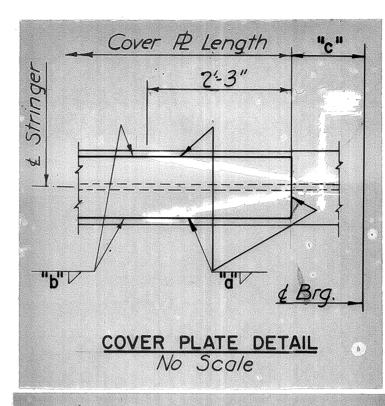
Dimensions are in inches.

As Built

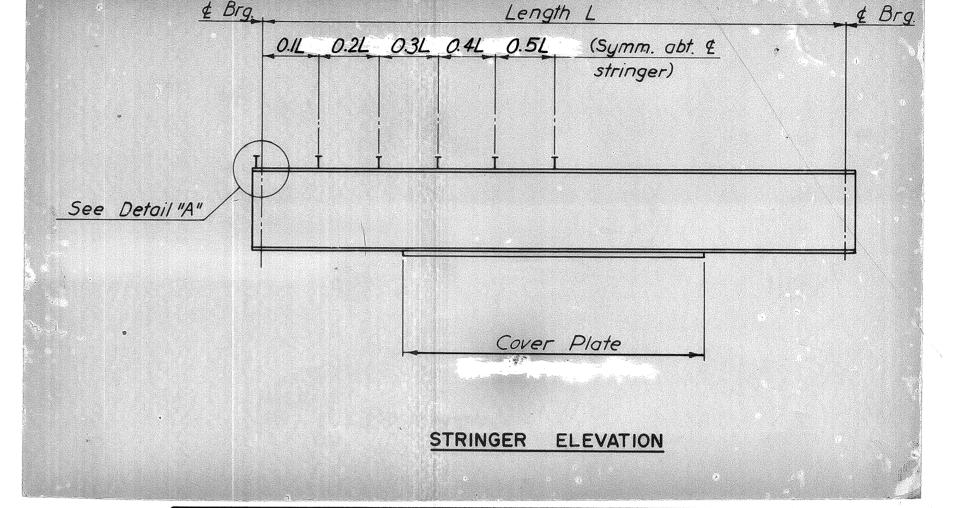
REVISION

TEM 7-77

DATE



COVER PLATE SCHEDULE								
STR.	COVER RL	" a "	"b"	"¢"				
51,56,57	102×14	¹ 2"	5" 16	5'-3"				
<i>52-55</i>	102×18	2"	5" 16	5'-9"				



RICHMOND EXPRESSWAY SYSTEM

215

PROJECT

DOWNTOWN

EXPRESSWAY

SECTION

STRINGER SCHEDULE									
STRINGER	LENGTH	COVER PLATE	SHEAR STUD SPACING						
	£ BRG.TO € BRG.	OOVER TEATE	O.IL	0.2L	0.3L	0.4L	0.5L		
51 -36 WF 150	65'-0"	102 x/4 x 54-6"	62"	8"	102"	/3:"	<i>15</i> "		
S2 — T	A	102 x 18x 53-6"	7"	Towns of the second	1 A 1 A 1	125"	14"		
53~		10 ² ×1'8 × 53'-6"	•		e j June se	1	A		
54~		102×18×53-6"		6			,		
S5~		102 x/8 x 53'-6"	—			•			
<i>56</i> —	₩	102×14×54-6"	7"			122"	14"		
<i>57 —</i>	65′-0″	10'2x1'4x54'6"	62"	8"	102"	/3"	15"		
<i>38 -</i>	48′_9"	No Cover Plates	72"	9"	10/2"	132"	152"		
<i>59</i> —	†	4		1	1	125"	14"		
S/0~						1	1		
S// ~									
5/2			. ,.			-	1		
S/3	V	+	•	\		122"	14"		
S14 -36 WF 150	48′-9″	No Cover Plates	72"	9"	102"	/32"	152"		

Note: Lengths shown are horizontal distances measured along centerlines of stringers.

SHOE SCHEDULE								
	NSION OES	FIXED SHOES						
TYPE	NO. REQD.	TYPE	NO. REQD.					
EI	7	FI	10					
E2.	7	FIM	4					

Steam Stud Note:

Capacity = 3,400 lbs. per stud.
The contractor may, if he elects, use three 3" diameter studs at the same longitudinal spacing in lieu of the four 3" diameter studs shown.

Stud rows shall be placed parallel to the main deck reinforcing.
Shear stud spacing shown is maximum

butt. weld (Typ.)

SHEAR STUD DETAIL



DOWNTOWN EXPRESSWAY

BRIDGE B-60
IOTH STREET OVER
DOWNTOWN EXPRESSWAY
FRAMING PLAN

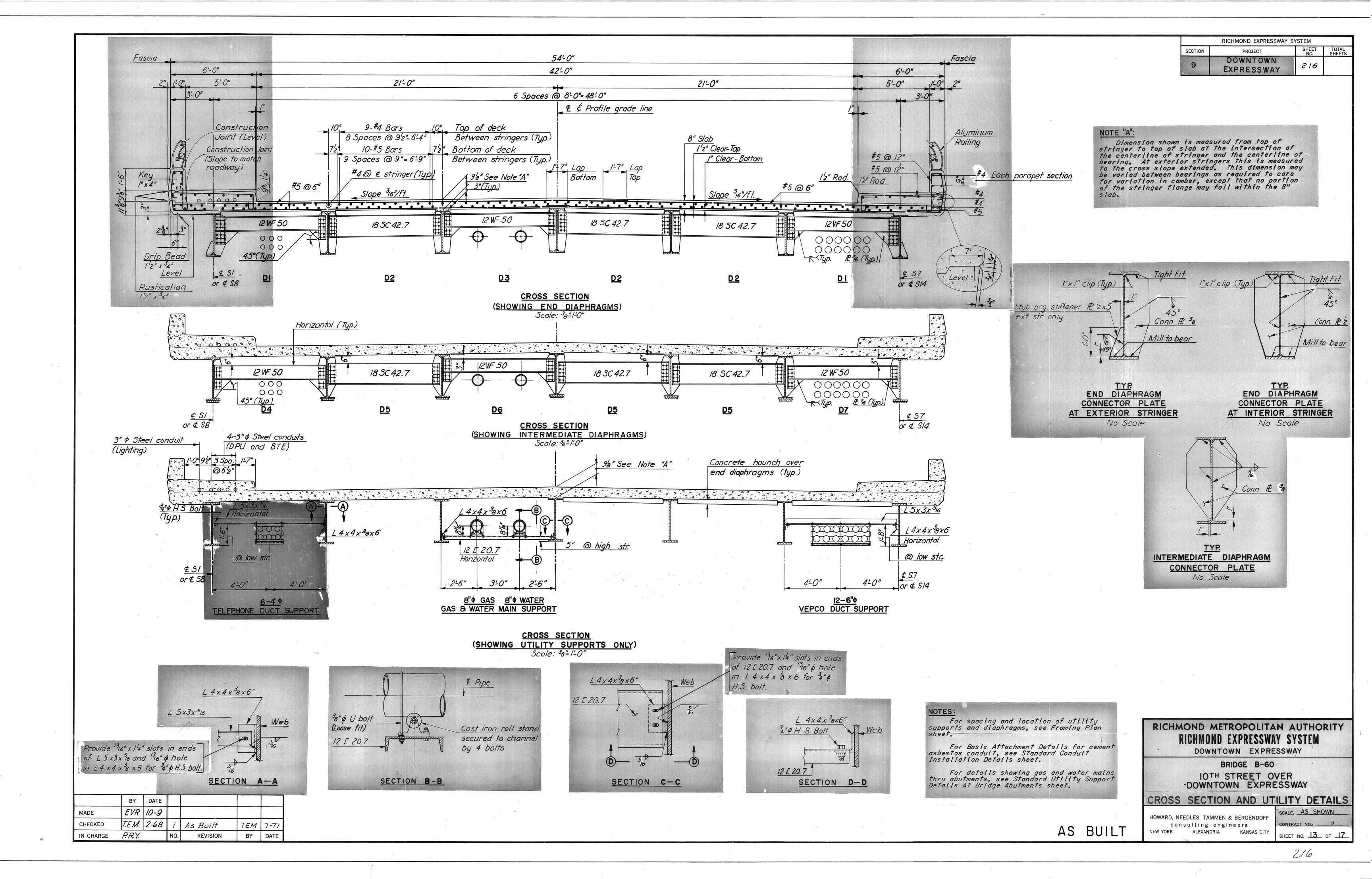
HOWARD, NEEDLES, TAMMEN & BERGENDOFF
consulting engineers
NEW YORK ALEXANDRIA KANSAS CITY

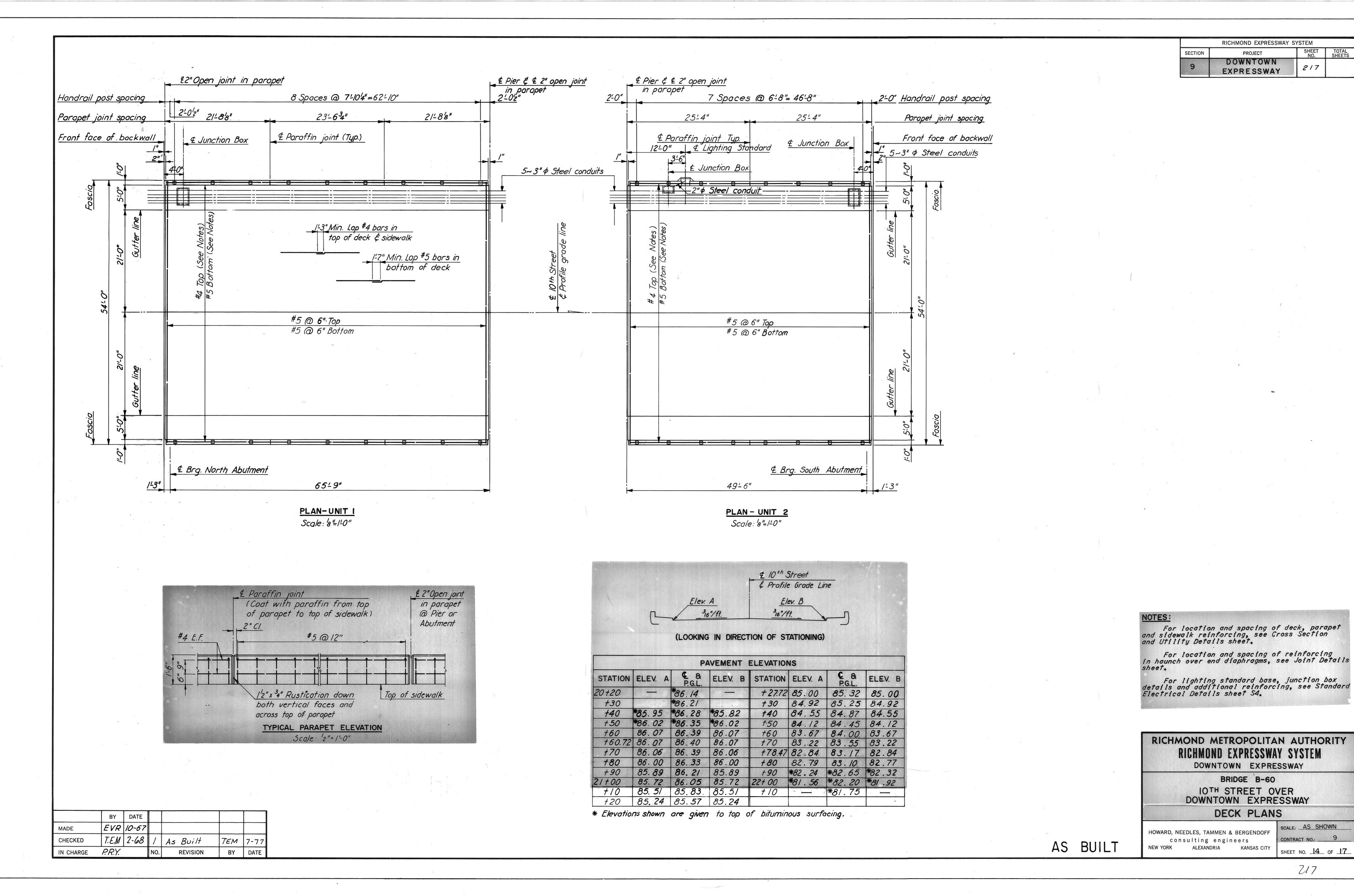
SCALE: AS SHOWN

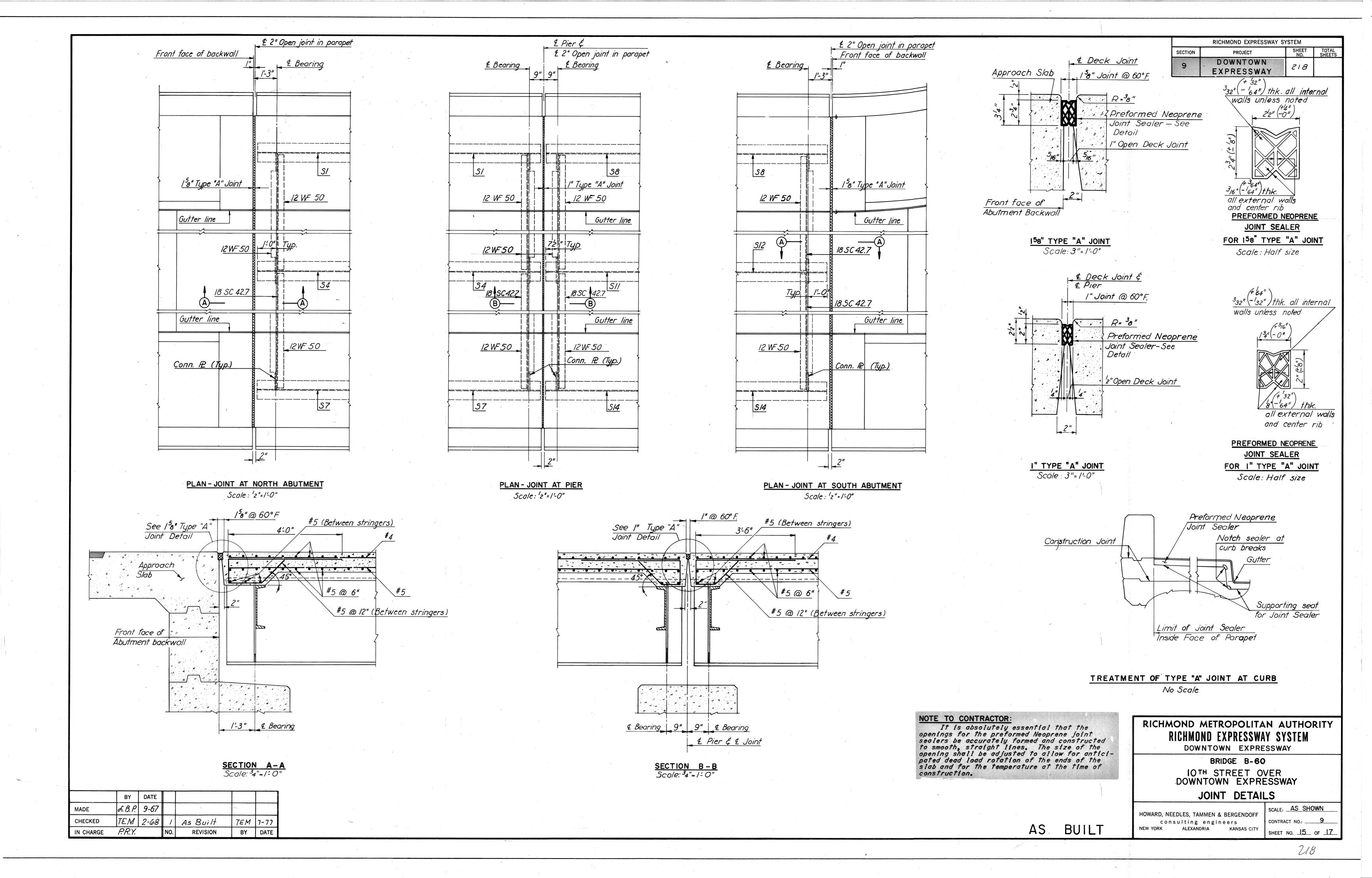
CONTRACT NO.: 9

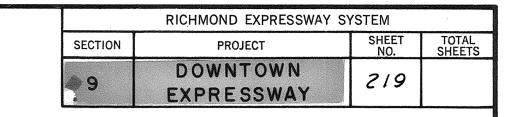
SHEET NO. 12 OF 17

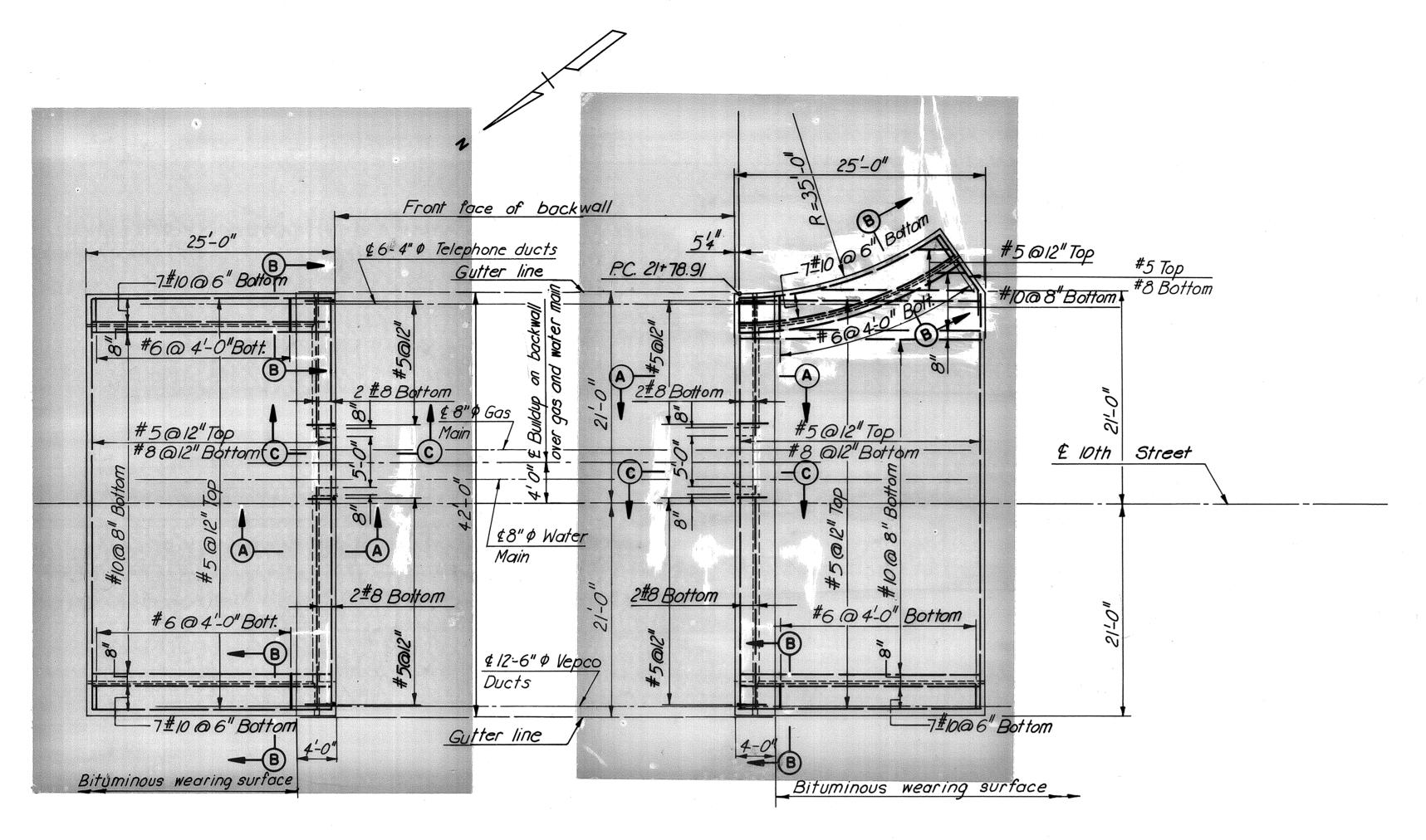
AS BUILT











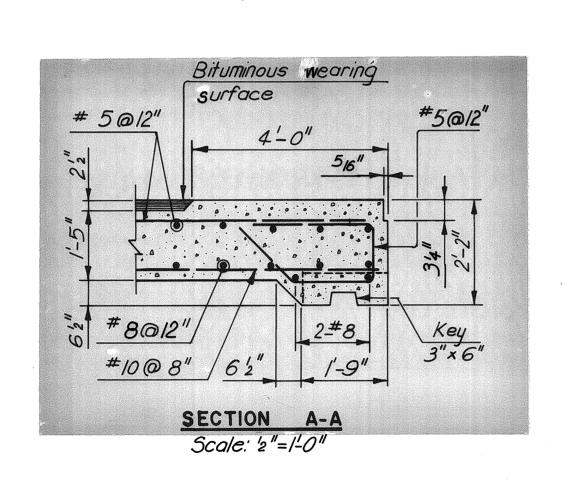
NORTH APPROACH SLAB

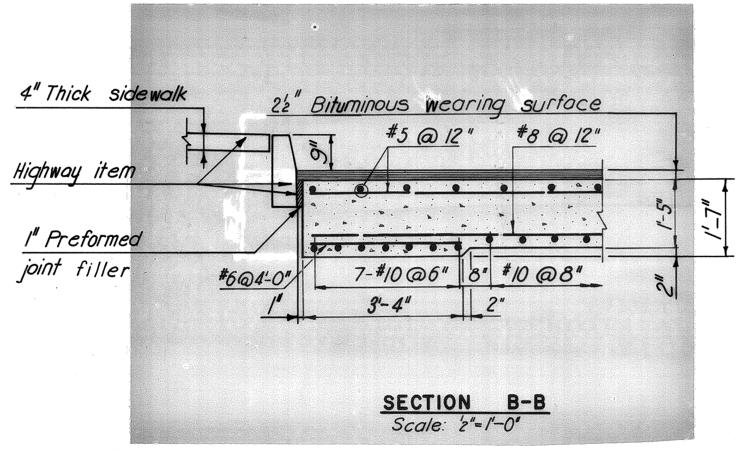
Scale: 'g"=/-O"

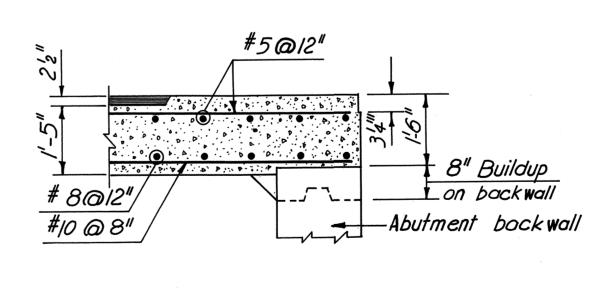
SOUTH APPROACH SLAB

Scale: '8"=/-0"

NOTE:
All reinforcing steel shall be 2" min.
clear from edge of bar to edge of concrete.







<u>SECTION</u> C-C Scale: '2"=/-0"

	BY	DATE				
MADE	elm	12-67				
CHECKED	TEM	2-68	1	As Built	TEM	7-77
IN CHARGE	P.R.Y.	^	NO.	REVISION	BY	DATE

RICHMOND EXPRESSWAY SYSTEM DOWNTOWN EXPRESSWAY

RICHMOND METROPOLITAN AUTHORITY

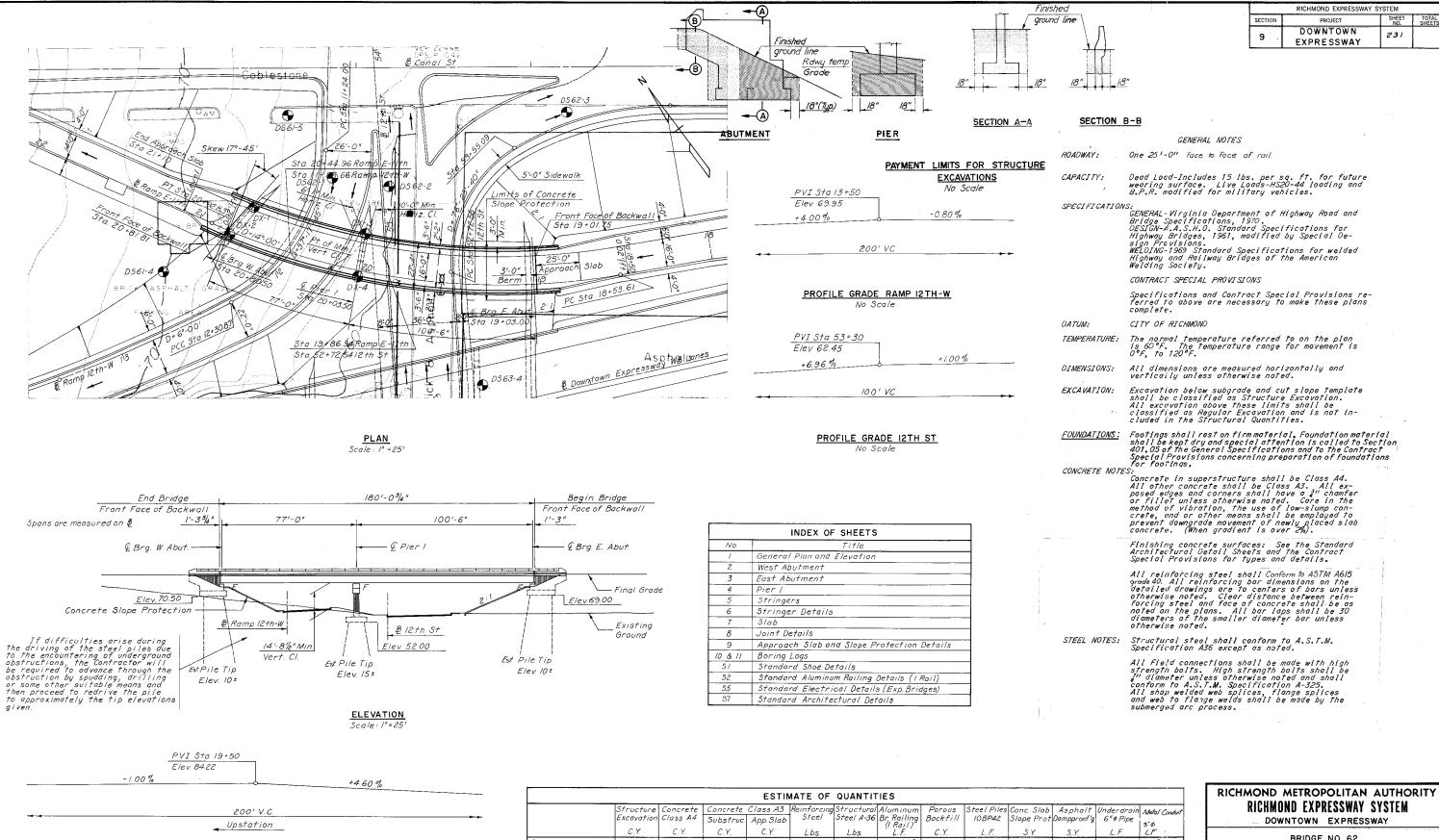
BRIDGE B-60 IOTH STREET OVER DOWNTOWN EXPRESSWAY APPROACH SLABS

SCALE: AS SHOWN HOWARD, NEEDLES, TAMMEN & BERGENDOFF

AS BUILT

consulting engineers
NEW YORK ALEXANDRIA KANSAS CITY SHEET NO. 16 OF 17

(Westbound Downtown Expressway {Rte. 195} 11th Street Off-Ramp Over South 12th Street)



Superstructure

West Abutment

East Abutment

Approach Slabs

Pier I

otal

171.57

38/ /71.5/

78.63

81.50

71.**3**7

/33

/35

113

39,055 | 168,253

4,298

4.235

22,443

49.82 11,682

231.50 49.82 81,7/3 /68,253

349

409

30

30

581.3 /23.25

AS

719.3 162.2

10 1970.7 286.45

670.1

PROFILE RAMP E-IITH

No Scale

TEM 17-77

Revised Light Location T.E.M 9-74

BY DATE

JLJ 11-67

LDL 1-68

CHECKED

IN CHARGE PRY

BRIDGE NO. 62 RAMP E-IITH OVER RAMP 12TH-W AND 12TH ST GENERAL PLAN AND ELEVATION

HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers ALEXANDRIA KANSAS CITY

183

59

28 114

BUILT

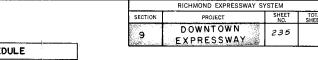
55

. 23.5

23.5

230

As Noted



		SHOE SO	HEDULE	
		NSION DES		XED IOES
TY	PE	NO. REQD.	TYPE	NO. REQD.
Ε	5	8	F3	3
	,		F14	1.
ž.				

& Brg. E. Abut.

All Shoes E.5

R 12x 2x18:78

R12x1x18'-216

Æ16x1x17'-10

5 Spa Stringer A
5 Spa Stringer B
5 Spa Stringer B
4 Eq. Spa 5 Spa Stringer C
4 Eq. Spa 5 Spa Stringer C

R 16x1x17'78 Stringer B

PE 16x14x17:54 Stringer C

PE 16x 1 1/2x429 Stringer D

R12x 4x18:54

B Ramp E-lith

on B

Stringer A

Stringer B

Stringer D

Stringer A

Stringer A

Stringer B

Stringer C

Stringer D

Stringer A

Stringer B

Stringer C

Stringer,D

Stringer C

Type C Diaphragm

stringer

stringerC

stringer D

8@ 12'-63"

100'-6"

Area A

R12x 2x 79:10

R12x1x40'0

R 12x14x45'-0

R 12x 13x44-0

R 46x 38

8 Spa. @ abt. 11'-11 34" = 95:-10"

8 Spa. @ abt. 12'-5'8 " = 99'.5'4' 8 Spa. @ abt. 12'-7'8 " = 101'-2'%

95'-10'

97'-758"

99'-54

101-236

8 Spa. @ abt. 12'-2 16" = 97'-7 8"

(Typ.)

R 16x12x45'-0

R 16x12x45-0

R 16x2 x47'-0

R 16x2 4x55'-0

No Stiffeners

No Stiffeners

No Stiffeners No Stiffeners

Skew 17°45'-

All Shoes E.

Shear Stud

Top Flange

Bottom Flange

Intermediate Stiffener Spacing

Diaphragm

Spans

MADE

CHECKED

IN CHARGE PRY

Stiffener Spacing 4 Spa. 0
'Between Diaphragins, 5 Spa. 0

BY DATE

RDR 12-67

JLJ 1-68

3 Spa. ① 4 Spa. ②

6'-1%"

8'-8 1/6"

13'-10%"

1 As Built

11-3%

@ Brg. W. Abut.

Type C Diaphragm

6@12:10"

Taper flg. width @ 1:24

Area A

R 12 x 2x41'-38

R 12x 2x45'-03

R12x/2x46'-9%

R12x 2x48 .6516

Web R 46x 38

(Typ.)

R 16x \$x51:35

R 16x 4 x 55'-0

R16x 4x56.93

R 16x1x 58'-6 5/16

No Stiffeners

TEM 7-77

No Stiffeners

No Stiffeners

71'-0多

74'-9%

78'-65/16"

5 Spa.@ abt. 12'-27/8"= 61'-21/4"

5 Spa. @ obt. 12'-5 &"=62'-4 1/6"

5 Spa. @ abt. 12'-8 = 63'-5 6

5 Spa @ obt. 12'-118"= 64'-78

& Field Splice _

.C. Pier I

Type B Diaphragi

Varies

F3 Shoes, Stringers A.BEC F.14 Shoes, Stringer D

Area A

201-016 Oil -021 021-031 030-041 041-051 051-061 061-071 071-081 081-091 091-1013

[10" | 13" | 17" | 24" | 21" | 15" | 12" | 15" | 13" | 12" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 15" | 16" | 12" | 14" | 18" | 24" | 24" | 18" | 14" | 10" | Stringer B

[10" | 12" | 16" | 20" | 21" | 16" | 12" | 15" | 15" | 13" | 13" | 15" | 11" | 14" | 18" | 24" | 22" | 17" | 12" | 10" | Stringer C

[8" | 10" | 14" | 20" | 18" | 12" | 10" | 12" | 10" | 9" | 9" | 9" | 7" | 10" | 14" | 20" | 20" | 14" | 9" | 7" | Stringer D

R 16x 1x 25'-0

R 16x1x27-0

R 16x14x27-0

R 16x12x22'-0

Varies

FRAMING PLAN Scale I"=10

 RIGX 4x10'0 RIGX14x8'0
 R 16x
 2x16'0
 R 16x18x8'0

 RIGX 4x10'0 RIGX14x8'0
 R 16x
 2x16'0
 R 16x1x10'0
 R 12x2x21'0

 RIGX 4x10'0 RIGX12x10'0
 R 16x
 24x16'0
 R 16x12x8'0
 R 12x34x20'0

 R 16x1x10'0 RIGX12x10'0
 R 16x
 3x20'0
 R 16x12x8'0
 R 12x12x1x21'0

₽ 46 x ³8

(Typ.)

 4 Eq. Spa
 4 Eq.

GIRDER ELEVATION

No Scale

P.16x14x8'0 P.16x 2x16'0

R16x14x8'0 R16x 2x16'0

R16x12x10:0 R16x 24x16:0

#16x14x10-0 # 16x 3x 20-0

. C Field Splice

Type A

Diaphragm (Typ.)

D=14°00'

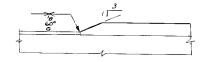
DEFLECTION AND CAMBER										
Required Camber Vertical Curve							've			
	4 / 12	34					1,	1	7.	
W.Abut. Pier Dead Load Deflection E. Abut.						out.				
STRINGER	ITEM	W. ABUT.	1/4	1/2	3/4	PIER I	1/4	1/2	3/4	E. ABUT.
	Steel	0	46	V16	0	0	3/16	3/8	1/4	0
	Slab	0	3/16	1/8	0	0	5/B	1 416	78	0
Α	Curb and Parapet	0	416	V16	-46	0	V4	7/16	38	0
	Vertical Curve	0	15/16	15/8	13/8	0	33/16	4/4	33/16	0
	Required Camber	0	1/4	17/8	15/16	0	45/8	61/8	4 11/16	0
	Steel	0	V16	V16	0	0	4	3/8	14	0
	Slab	0	3/16	1/8	0	0	3/4	15/16	146	0
В	Curb and Parapet	0	0	0	0	0	0	V16	0	0
	Vertical Curve	0	15/16	15/8	13/8	0	3316	41/4	33/16	0
	Required Comber	0	13/16	1/3/16	13/8	0	43/16	6	41/2	0
	Steel	0	V16	416	0	0	3/16	3/8	1/4	0
•	Slob	0	14	3/16	0	0	3/4	15/16	146	0
С	Curb and Parapet	0	0	0	0	0	V16	VB	V16	0
	Vertical Curve	0	15/16	1 11/16	17/16	0	33/16	41/4	3 3/16	0
	Required Camber	0	11/4	115/16	17/16	0	43/16	6/16	4 2/16	0
	Stee!	0	416	V16	0	0	V4	5/16	14	0
	Slab	0	14	3/16	0	0	5/8	1/8	7/8	0
D .	Curb and Parapet	0	V8	VB	0	0	5/16	8/16	7/16	0
	Vertical Curve	0	15/16	11/16	17/16	0	3 3/16	4 1/4	3 3/6	0
	Required Camber	0	13/8	21/16	17/16	0	43/8	64	4 34	0

The stringers shall be fabricated with an upward camber amounting to the tabulated values. These values are in inches.

This will provide approximate compensation for deflection under full dead load and for conformity with finished grade.

the deflections anticipated to occur in the stringer upon placement of the total concrete deck dead load are equal to the sum of the values listed for "Slab" and "Curb and Parapet".

Deviations from exact camber of the stringers shall be provided for by adjusting forms to vary the thickness of the concrete bolster between the boltom of the slab and the top of the stringers.



SHOP FLANGE SPLICE DETAILS

- The first two spaces at the ends of the stringers shall be one-half of the remaining spaces.
- © Stiffeners shall be relocated as required to clear web splice plates.
- ③ See Shear Stud Details At End Bearings.

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

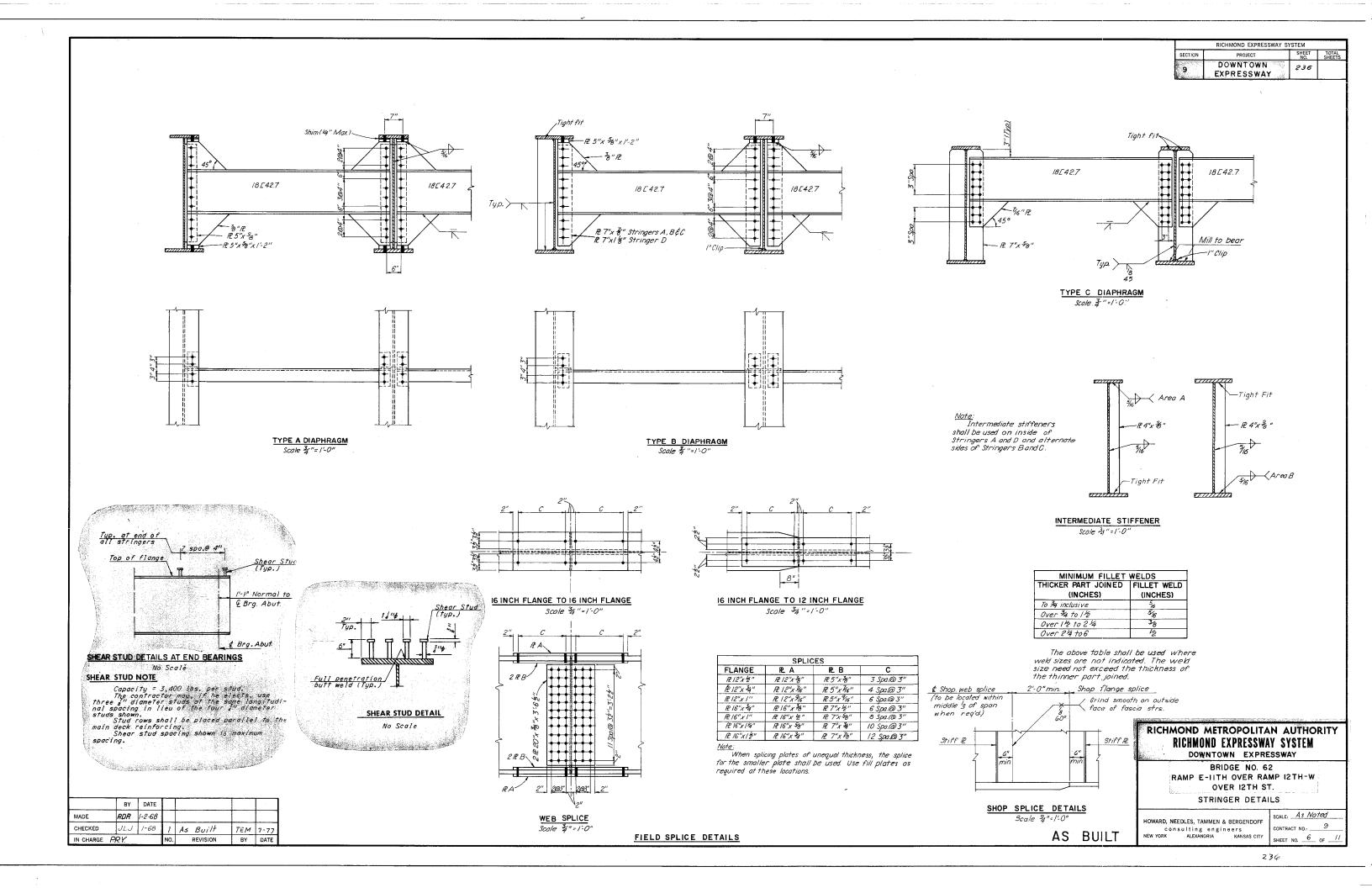
DOWNTOWN EXPRESSWAY BRIDGE NO. 62

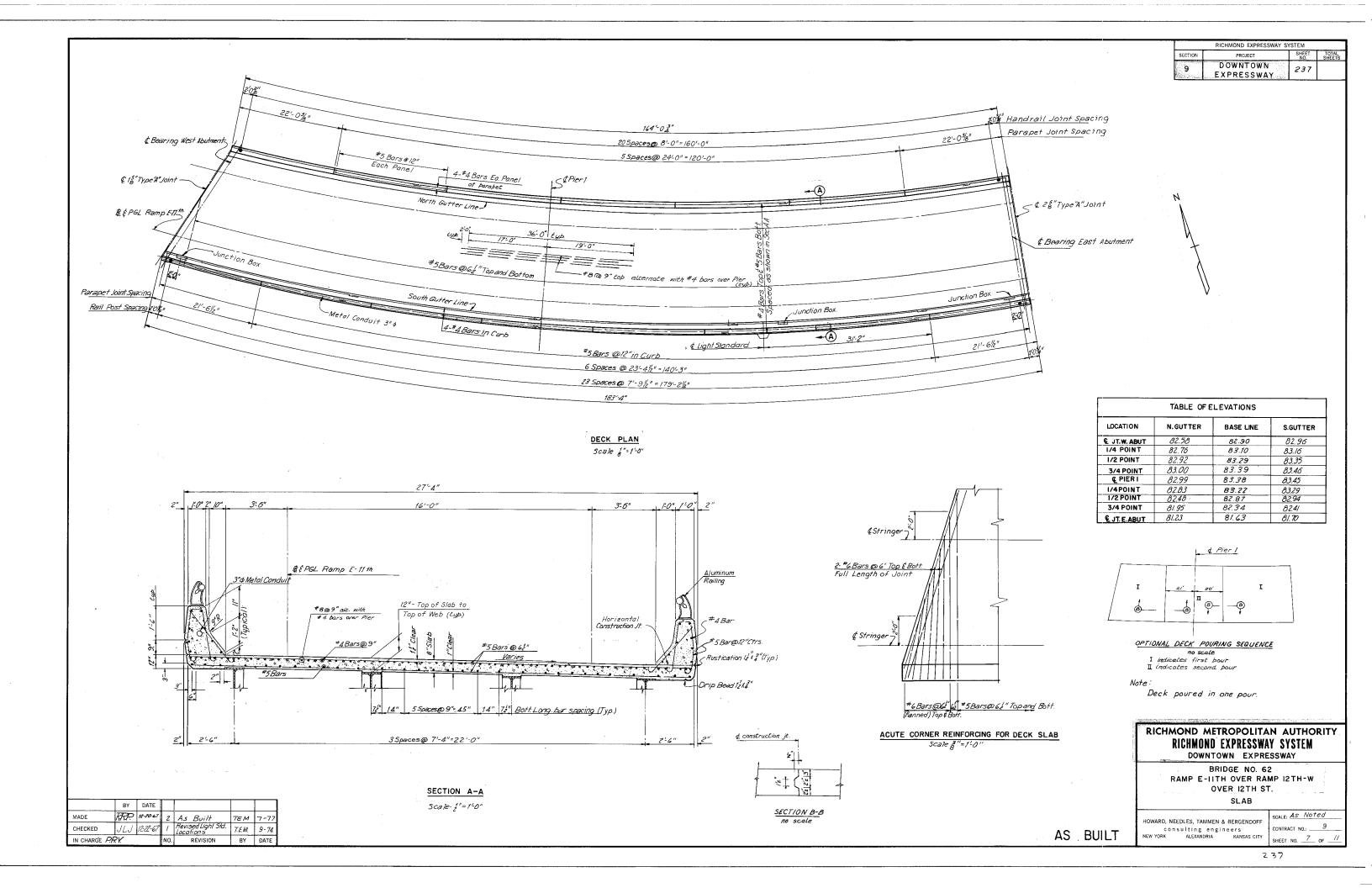
RAMP E-IITH OVER RAMP 12TH-W OVER 12TH ST. STRINGERS

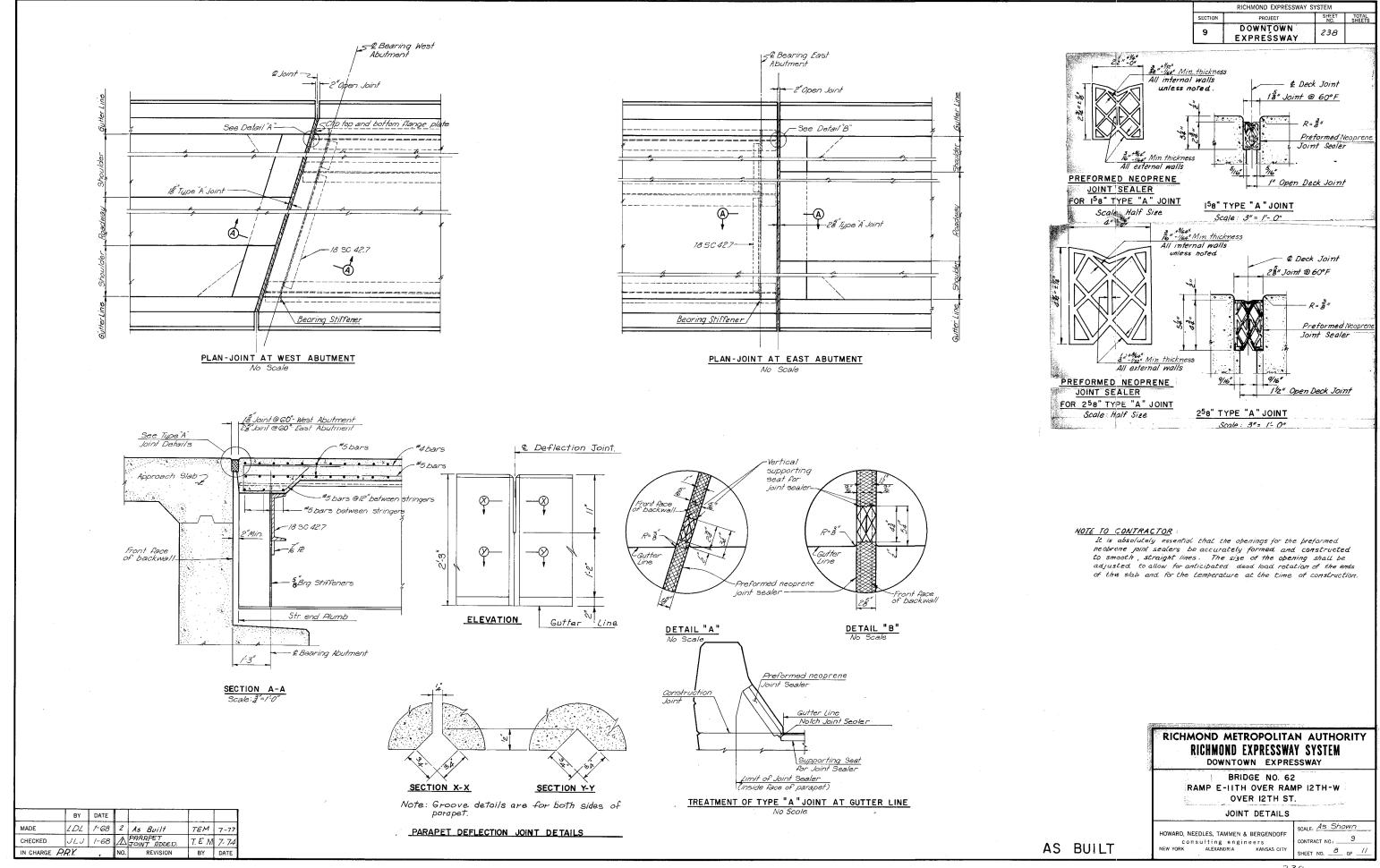
HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers ORK ALEXANDRIA KANSAS CITY

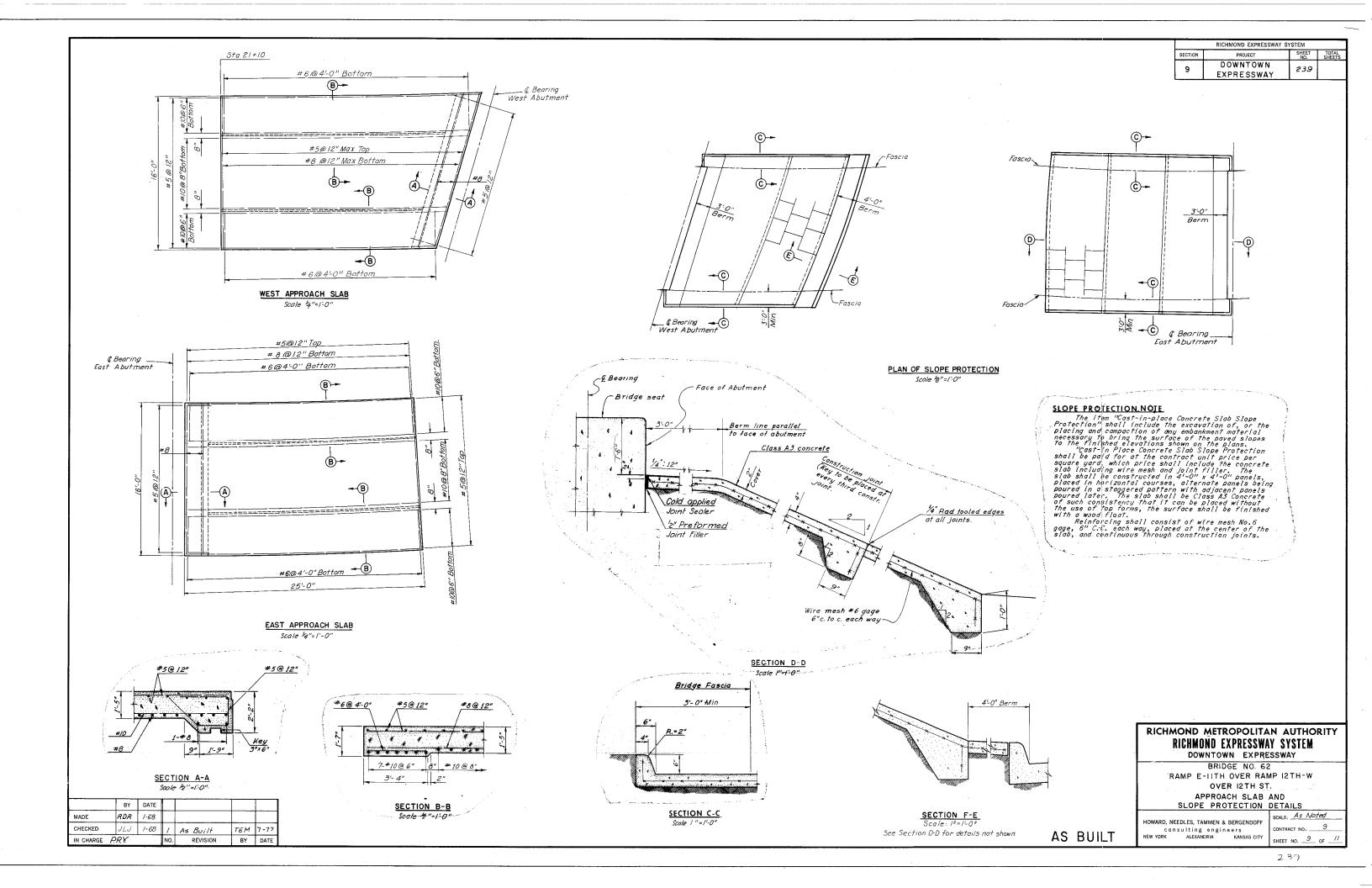
AS BUILT

SCALE: As Noted



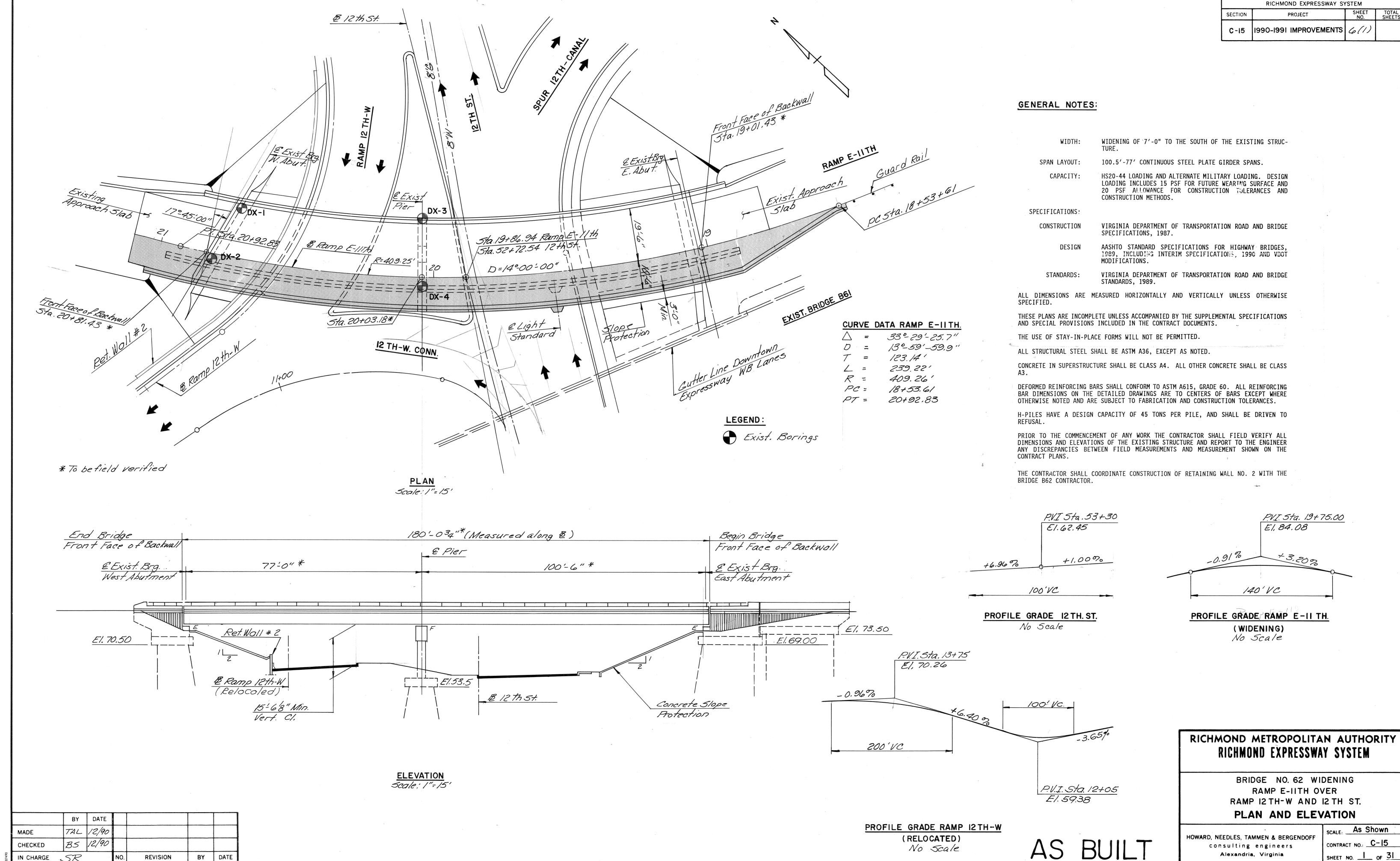






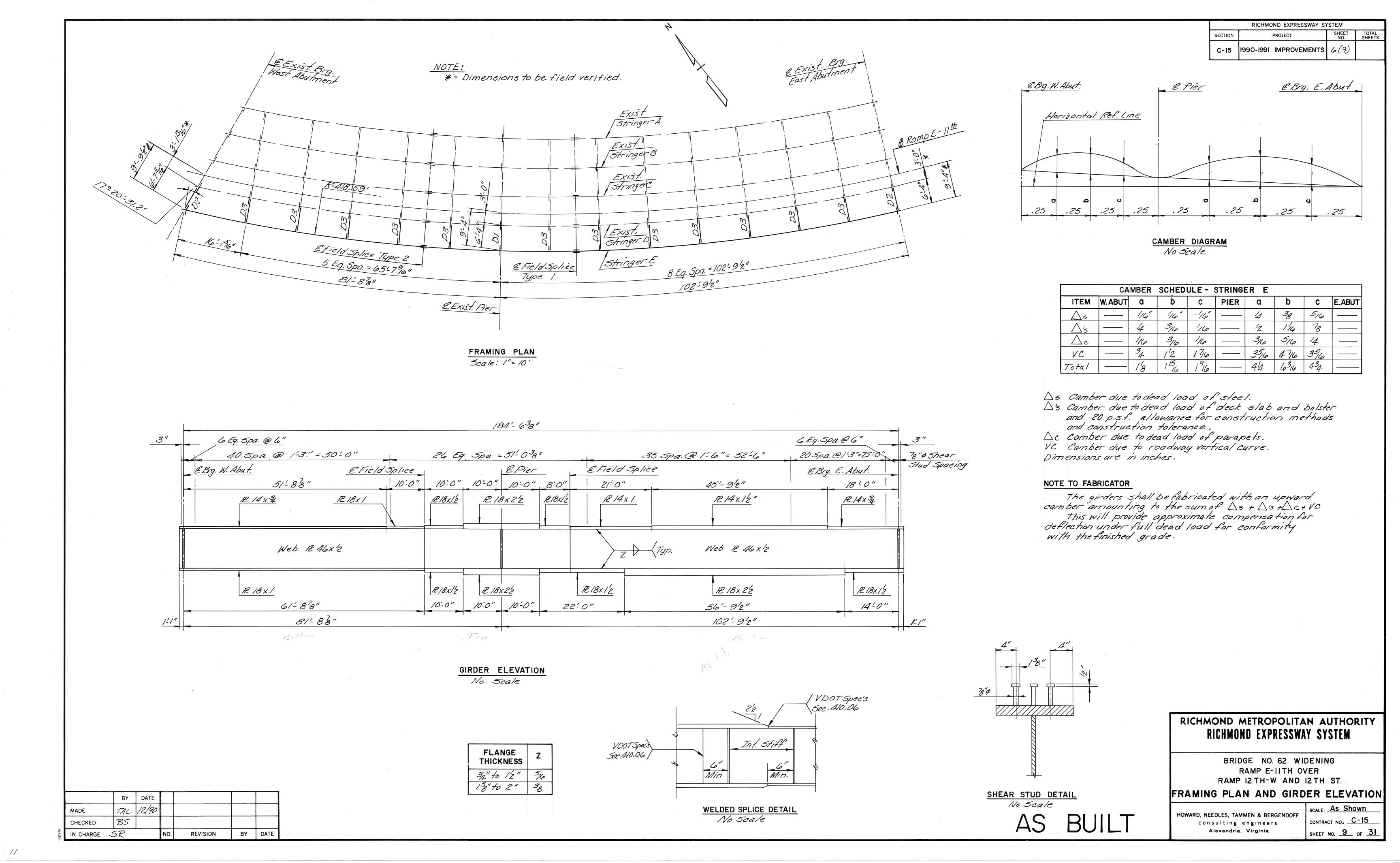
Bridge 62 Widening

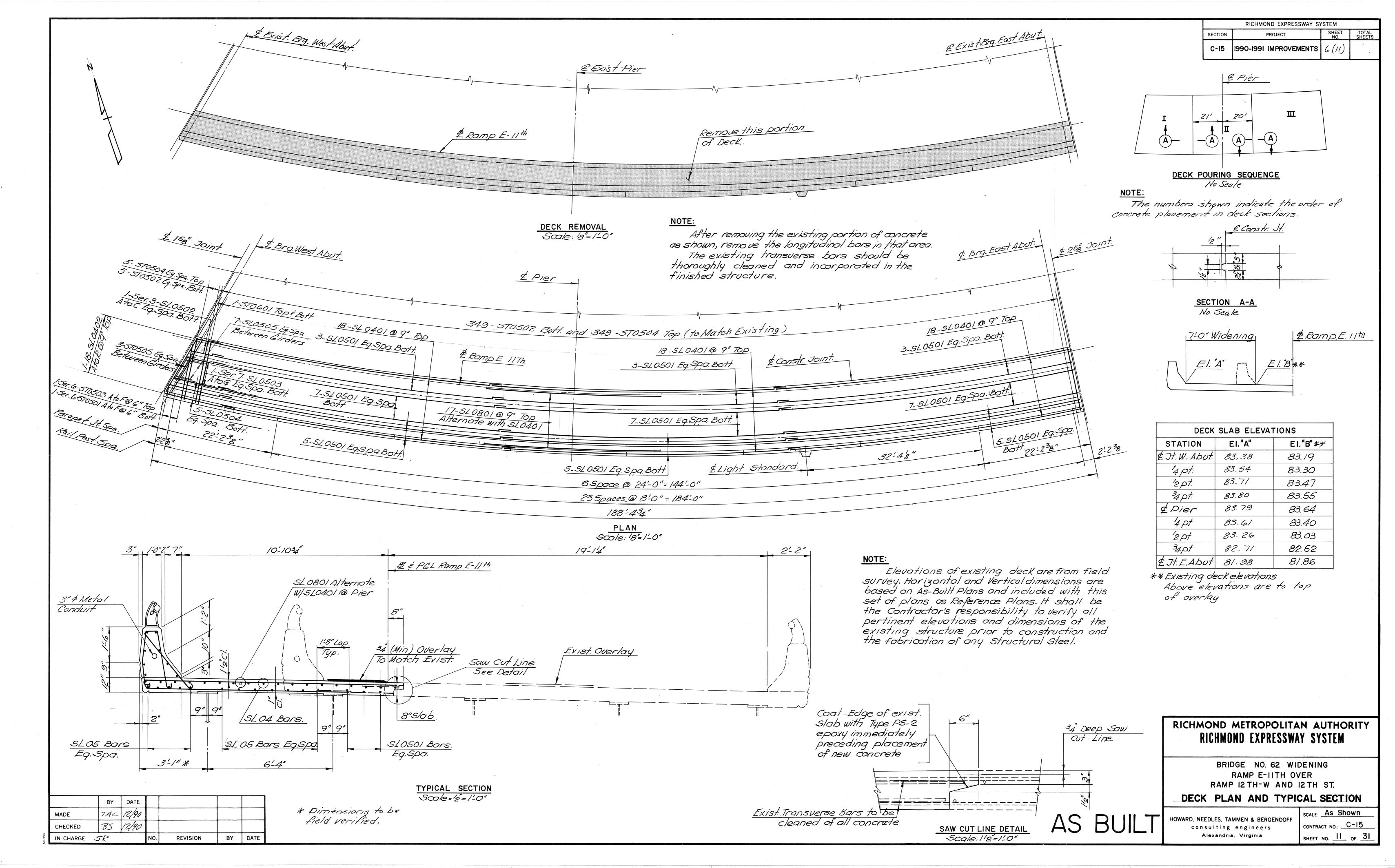
(Westbound Downtown Expressway {Rte. 195} 11th Street Off-Ramp Over South 12th Street)



14237-21-02

SHEET NO. 1 OF 31





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