## RICHMOND METROPOLITAN TRANSPORTATION AUTHORITY

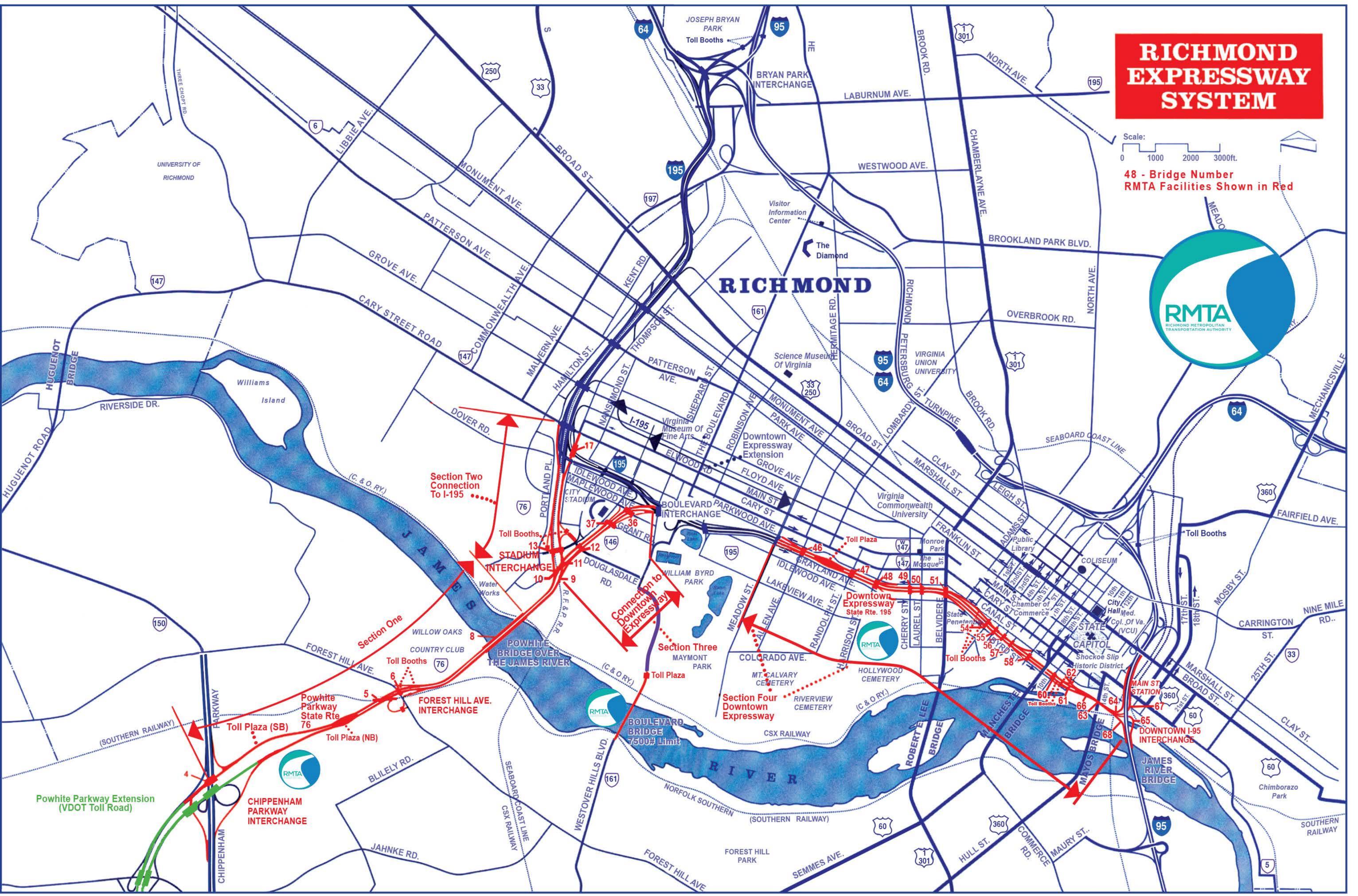
# PC-2023 APPENDIX

### **RECORD DRAWINGS**

RMTA BRIDGES 4, 5, (8 Original and 8 Widening), 12, 13, 17, 36, 37, 50, 51, 54, 55, 56, 57, 58, and 60

(NOTE: Additional As-built Plans are Available upon Request to the Engineer)

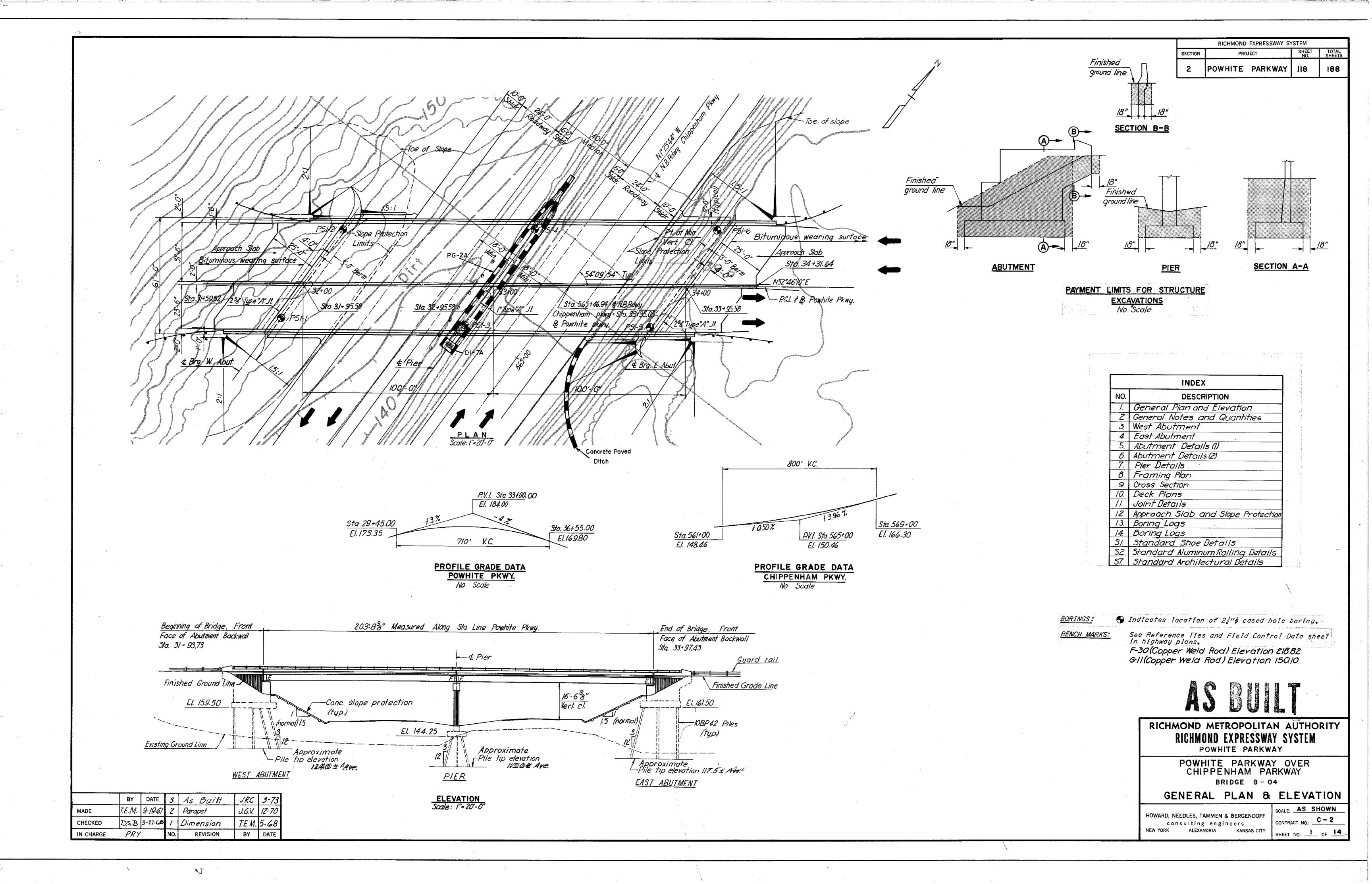
# RMTA System Map



# **Bridge 4**

Southbound Powhite Parkway (VA-76)
Over
Both directions of Chippenham Parkway (VA 150)

**Partial Record Set of Plans** 



**GENERAL NOTES** 

One 31'-6" and one 23'-6" clear roadway.

Dead Load-Includes 15 lbs. per sq.ff. for
future wearing surface.
Live Loads-HS20-44 loading and B.P.R. modified
for military vehicles.

SPECIFICATIONS:

GENERAL-Virginia Department of Highway Road and Bridge Specifications, 1970. DESIGN-A.A.S.H.O. Standard Specifications for Highway Bridges, 1961, modified by Special Design Provisions.

WELDING-1969 Standard Specifications for Welded
Highway and Rallway Bridges of the American
Welding Society.

CONTRACT SPECIAL PROVISIONS

Specifications and Contract Special Provisions referred to above are necessary to make these plans complete.

DATUM:

CITY OF RICHMOND

TEMPERATURE:

The normal temperature referred to on the plan is 68°F. The temperature range for movement is 0°F to 120°F.

DIMENSIONS:

All dimensions are measured horizontally and vertically unless otherwise noted.

EXCAVATION:

Excavation below subgrade and cut slope template shall be classified as Structure Excavation. All excavation above these limits shall be classified as Regular Excavation and is not included in the Structural Quantities.

CONCRETE NOTES: Concrete in superstructure shall be Class A4. All exposed other concrete shall be Class A3. All exposed edges and corners shall have a 3/4" chamfer or fillet unless otherwise noted. Care in the method of vibration, the use of low-slump concrete, and/or other means shall be employed to prevent downgrade movement of newly placed slab concrete (when gradient is over 2%).

Finishing Concrete Surfaces: See the Standard Architectural Details sheets and the Contract Special Provisions for types and details.

All reinforcing steel shall conform to A.S.T.M. A615. Grade 40.
All reinforcing bar dimensions on the detailed drawings are to centers of bars unless otherwise noted. Clear distance between reinforcing steel and face of concrete shall be as noted on the plans. All bar laps shall be 30 diameters of the smaller diameter bar unless otherwise noted.

STEEL NOTES:

Structural steel shall conform to A.S.T.M. Speci-fication A36 except as noted. All field connections shall be made with high strength bolts.
High strength bolts shall be 7/8" diameter unless otherwise noted and shall conform to A.S.T.M.
Specifications A325.

See Reference Ties and Field Control Data sheet in highway plans.

F30(Copper Weld Rod) Elevation 218.82 G-11(Copper Weld Rod) Elevation 150.10

				ES	TIMATED	QUANTITIE	S					
	STRUCTURE EXCAVATION  C.Y.	CONGRETE CLASS A3 CY	CONCRETE CLASS A3 BR. APPR SLABS C.Y	CLASS A4	REINFORCING STEEL LBS	STRUCTURAL STEEL LBS	ALUMINUM BR. RAILING (I RAIL) L.F.	POROUS BACKFILL CY	10BP42	CONC SLAB SLOPE PROTECTION S Y	DAMP- PROOFING S.Y.	UNDERDRAIN 6" Ø L F
SUPERSTRUCTURE	State			380.39	103,414	427,036	407			S may read a bus to a substant season of substantial season of sub	The second secon	
WEST ARUTMENT	540	294.15		·	12,729	:	36	45	1934.8	228	140	1//
PIER I	175	136.91			24,625				1174.5			
EAST ABUTMENT	460	2/7.95			10,872		24	26	1634.6	228	.91	94
APPROACH SLABS			204.77		57,934							
	1,175	649.01	204.77	380.39	209,574	427,036	467	7/	4743.9	456	23/	205
TOTAL												

	BY	DATE	3	As Built	JRC	3-73
MADE	EVR	3-68	2	Parapet	HBW	12-70
CHECKED	PRY	3-68	/	General	J.G.V.	10-70
IN CHARGE	PRY	, <u> </u>	NO.	REVISION	BY	DATE



RICHMOND EXPRESSWAY SYSTEM

POWHITE PARKWAY 119

PROJECT

SECTION

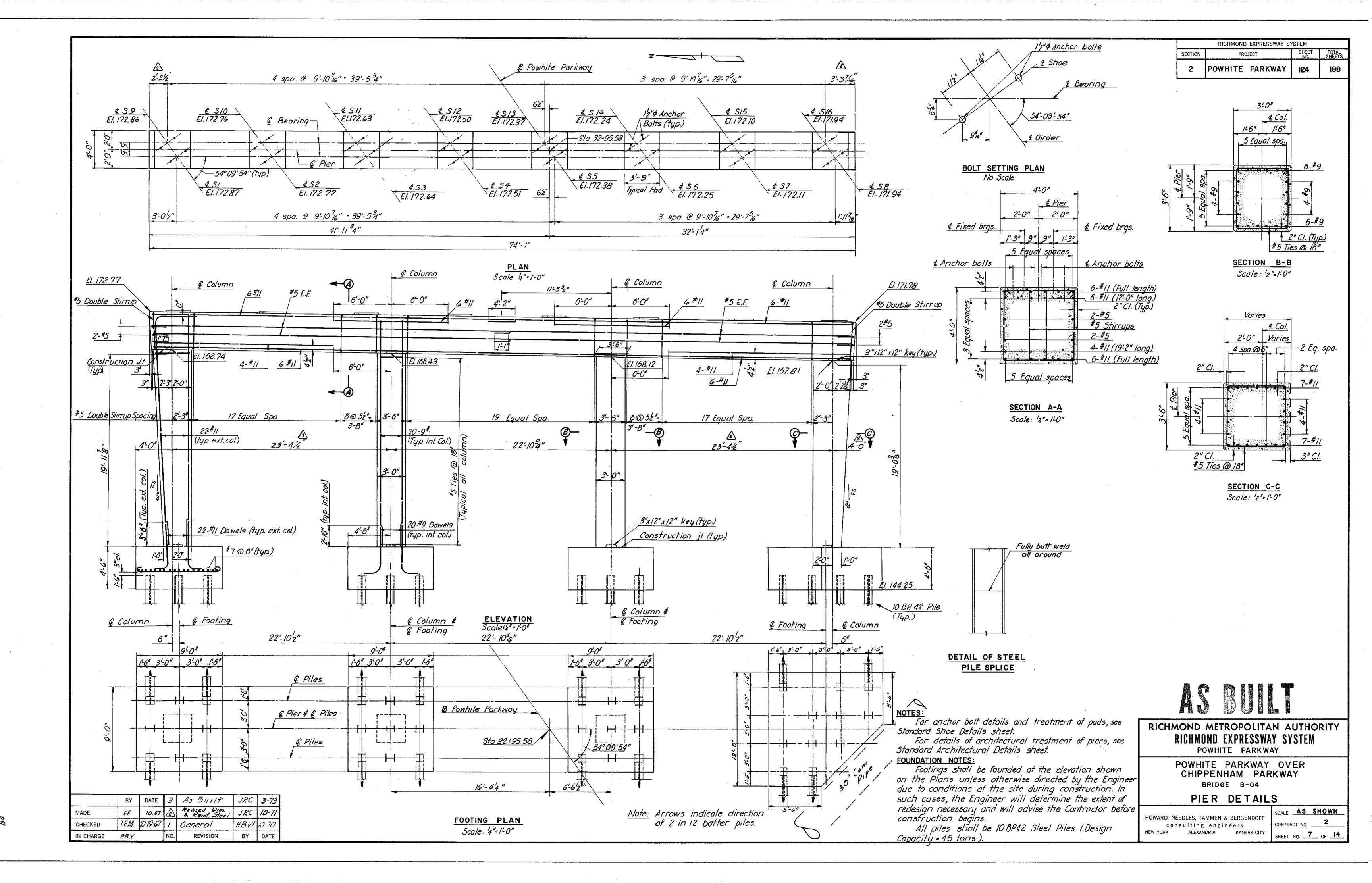
SHEET TOTAL NO. SHEETS

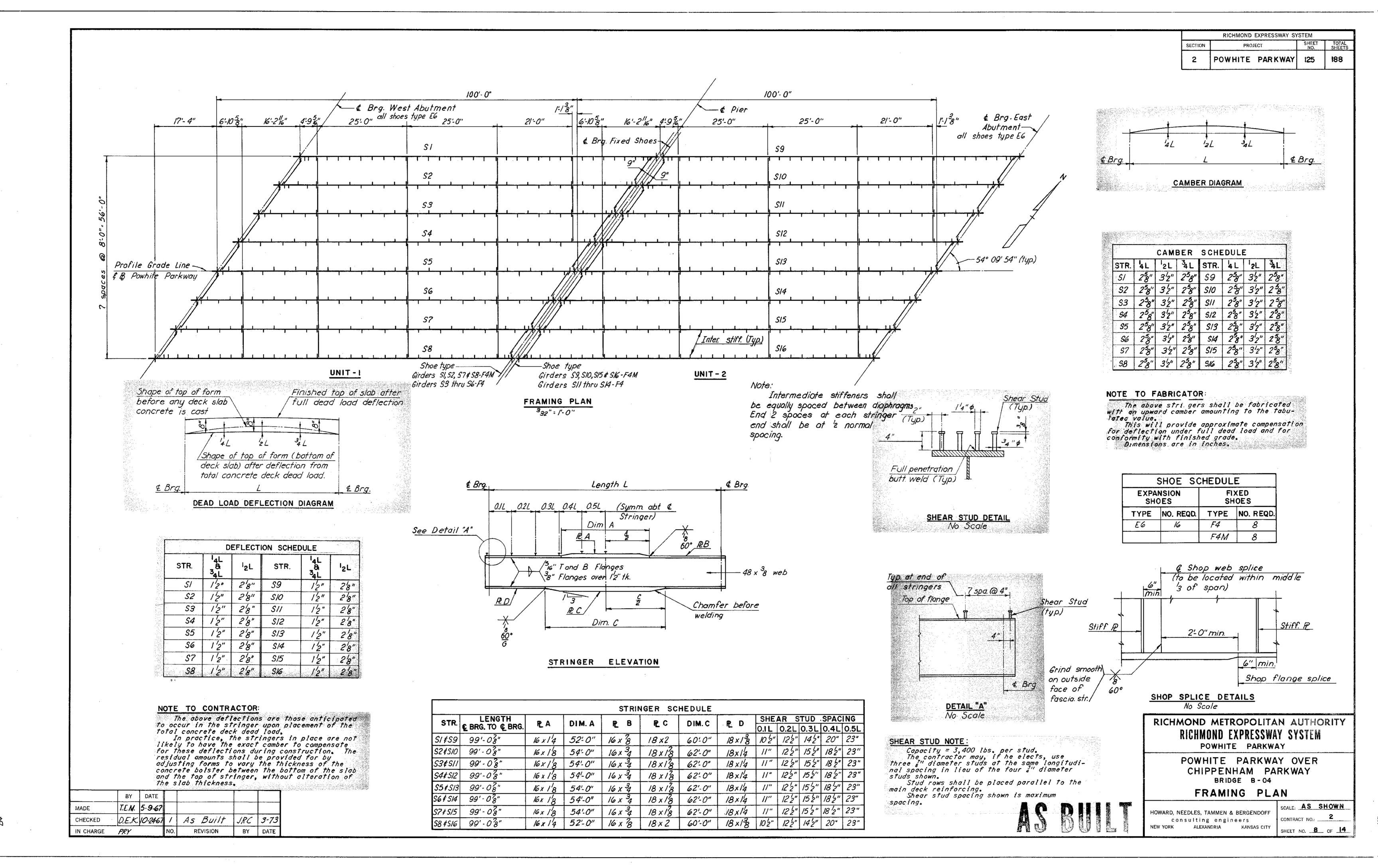
RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

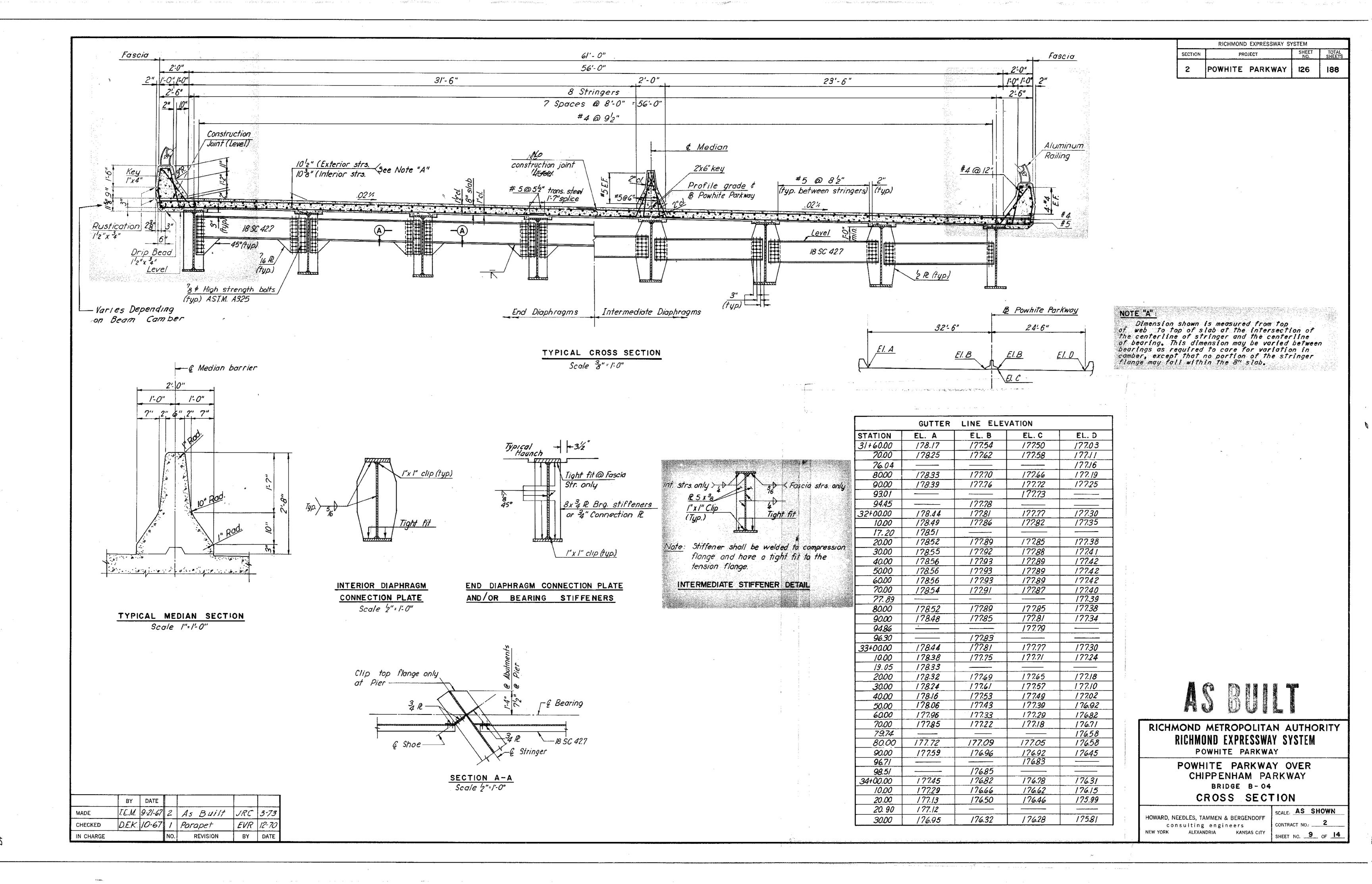
POWHITE PARKWAY

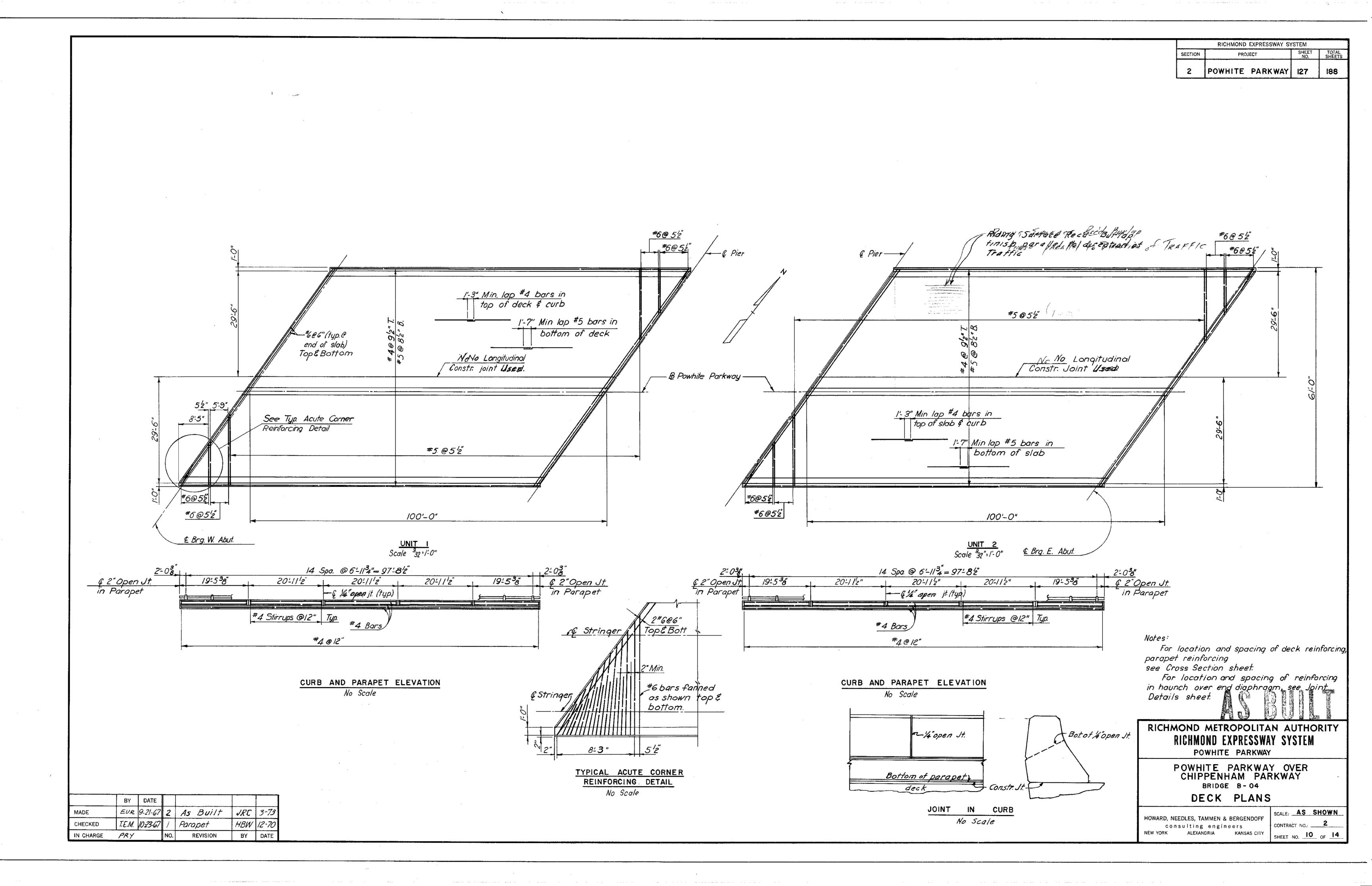
POWHITE PARKWAY OVER CHIPPENHAM PARKWAY BRIDGE B-04 GENERAL NOTES AND QUANTITIES

HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers ALEXANDRIA

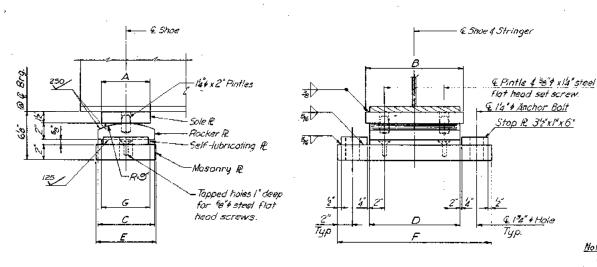






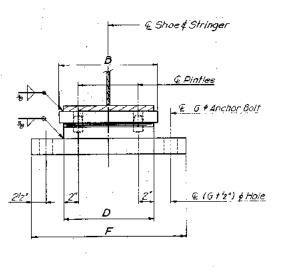


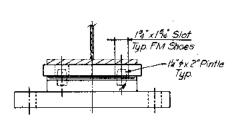
	RICHMOND EXPRESSWAY ST	STEM	
SECTION	PROJECT	SHÉÉT NÚ.	TOTAL SHEETS
2	POWHITE PARKWAY	182	188



14tx2"Pintles Rocker Æ -Masonry R

SIDE ELEVATION





Fixed Modified Shoes same as Fixed Shoes except as shown.

SIDE ELEVATION

END ELEVATION

Sole R is to be beyeled to compensate for grade.

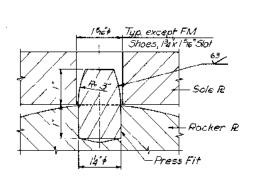
END ELEVATION

FIXED SHOE

END ELEVATION

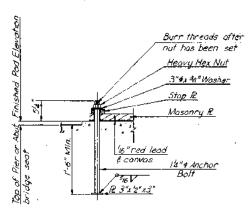
FIXED MODIFIED SHOE



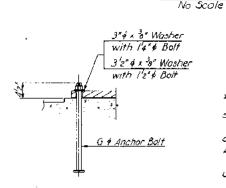


PINTLE DETAIL

Scole: 4"=/"



EXPANSION SHOE



#### FIXED SHOE

#### ANCHOR BOLT DETAIL

No Scale

Anchor Bolf for Fixed Shoes same as Anchor Bolt for Exp. Shoes except as shown.

Shoe Notes:
Top of masonry plates, bottom of rocker plates and top and bottom of sole plates shall be planed, straightened or otherwise treated to secure true plane surfaces. Contact surfaces noted on the plans with finish symbols shall be finished in occordance with the American Standards Association surface roughness requirement as defined in ASA 8461 , Surface Roughness, Waviness and Lay, Port I.

The plates comprising the expansion shoes shall be set so as to be truly centered under full dead load at a temperture of 68°F.

Concrete pads shall be formed integral with abutment or pier and not less than 8" or more than 4" above finished elevation. Dress down pads by rubbing, grinding or as otherwise approved by the Engineer to true level surfaces at the finished elevation.

Anchor bolt assemblies shall conform to AST.M. A-307 and shall be hot-dip galvanized conforming to A.S.T.M. A-153.

Templates shall be used to accurately set the anchor bolts.

Material for shoes (exclusive of self-lubricating plates) shall be high strength low alloy

structural steel conforming to A.S.T.M. Specification A-588,

Moterial for self-lubricating plates shall be Leaded Tin Branze conforming to ASTM. Specification 822, alloy D modified to the extent that 1.5 to 2.5 percent lead is allowable. Shoes shall be included with structural steel item for payment.

						3 H (	0 E	DII	MENS	IONS	_						
		EXP/	ANSIO	N SH	0ES						F	IXED	SHO	ES			
TYPE	NO. REQD.	Δ	В	С	Đ	E	F	Ġ	TYPE	NO. REQD.	Δ	В	С	D	Ε	F	G
E1	29	6"	1.1"	72"	1:0"	8"	1.82	6"	F/	40	6"	1-1"	6"	f-0"	712"	1.9"	14
E2	8	6"	1'-1"	8'2"	1.0"	9"	1:82	62"								:	
E3	3	6"	1-1"	9"	/'-/"	95"	1.92	7"	F4	8	6"	1-7"	6"	1-6"	7"	2.4"	12
E6	16	6"	1'-7"	8"	1-6"	82"	2.25	6"	F4M	8	6	1-7"	6"	1'-6"	7"	2'4"	1/2
																	Ì
												l					

#### RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM POWHITE PARKWAY

STANDARD SHOE DETAILS

Bridges 4 and 5

HOWARD, NEEDLES, TAMMEN & BERGENDOFF CONSULTING ENGINEERS
NEW YORK ALEXANDRIA KANSAS CITY

SCALE AS SHOWN SHEET NO S.L. OF

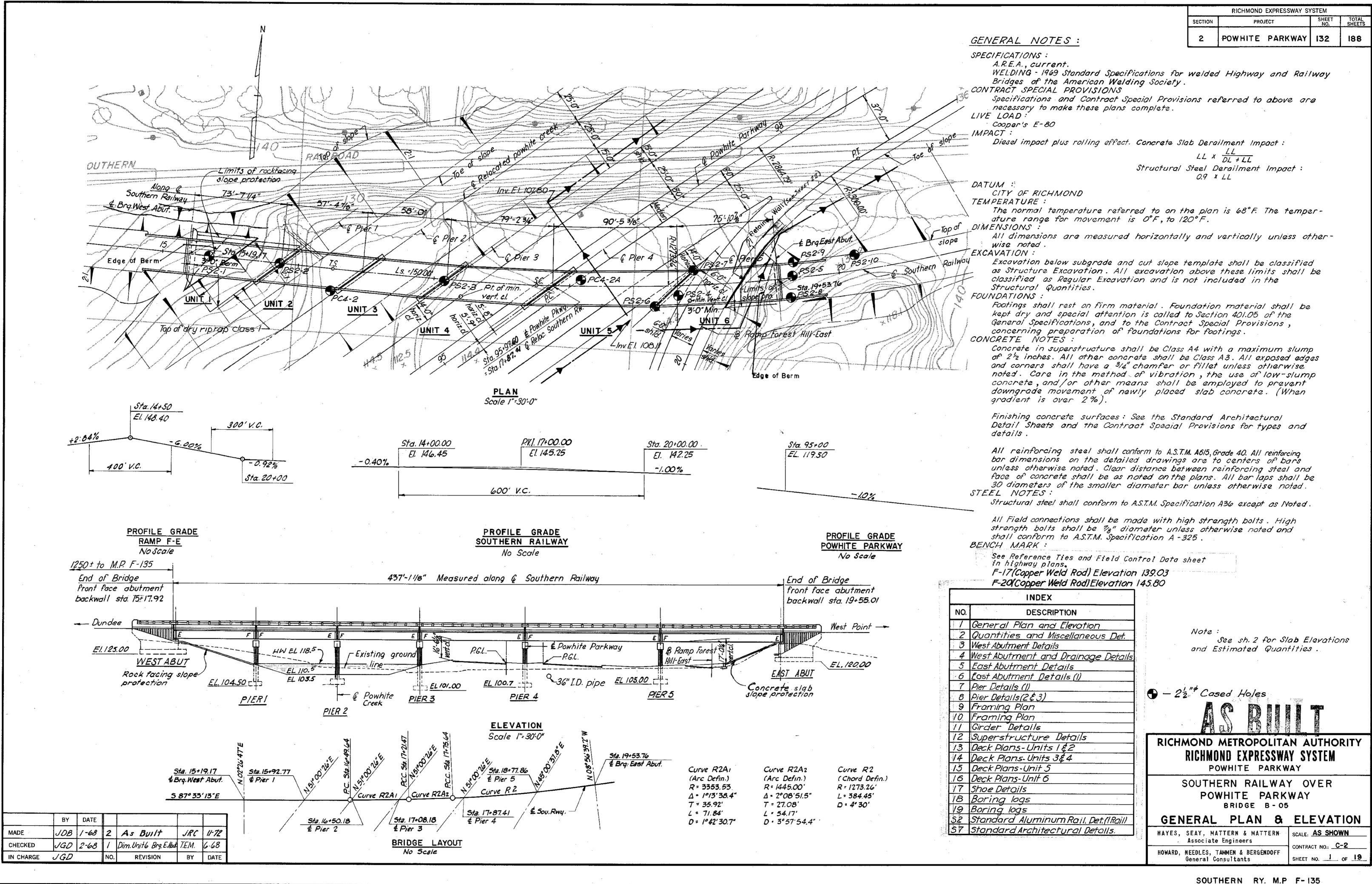
	BY	DATE				
MADE	TEM	1-12-68		101 Broken	٠.	
CHECKED	S.B.P.	3-68	1	General	J.G.V	10-70
IN CHARGE	PRY	·	NÓ.	REVISION	BY	DATE

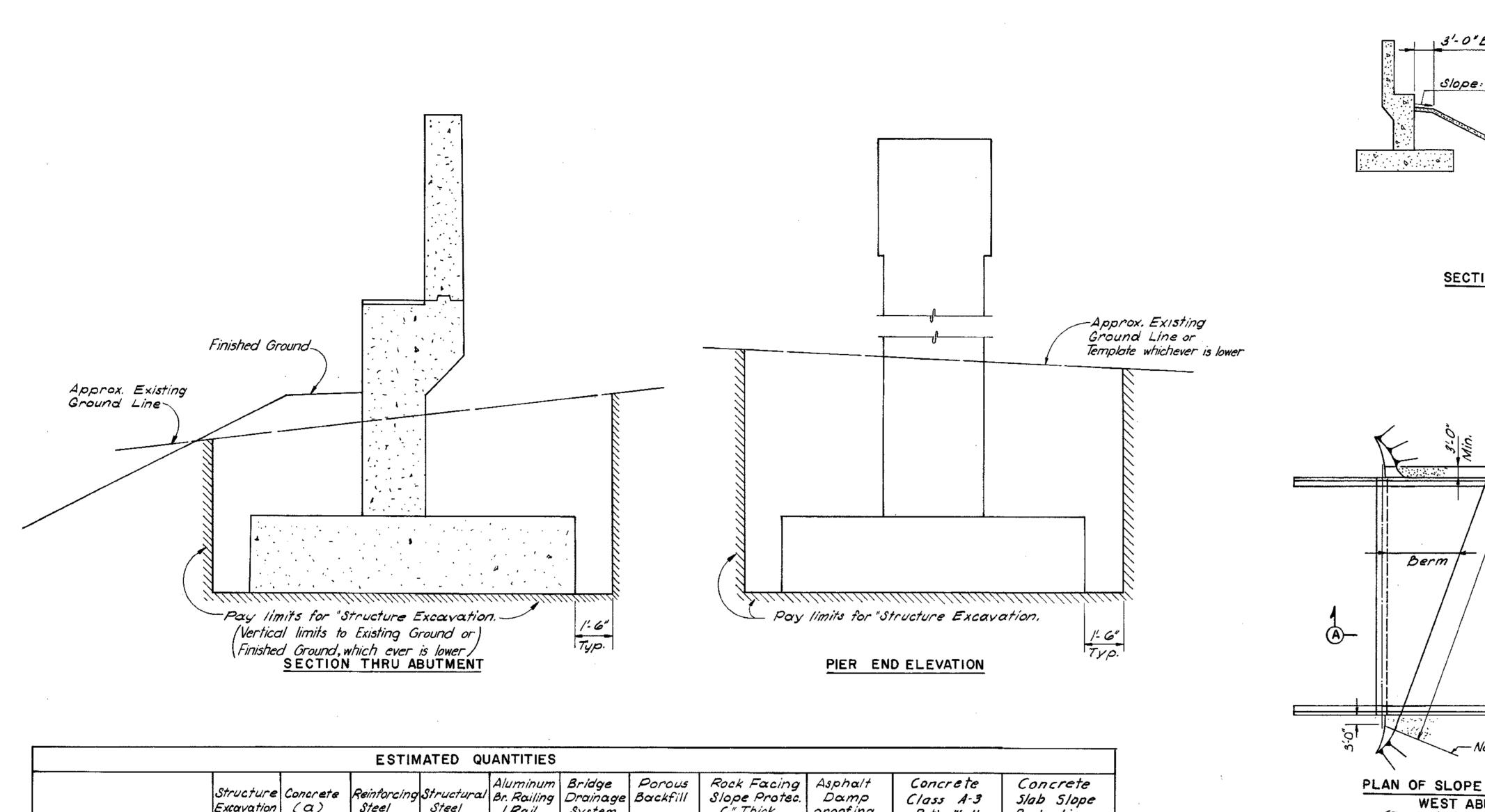
# **Bridge 5**

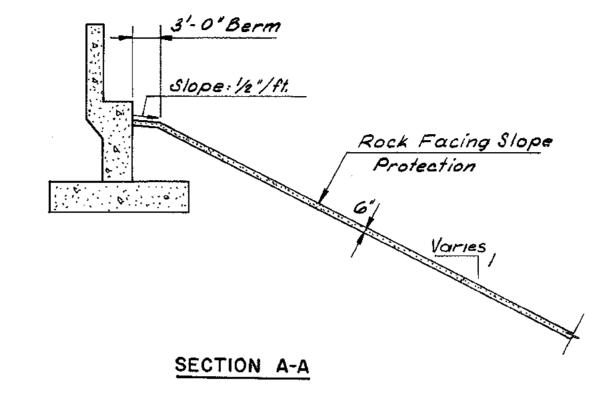
Single Track of Railroad operated by Norfolk Southern Railroad Over

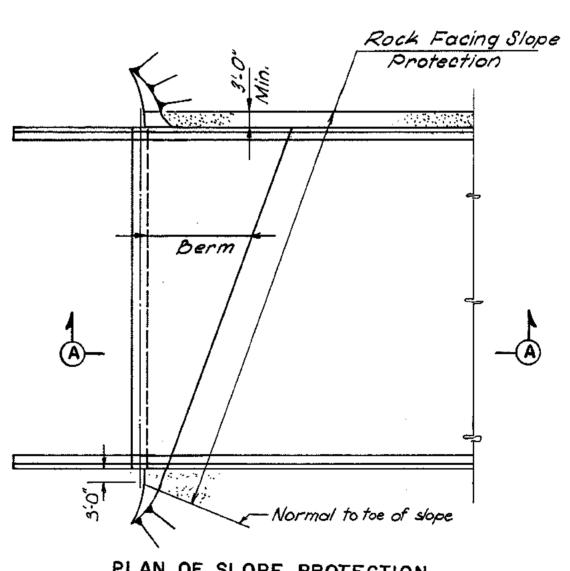
Southbound Powhite Parkway "Cash" Lanes, Southbound Powhite Parkway "Express" Lanes and a Single Lane Ramp from Westbound Forest Hill Ave. to Northbound Powhite Parkway

**Record Set Plans** 









PLAN OF SLOPE PROTECTION
WEST ABUTMENT
(East Abutment Similar)
(East Abutment Similar) with concrete slope protection.
profection.

Rt Curb Elev.

143.98

143.65

143.30

142.92

142.35

141.60

140.76

												l .		
	Structure Excavation C.Y.	Concrete (a) C.Y.	Reinforcing Steel Lbs.	Structural Steel Lbs.	Aluminum Br. Railing I Rail L.F.	Bridge Drainage System L.S.	Porous Backfill C.Y.	Rock Facing Slope Protec. G"Thick S.Y.	Asphalt Damp proofing S.Y.	Concrete Class A-3 Retg. Wall C.Y.	Concrete Slab Slope Protection 5. Y.		;	
		(6)		,	(0)	(d)						Ĺ		
Superstructure		544.88	83,283	1482,037	1,008	,						SLA	AB ELEVA	TIONS
West Abutment	445	225.15	21,598				44	326	102			Location	Lt. Curb Elev.	Crown ELev.
Pier I	304	184.45	40,506									Face of Bkwl. West Abut.	143.98	144.10
Pier 2	/38	/74.28	24,870				-		·			£ Pier /	/43.50	143.69
								,				£ Pier 2	143.13	/43.34
Pier 3	263	203.09	41,816									£Pier 3	142.73	142.94
Pier4	498	264.45	3/4/4				1					g Pier 4	142.13	142.35
											<u> </u>	&Pier 5	141.36	141.60
Pier 5	356	231.15	41,800									Face of Bhwl.	.,,,,,,,	11000
East Abutment	<i>393</i>	241.92	25,359				55		127	17	//3	East Abut.	140.76	140.88
Total	· ·	1,524.49(a)	3/0,646	1.482,037	1,008	,	99	<b>3</b> 26	229	17	//3			

			4	As Built	RJH	11-72
	BY	DATE		General	J.G.V.	10-70
MADE	JOB		<del> </del>	Anchor Bolt set dim.		L
CHECKED	JGD	3-68	7	Quantity	DSB	5-68

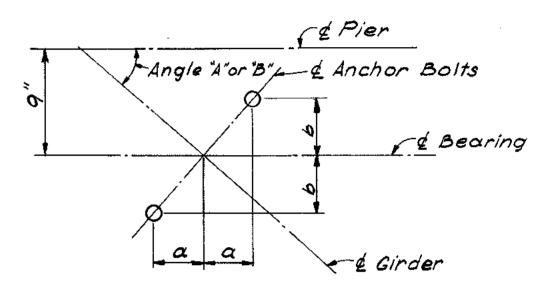
REVISION

BY DATE

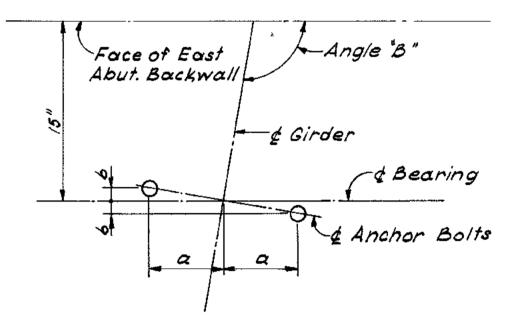
(a) Class A3, Unless noted. (b) Class A4. (c) Includes that portion on abutments & wingwalls.

(d) Includes underdrains

	RICHMOND EXPRESSWAY SYSTEM											
SECTION	PROJECT	SHEET NO.	TOTAL SHEETS									
2	POWHITE PARKWAY	133	188									



PIER ANCHOR BOLT SETTING DIAGRAM Scale = 1/2" = 1'-0"



EAST ABUTMENT ANCHOR BOLT SETTING DIAGRAM

Scale - 1/2" = 1'-0"

	ANCHOR	BOLT SET	TING DIMEN	ISIONS			***
C:/	Angle "A" o	or "B"	Girder	Angle	"A"	Angle	, "B"
Girder	α		Giraer	ď	b	a	5
31-516	75/8"	858"	533	73/6	10/4"	83/16	97/6"
S/7	7/2"	8 4"	534	73/6	10%	8/4"	9%
518-522	フた"	8"/16"	<i>835-</i> 838	7/4"	103/16	8%	938
S23	7 %16"	පි%"	539	7%	10%	85/6"	9%"
524	7 <b>5%</b> "	ර <del>ද</del> ේ	540	78"	101/6"	8%	94"
<i>S25</i>	7340	9"	541-546	71/6	916	11/2"	0%
S 26	7/4"	81%	347 <del>i</del> S48	75/8"	9%"	1'-01/2"	9%"
\$27-\$3/	74"	8 %.					
<i>\$32</i>	75/6"	878"					
							•
<u> </u>						:	
					,		



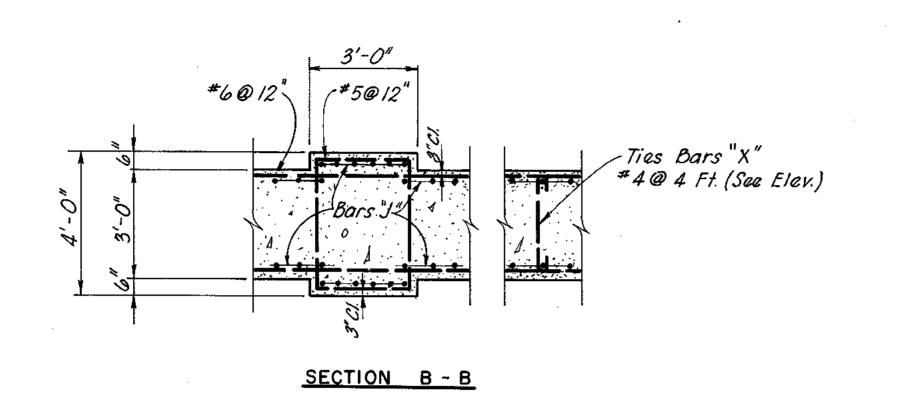
## RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

POWHITE PARKWAY

SOUTHERN RAILWAY OVER POWHITE PARKWAY BRIDGE B-05

# QUANTITIES & MISC. DETAILS

HAYES, SEAY, MATTERN & MATTERN SCALE: AS SHOWN
Associate Engineers CONTRACT NO.: 2 HOWARD, NEEDLES, TAMMEN & BERGENDOFF General Consultants SHEET NO. 2 OF 19



TYPICAL ELEVATION

TEM. 4-68

T.E.M. 6-68

RH 11-72

BY DATE

BY DATE / Note added

THN 2-68 3 As Built

MADE

CHECKED

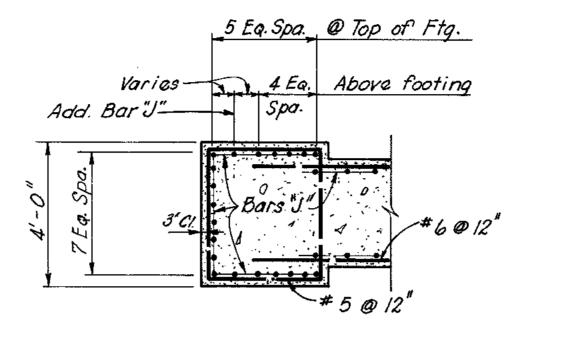
IN CHARGE

JGD

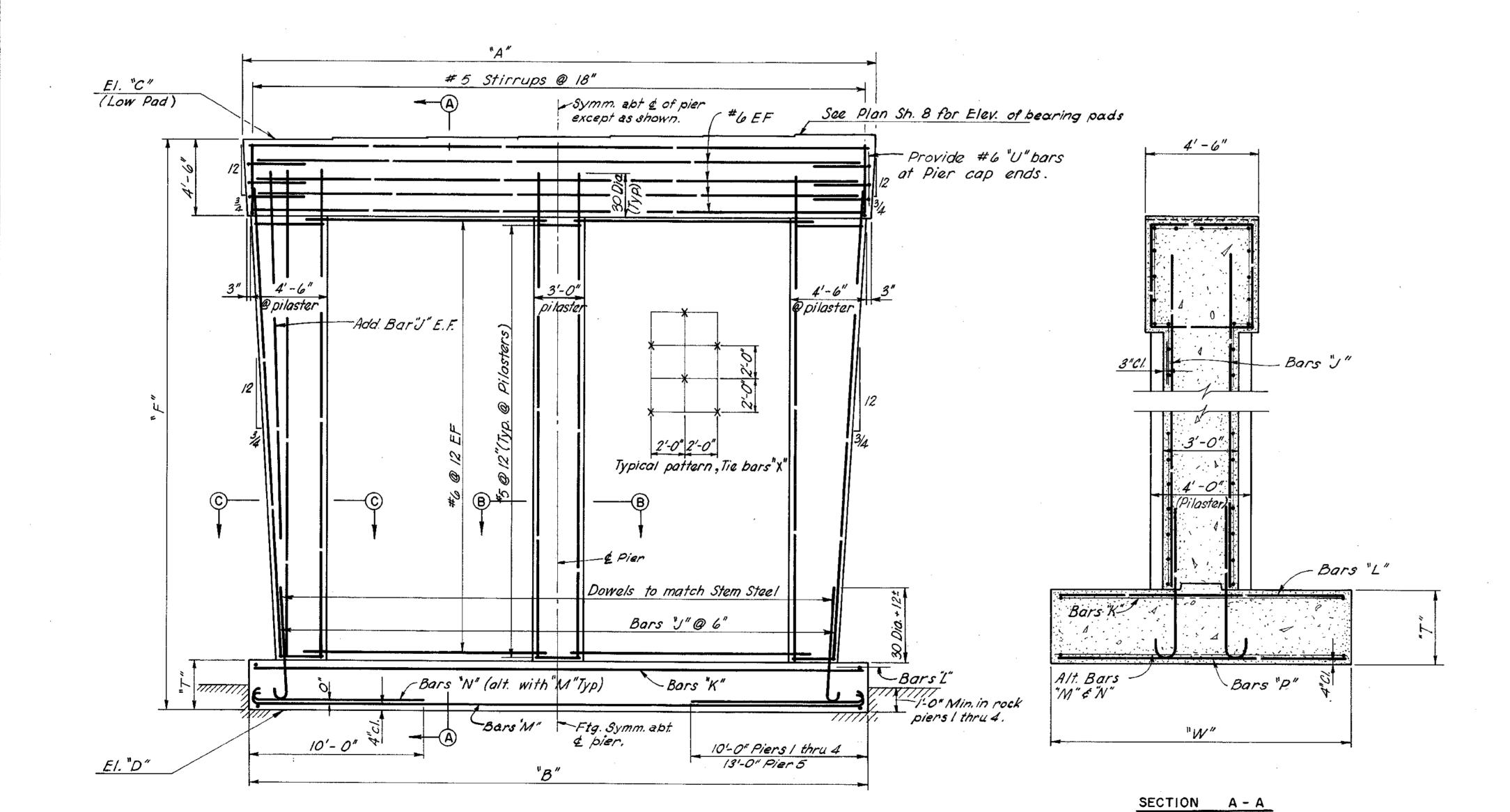
1-68 2 Pad El. \$ Dim.

REVISION

(Looking ahead on stationing)



SECTION C - C



RICHMOND EXPRESSWAY SYSTEM 2 POWHITE PARKWAY 138

Woke to Contractor

Attention is called to the apparent erratic location of the top of rock in the vicinity of Pier 5. If, after excavation for Pier 5, the majority of founding material located at Elevation 105 is rock, all pockets of decomposed material within the limits of the excavation shall be removed and backfilled with Class A3 concrete. Conversely, if the majority of founding material is decomposed rock. areas of projecting bedrock shall be removed to an elevation one foot below the footing elevation and backfilled to the footing elevation with a blanket of well compacted granular material similar in nature to the majority founding material.

Any additional excavation required by the above will be paid for at the unit price bid for Structure Excavation. Additional Class A3 concrete required, amounting to toubic yard or more, will be paid for at the unit price bid for Concrete, Closs A3, Substructures. No additional payment will be made for backfilling with granular material.

PIER	ELE	VATION		DIV	•	AL L (2) AL		
PIER	11C"	" <i>D"</i>	"A"	<i>"B"</i>	"F"	"W"	"7"	ALLOW.
/	136.07	104.5	37-9"	37-0"	31.57	12'-0"	3'-6"	7 ton
2	/35.52	103.5	37'-9"	37'-0"	32.02	9'-0"	3'-6"	7 ton
3	/35.3/	101.0	38'-6"	40'-0"	34.31	12'-0"	3'-6"	6 ton
4	134.66	100.7	40'-6"	45'-0"	33.92	17'-0"	4'-3"	4 ton
5	133.87	105.0	39'-0"	45'-0"	28.87	17'-0"	4'-0"	4 ton
PIER		EINFOR	CING :	STEEL				
FILK	" <i>J"</i>	"K"	" <i>L</i> "	"M"	"\\"	"P"		
/	#//	#5@18	#6012"	#60/4"	#6@14"	#10@6"		
2	#9		_	#6016"	#6@16	#707"		
3	#//	#5@18	<b>*</b> 6@12".	#8@16 <sup>"</sup>	#8 <b>@</b> 16	#9 <b>@</b> 8"		
4	#10	_	_	#6@15"	#6@15"	#9@8"		
5	#9	_		#9 <i>@14</i> "	#9@14"	#9@7"	* 13' 1	ong

NOTE: Footing elevations are approximate only and may be varied to suit field conditions as directed by the Engineer Vertical shaft reinforcing shall not be cut until these elevations are established. Where elevations change more than 2 ft., redesign will be required.

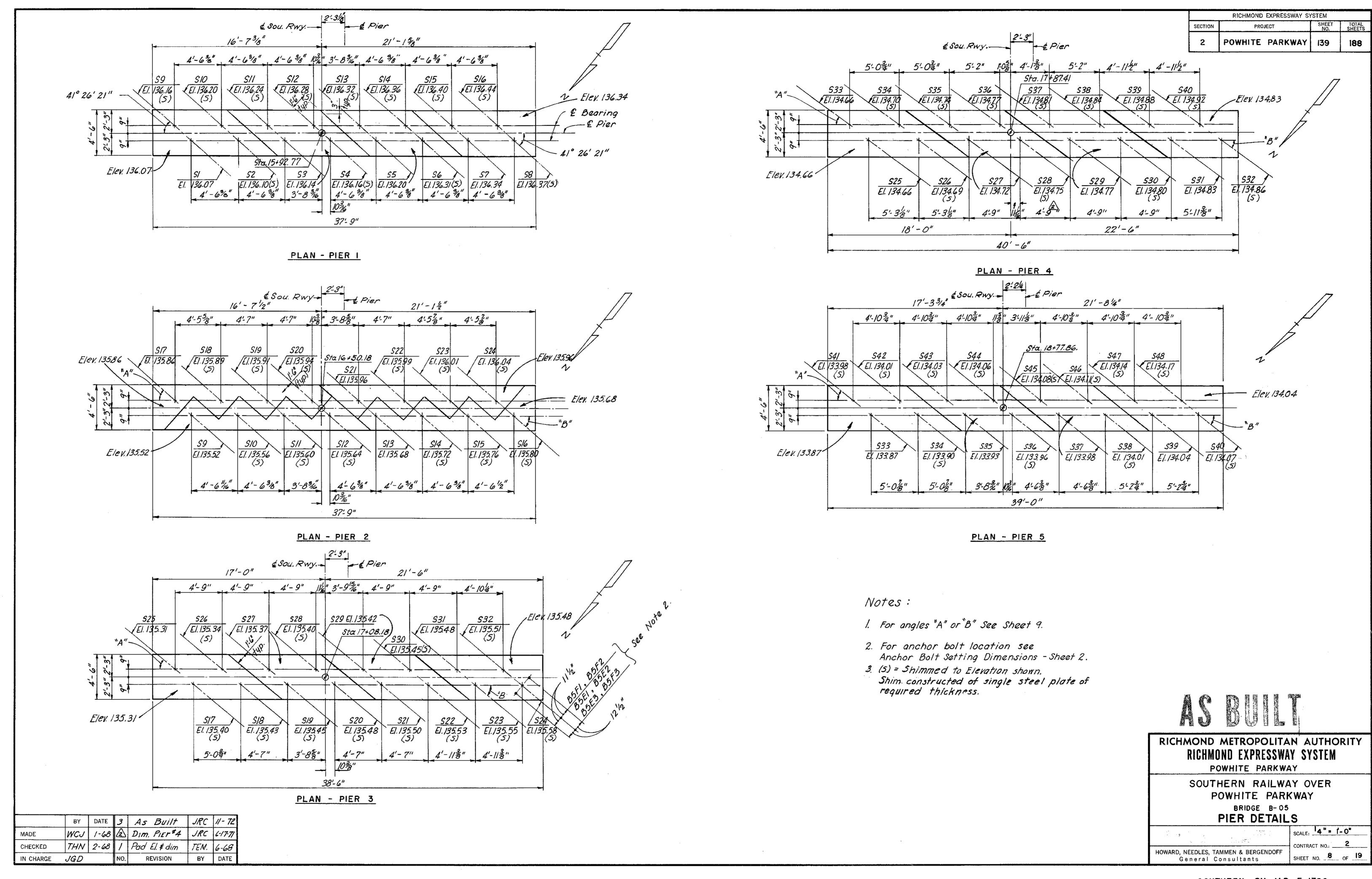
## RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM POWHITE PARKWAY

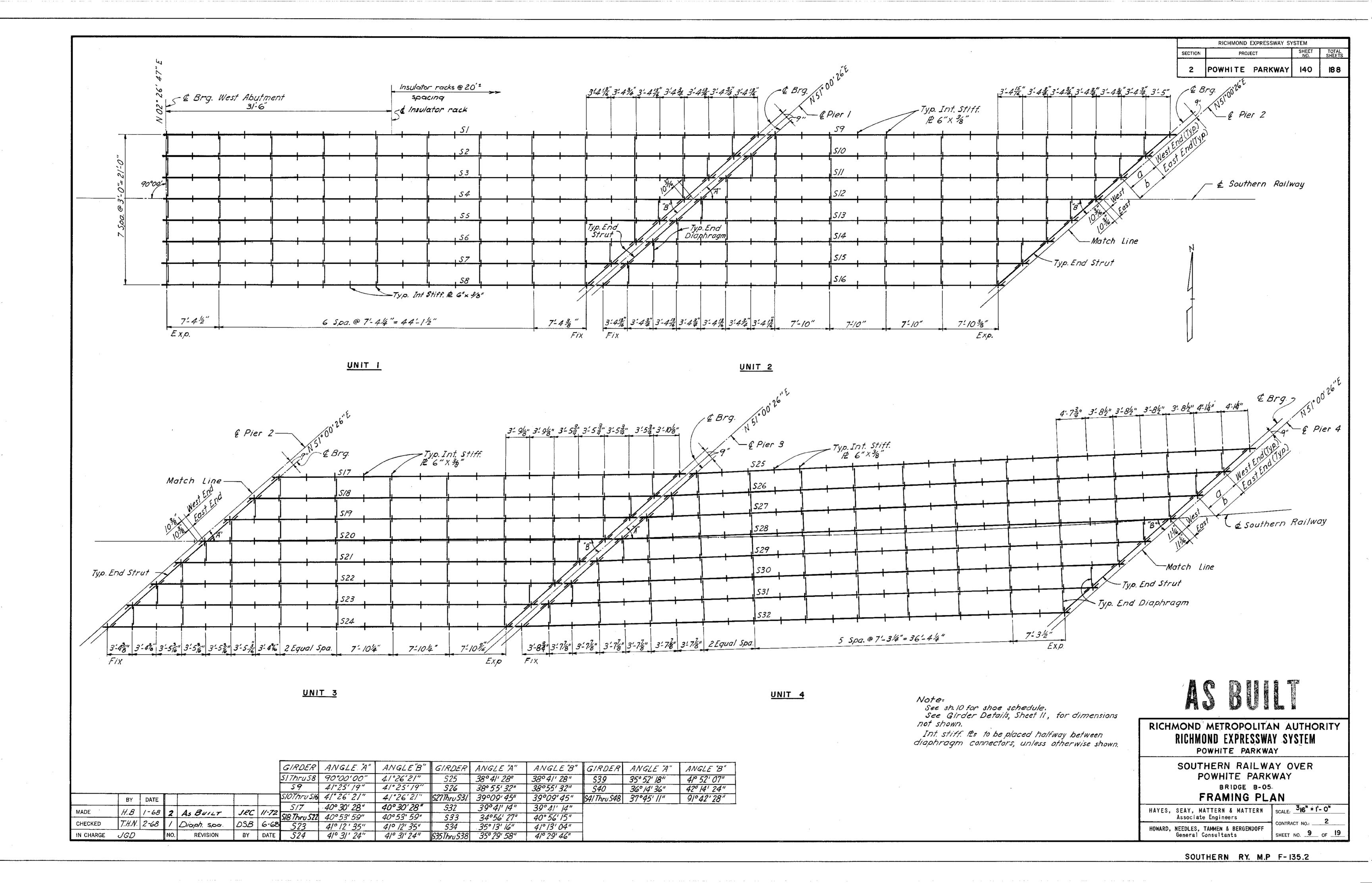
SOUTHERN RAILWAY OVER POWHITE PARKWAY BRIDGE B-05

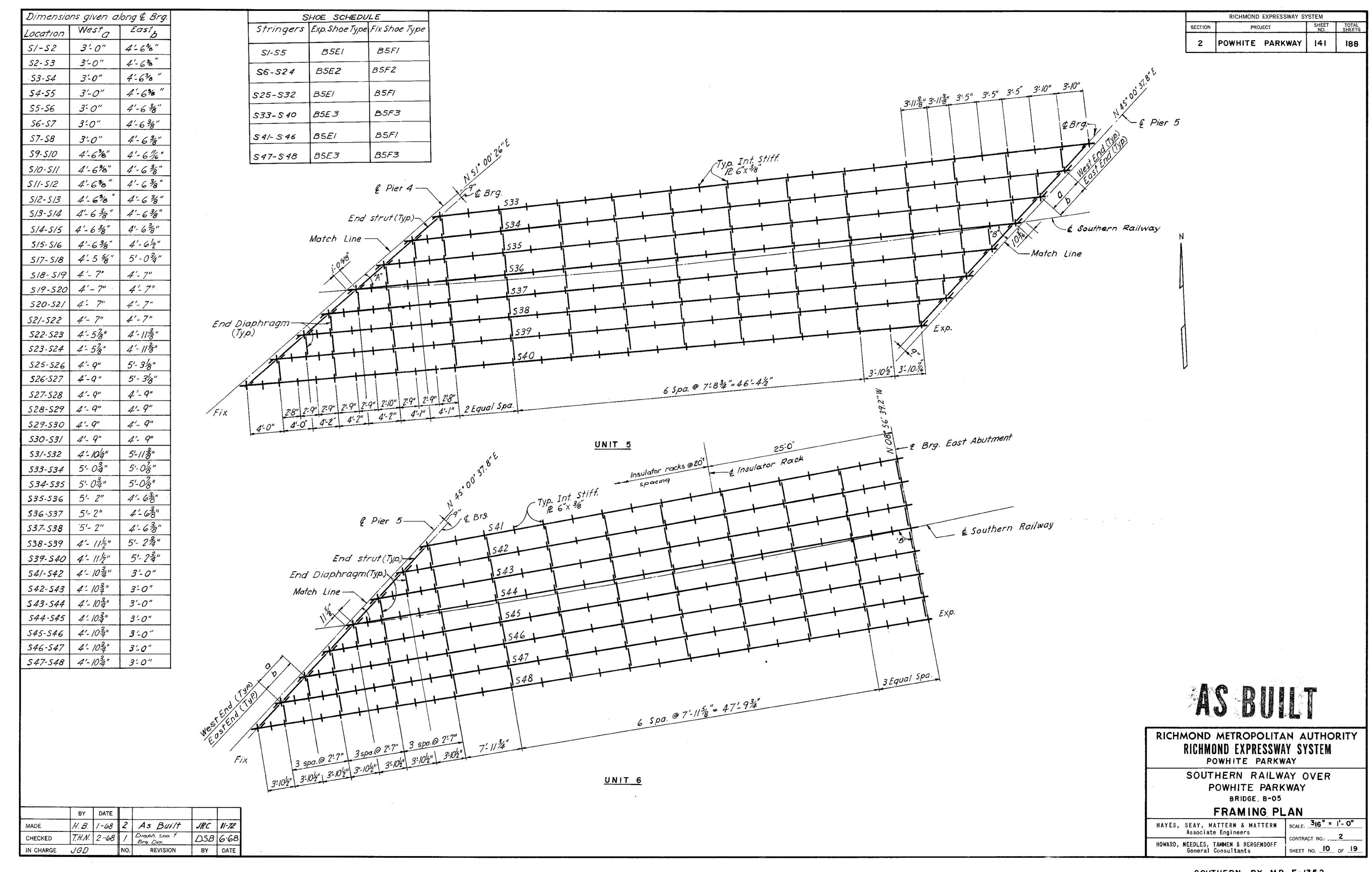
# PIER DETAILS

HAYES, SEAY, MATTERN & MATTERN Associate Engineers

SCALE: 38" = 1'-0" HOWARD, NEEDLES, TAMMEN & BERGENDOFF SHEET NO. 7 OF 19 General Consultants







		<u> </u>		RDER	SUN		<del></del> -	<u>.</u>		<u> </u>	
Girder			<i>A</i>		. A T E	B		nension		Brg. Stift	
Mark	5/3e	Length	Weld A	Size	Length	Weld B	С	D	E	5/3e	
5/	18" x 1 7/8"	16'-93/4"		18" x 2 3/4"		1/2	/'-3"	82' - 7 7/8"	1'-15/8"	8"x/"	
52	18"x 134"	16'-10%	3/8	18"x 2 '2"	47'-6"	1/2		79'-316"			
S-3	18"x 1/2"	15'-176"	5/16	18" x 24"	47-6"	3/8		75'-105%			
5-4	18"x 1/2"	16'-5 %	5/16	18"x 218"	41'-6"	3/8		72'-5'2"			
\$ <i>5</i>	18%/4"	14-98	5/16	18"X1%"	41'-6	3/8		69'-034"			
56	18"x/8"	14'-334"	5/16	18"x1%"	39'-0"	3/8		65'-718"			
<i>\$7</i>	18"x1"	13-73"	5/6	18"x12"	37:0"	5/6	<b>*</b>	62'-33/6"	<b>†</b>	•	
58	18"x %"	13'- 2"	5/16	18"x /4"	34-6"	5/16	1'-3"	58'-103"	1-15/8"	8"x/"	
59	18"x 1/8	14'-08"	5/16	18"x 1"8"	29'-0"	5/16	1'-18"	55'-25%"	1'-158"	8"x 3/4"	
510-516	18"x %"	14'-0'2"	5/16			5/16	1'-18"	55'-113"	1-158"		
5/7	18"x %"	14'-7"	5/6			5/16	1'-1%"	56-24"	1-18"		
5/8	/8"x %"	14'-43/6"	5/16			5/6	1'-134"	55'-8 <b>%</b> "	1'-13/4"		
5/9	18"x %"	14'-43/6"	5/16			5/16	1'-134"	55'-87's"	1'-13/4"		
520	18"x 7/8"	14-4 3/6"	3/16			5/16	1'-134"	55-8%"	1'-134"		
52/	18"x %"	14'-4 3/6"	5/16			5/16	1'-13/4 "	55'-8%"	1'-1%"		
522	18"x 7/8"	14'-43/6"	5/16			5/16	1'-13/4"	55'-8%"	1'- /3/4"		
523	18"x %"	14'-2"	5/16	+	+	5/16	1'-1%	55'-43/4"	/-1%"	<b>V</b>	
524	18"x %"	13-11%	5/16	18"x1"8"	29-0"	5/16	1'-1%"	55'-0%"	1-136"	8"x 3/4"	
525	18"x15g"	16'-315"	3/8	18" x 238"	47'-0"	1/2	1'-238"	77'-7%	1'-2%"	8'x %"	
526			3/8	18" x 23/8"	47-0"	1/2	1'-23/6"	77:2%	1'- 2 5/6"	8"x 78"	
527		15'-1146"				1/2	1'-24"	76'-9%"	1'-24"		
528		15'-1116"	3/8			1/2	1'-24"	76'-9%"	1'-24"		
529		15'-1116"				1/2	1'-24"	76'-9%"	1'-21/4"	<u> </u>	
530		15'-1116"				1/2	1'- 24"	76'-9%	1'-24"		
53/	•	15'-1116"	3/8	•	•	1/2	1'-24"	76'-9 %	1-24"		
532	18"x 15°	15'-5%	3/8	18"x 23;"	47'-0"	1/2	1'-2'8"	75-11/2"	1'-2'8"	8"x 78"	
533	20"x 2"		3/8	20"x27"	54'-0"	1/2	1'-334"	86'-6'2"	1'-134"	9"x/"	
534		17-5%	3/8		<u> </u>	1/2	1'-35%"	86-10%	1'-15/8"	1	
\$35		17-7%"	3/8		•	1/2	1'-31/2 "				
S36		18'-0%"	3/8	20"x 2 %	54'-0"	1/2	1'-31/2"	88'-0"	1'-158"	9"x/"	
537		16'-11 3/6"	3/8	20"x3"	57'-0	1/2	1'-31/2"	88'- 9%"	1'-15/8"	1	
538		17'-45%	3/8	1	1	1/2"	1'-31/2"	89:7/2 "	/'- / <sup>5</sup> /8"		
539	•	17-4156	3/8		1	1/2	/-3% "	89'- <b>9</b> "	1'-1/2"		
540	20'x2"	17'-5%	<u>/8</u>	20"x 3"	57'-0"	1/2	1'-3'4"	89'-10%"	1'-1%"	9"x1"	
541	18'x 1'8"	14'-2'2"	5/6	18" x 156"	36'-6"	1/2	1'-258"	62:938"	1'-3"	8"x/"	
542			5/16	18"x 134"	40'-0"	3/8	1'-258"	66'-8"s	/-3"	1 7/	
543	-	14'-8%"	116 5/16	18"x2"	43'-6"	*8 3/8	1'-25/8"	70'-8%	/'-3"		
544 544		15'-178"					1-258"	74'-8%"	/-3" /-3"		
545			3/8	18"x2"4"	· · · · ·	3/8					
			<u> </u>	18"x2"2"	47'-6"	1/2	1'-25%"	78'- <b>75</b> 8"	/-3"	2" "	
546		16'-6'5%	3/8	/8"x234"		1/2	1'-25"	82'-64"	1-3"	8"x/"	
S47	20"x1%"	1/-/16	3/8 3/8	20"x 23/4"	53'-6"	1/2	1'-25/6"	86'-634"	/'-3"	9"x /"	

			5/16	p. Inter. Stiff.	
Plate	A	Plate B		Plate A	
45°	66" x 7/16"	Web-s B		$A \rightarrow$	
c = £	Tight Fit  Mill to Bear  Bearing	Dimension D	7	Tight Fit £ Bearing —	E
	e of Bkwl. or É Pie	A CONTRACTOR OF THE CONTRACTOR			→

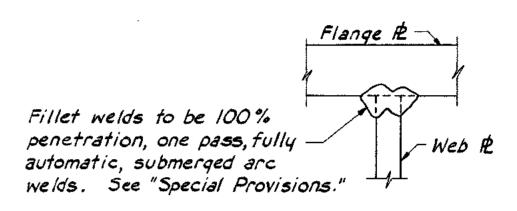
GIRDER DETAIL

No Scale

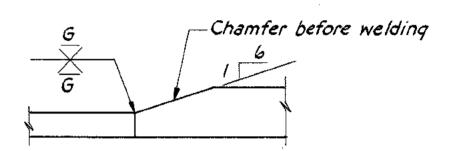
	AD DEFLECTION
GirderMark	Total Dead Load Defl. @ & Girder
51,52	1/2"
83	7/16"
S 4	3/8"
85,86	5/16"
87,88	1/4"
59,324	3/16"
325,332	7/16"
533, 538	9/16"
<i>\$39,540</i>	5/8"
<i>\$41</i>	14"
842	5/16"
\$43, \$44	3/8"
S 45	7/16"
346	1/2"
347	9/16"
548	5/8"

The above deflections are those anticipated to occur in the girder upon placement of the total dead load. The girders shall not be cambered to compensate for this deflection.

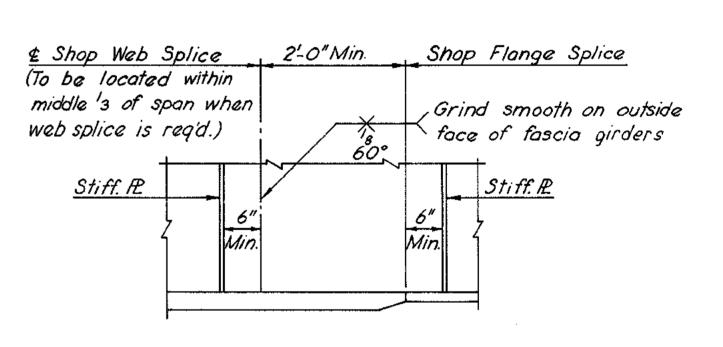
RICHMOND EXPRESSWAY SYSTEM											
SECTION	PRO.	SHEET NO.	TOTAL SHEETS								
2	POWHITE	PARKWAY	142	188							



FLANGE PLATE WELDS
No Scale



FLANGE THICKNESS TRANSITION
No Scale



SHOP SPLICE DETAILS

Scole: 34"=1-0"



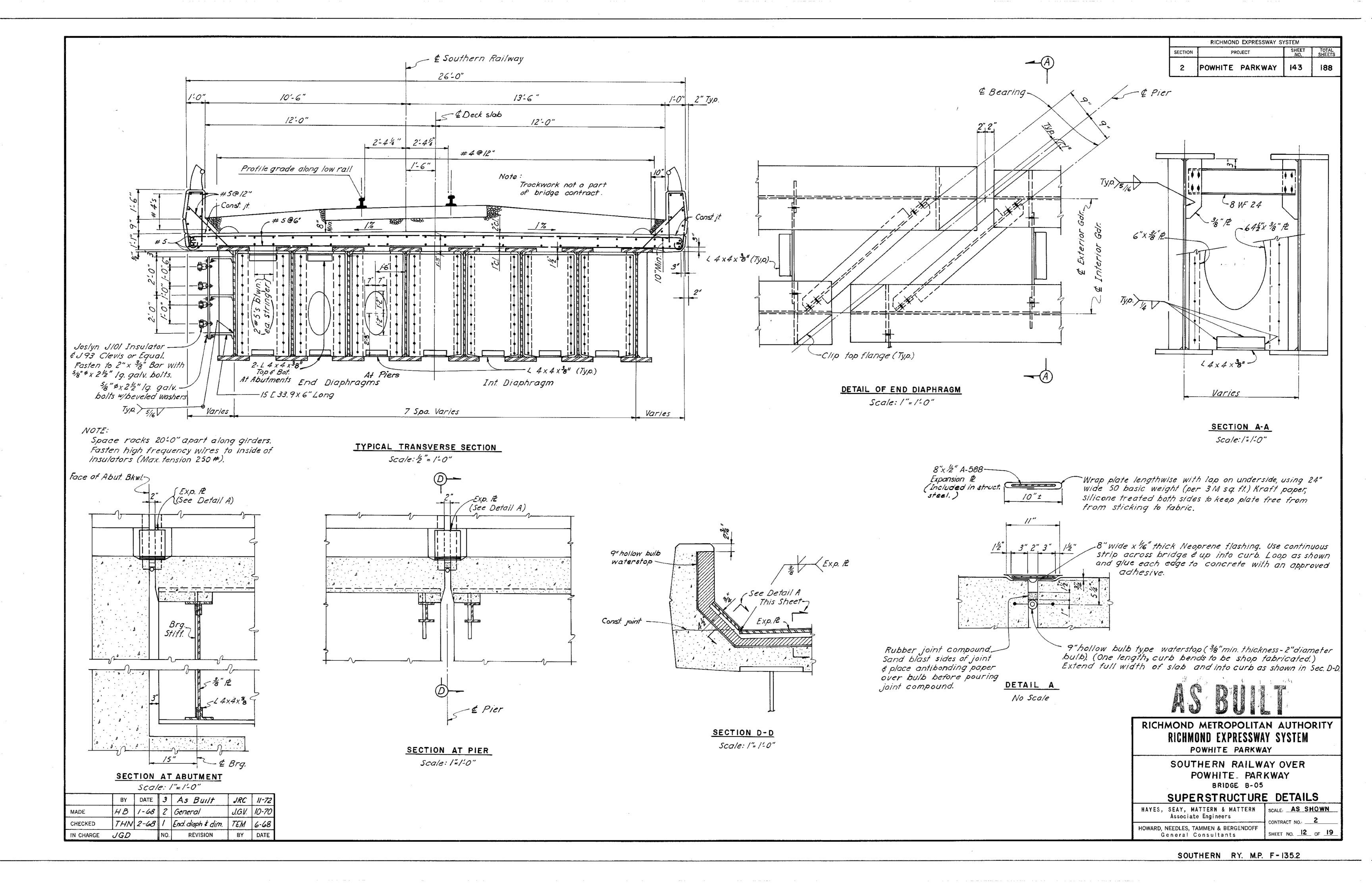
# RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM POWHITE PARKWAY

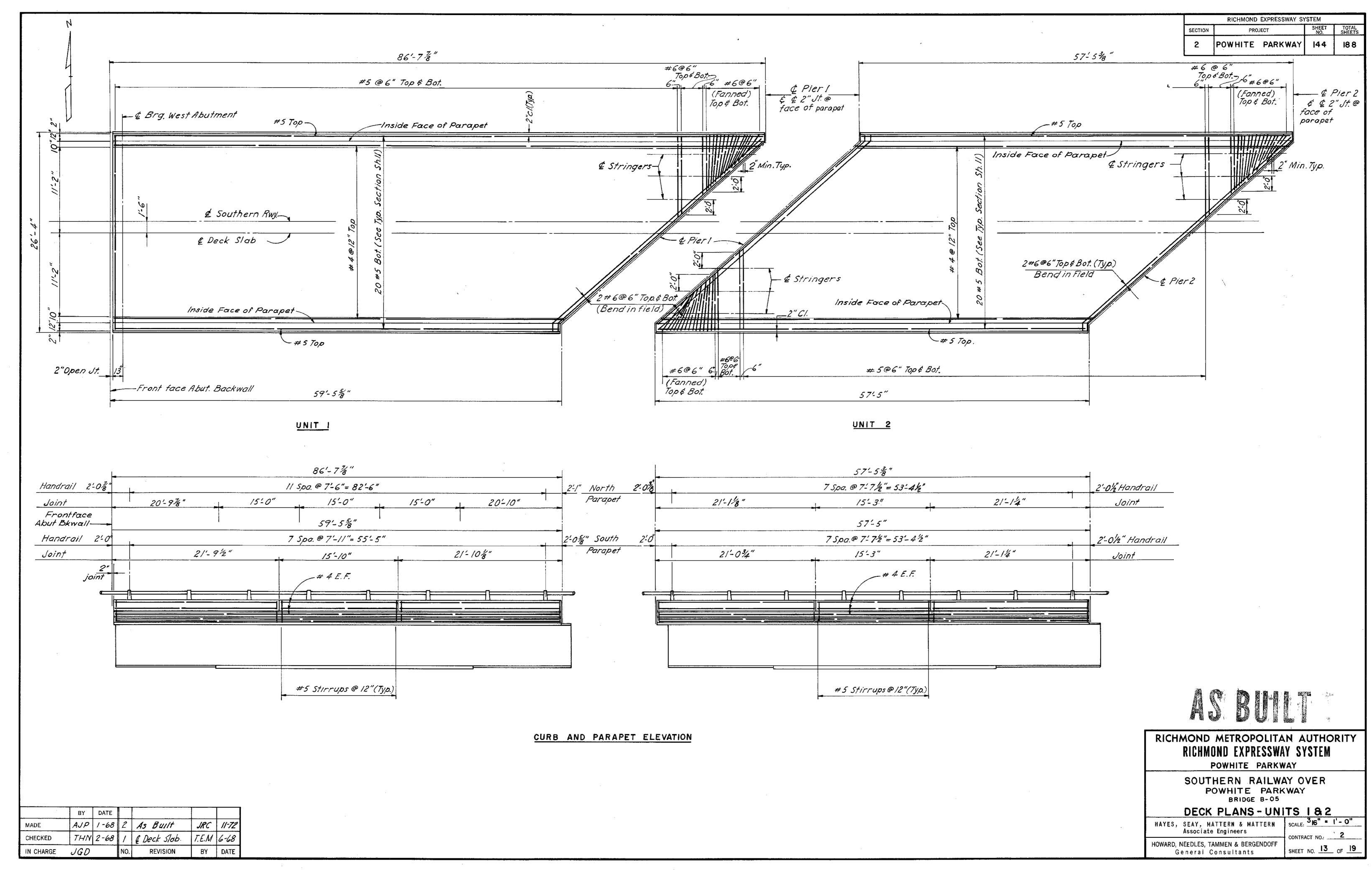
SOUTHERN RAILWAY OVER
POWHITE PARKWAY
BRIDGE B-05

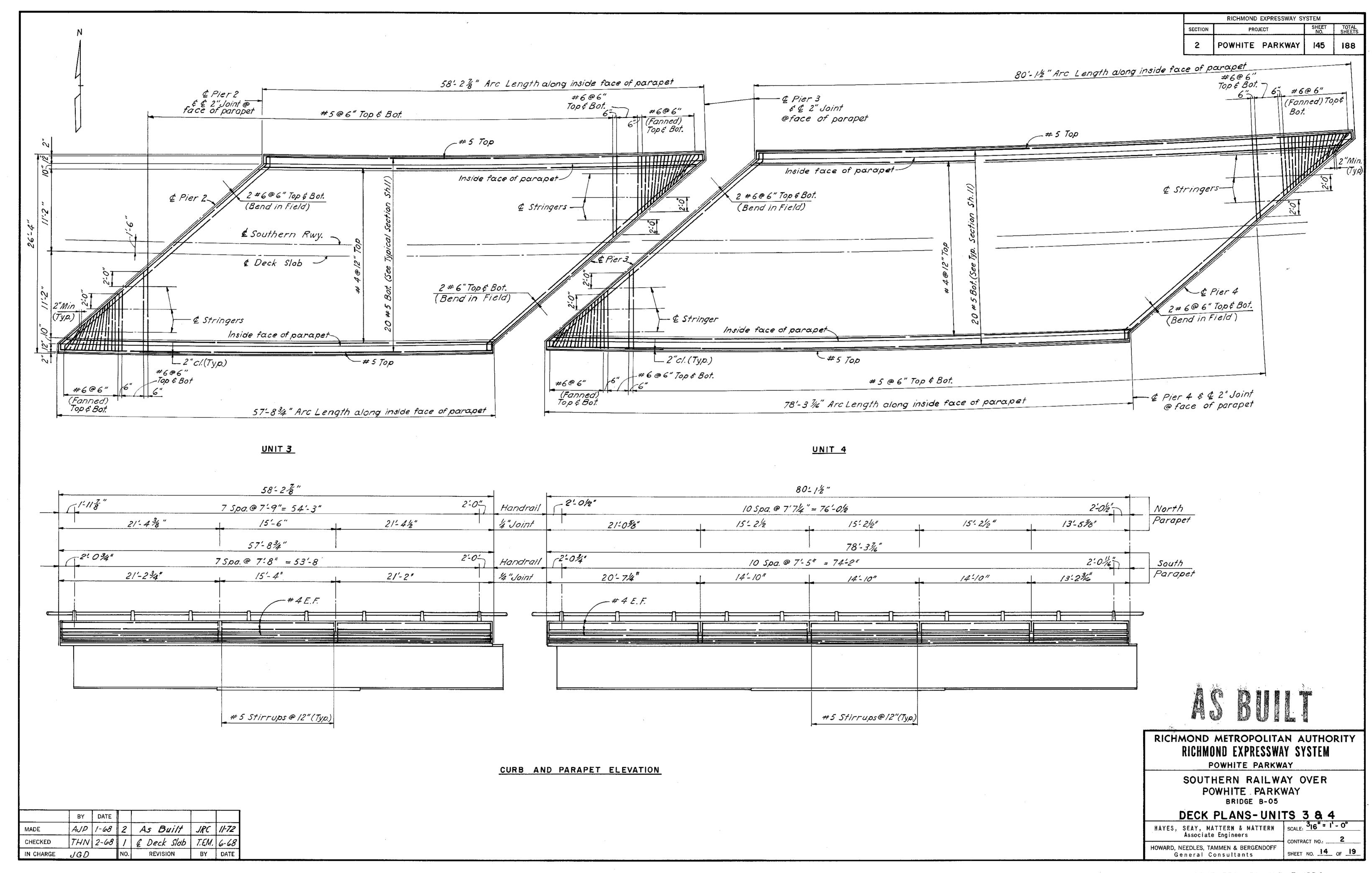
# GIRDER DETAILS

HAYES, SEAY, MATTERN & MATTERN SCALE: NO SCALE
Associate Engineers

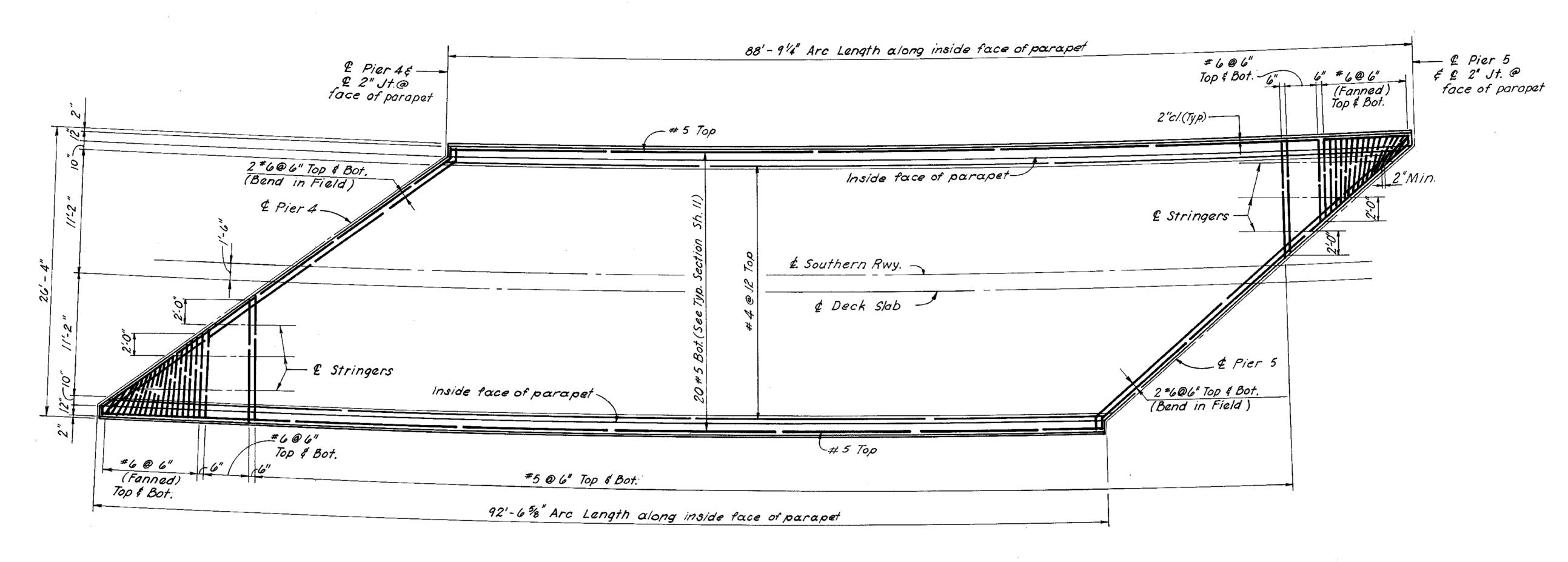
	BY	DATE				
MADE	H.B.	1-68	2	As Built	JRC	11-72
CHECKED	THN	2-68	1	Girder Summary	T.E.M.	6-68
IN CHARGE	JGD		NO.	REVISION	BY	DATE



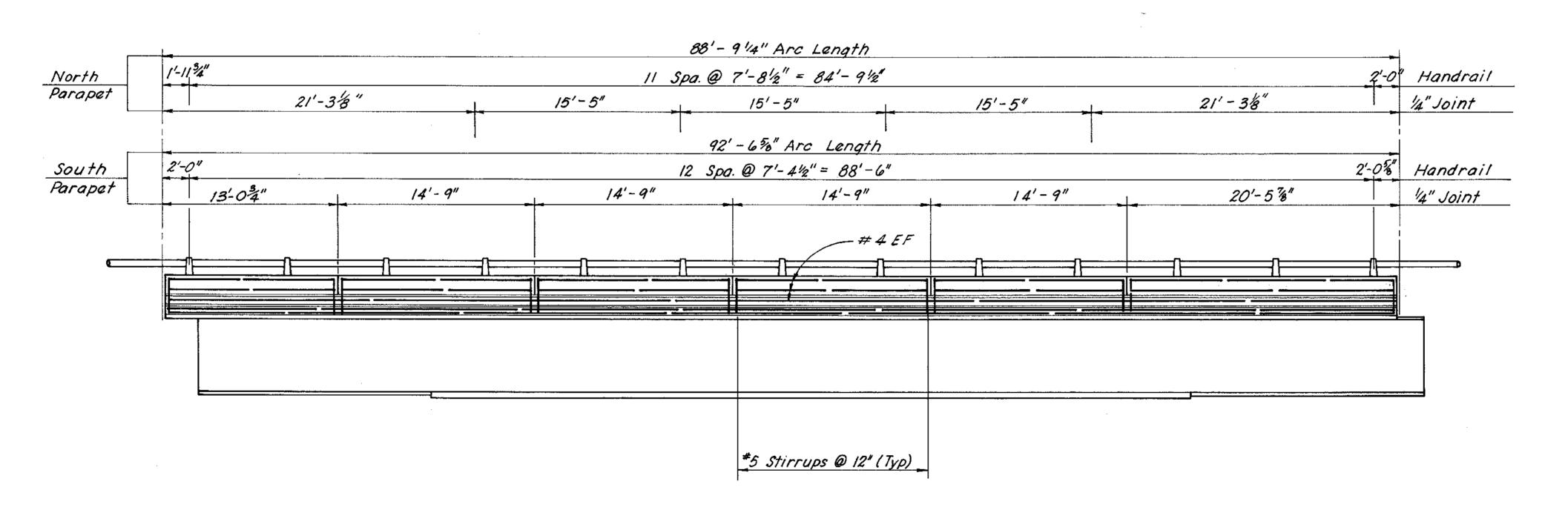








## UNIT 5



#### CURB AND PARAPET ELEVATION

	BY	DATE				
MADE	AJP	1-68	2	As Built	JRC	11-72
CHECKED	THN	2-68	/	& Deck Slab	TEM	6-68
IN CHARGE	JGD		NO.	REVISION	BY	DATE

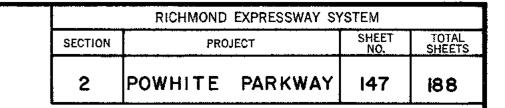
# RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

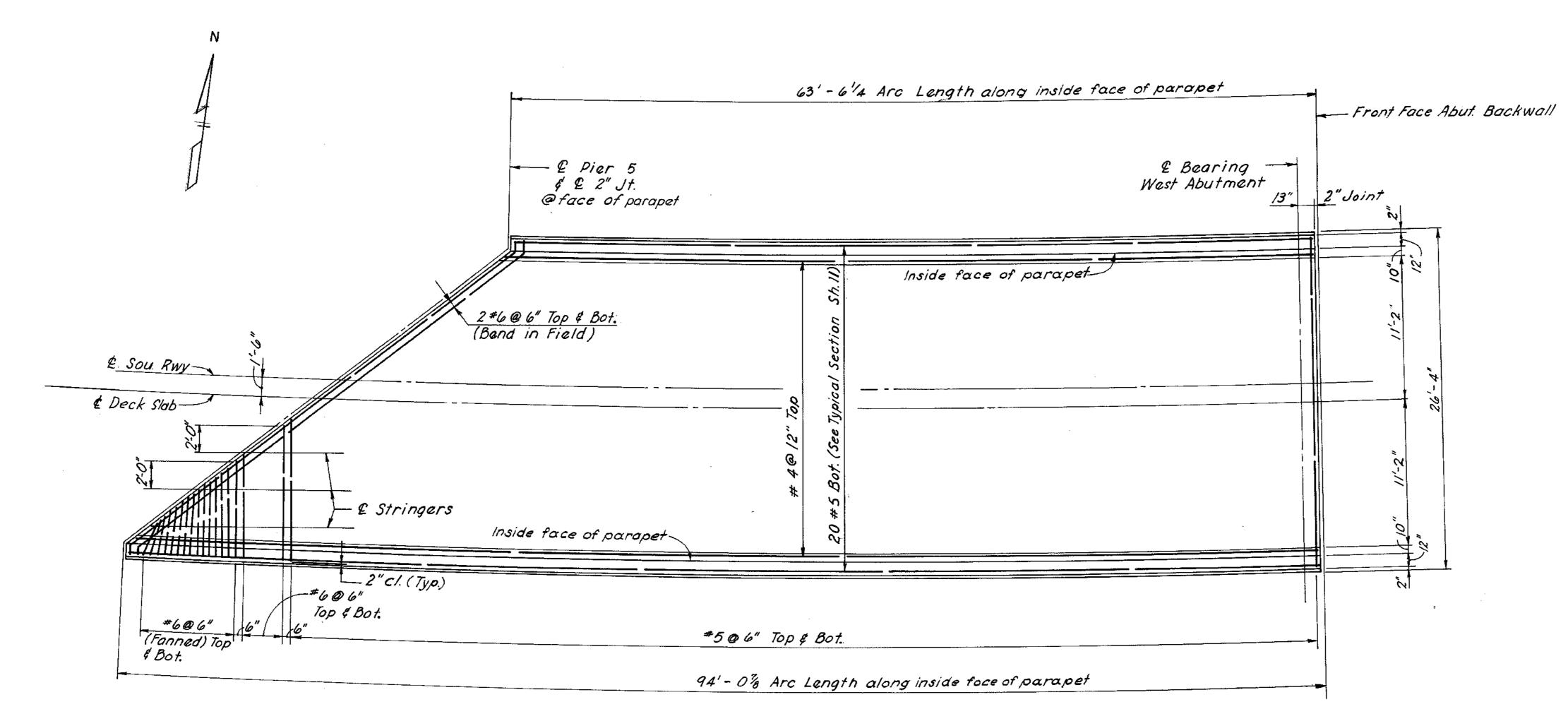
POWHITE PARKWAY

SOUTHERN RAILWAY OVER
POWHITE PARKWAY
BRIDGE B-05

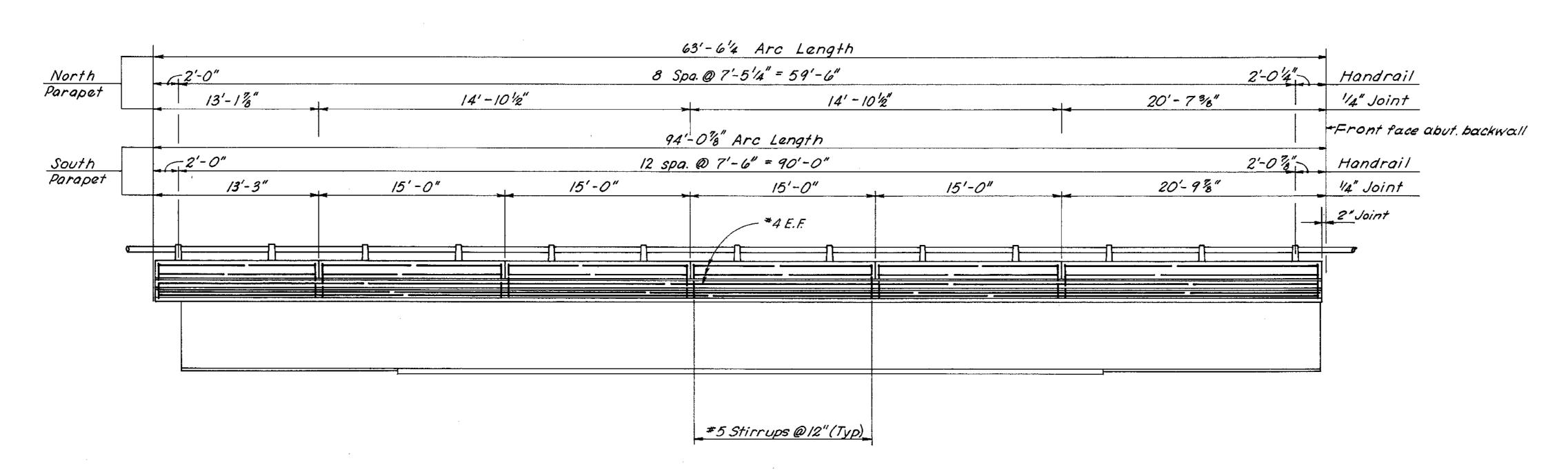
D	E	C	K	F	<u> </u>	Ą	N	S	-	U	1	V	<u> </u>	T	

=======================================	*
HAYES, SEAY, MATTERN & MATTERN	SCALE: 316" = 1'-0"
Associate Engineers	CONTRACT NO.: 2
HOWARD, NEEDLES, TAMMEN & BERGENDOFF	SHEET NO. 15 OF 19





UNIT 6



CURB AND PARAPET ELEVATION

AJP 1-68 2 As Built

CHECKED

IN CHARGE

JGD

THN 2-68 / @ Deck Slab TEM. 6-68

JGD NO. REVISION BY DATE

R.J.H. 11-72

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

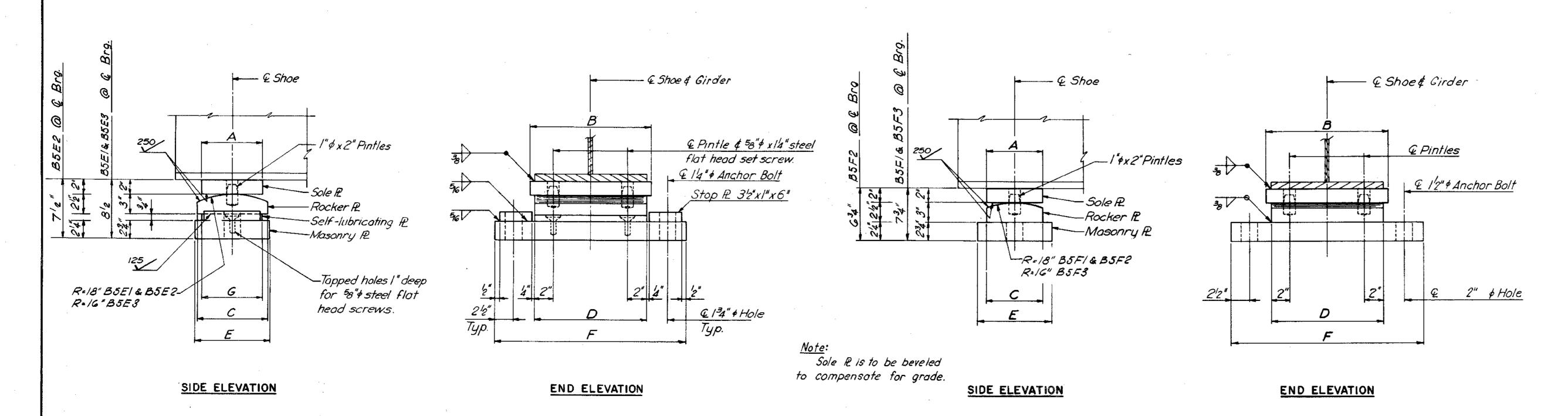
POWHITE PARKWAY

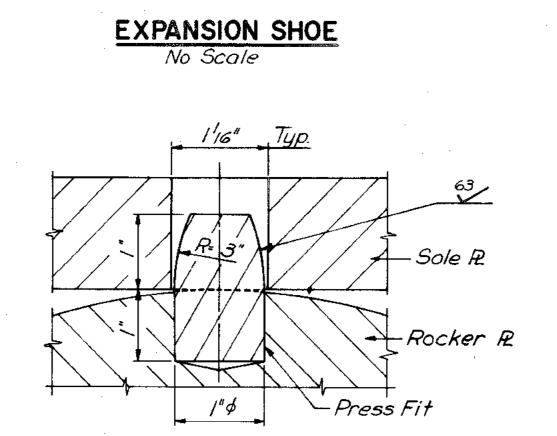
SOUTHERN RAILWAY OVER POWHITE PARKWAY BRIDGE B-05

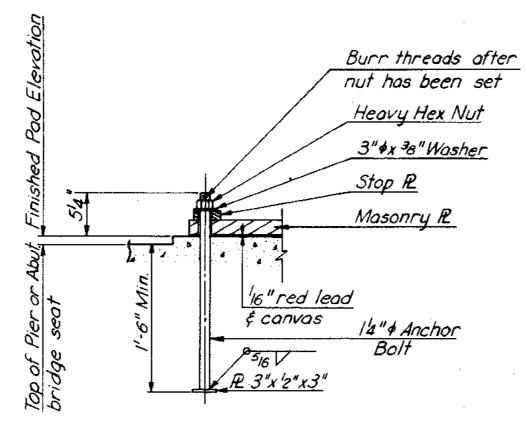
DECK PLANS-UNIT 6 HAYES, SEAY, MATTERN & MATTERN Associate Engineers

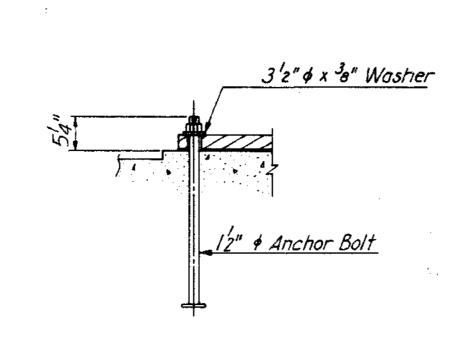
HOWARD, NEEDLES, TAMMEN & BERGENDOFF General Consultants SHEET NO. 16 OF 19

RICHMOND EXPRESSWAY SYSTEM 148 POWHITE PARKWAY









FIXED SHOE

Note:

## EXPANSION SHOE

#### PINTLE DETAIL Scale: 34"= |"

# ANCHOR BOLT DETAIL

No Scale

Anchor Bolt for Fixed Shoes same as Anchor Bolt for Exp. Shoes except as shown.

## FIXED SHOE No Scale

Shoe Notes: Top of masonry plates, bottom of rocker plates and top and bottom of sole plates shall be planed, straightened or otherwise treated to secure true plane surfaces. Contact surfaces noted on the plans with finish symbols shall be finished in accordance with the American Standards Association surface roughness requirement as defined in

ASA B461, Surface Roughness, Waviness and Lay, Part I.

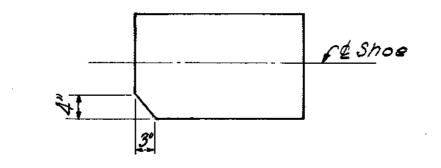
The plates comprising the expansion shoes shall be set so as to be truly centered under full dead load at a temperture of 68°F.

Concrete pads shall be formed integral with abutment or pier and not less than '8" or more than 'a" above finished elevation. Dress down pads by rubbing, grinding or as otherwise approved by the Engineer, to true level surfaces at the finished elevation. Anchor bolt assemblies shall conform to AST.M. A-307 and shall be hot-dip

galvanized conforming to A.S.T.M. A-153. Templates shall be used to accurately set the anchor bolts.

Material for shoes (exclusive of self-lubricating plates) shall be high strength low alloy structural steel conforming to A.S.T.M. Specification A-588.

Material for self-lubricating plates shall be Leaded Tin Bronze conforming to A.S.T.M. specification B22, alloy D modified to the extent that 1.5 to 2.5 percent lead is allowable. Shoes shall be included with structural steel item for payment.



TYPICAL DETAIL OF MASONRY P No Scale

## RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM POWHITE PARKWAY

SOUTHERN RAILWAY OVER POWHITE PARKWAY
BRIDGE B-05

SHOE DETA	ILS
MATTERN & MATTERN	SCALE:

YES,	SEAY, MATTERN & MATTERN	SCALE: AS SHOWN
	Associate Engineers	CONTRACT NO. 2
WARD,	NEEDLES, TAMMEN & BERGENDOFF General Consultants	SHEET NO. 17 OF 19

						SHO	) E	DI	MENS	IONS						
		EXP/	NSIO	N SH	OES		-			, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	· F	IXED	SHO	ES		
TYPE	NO. REQD.	Α	В	C	D	Ε	F	G	TYPE	NO. REQD.	Α	В	С	D	E	F
<i>85E</i> / 1	19	12"	19"	1312"	17"	17/2"	28"	12"	85F/	19	12"	19"	1312"	17"	172	28
85E2	19	10"	19"	11/2"	17"		28"	10"	B5F2	19	10"	19"	11/2"	17"	14'2"	28
85E3	10	12"	2/"	14"	19"	1728	30*	12"	85F3	10	12"	21"	14"	19"	17'2"	30
				· · · · · ·										,		
					-											
										<u> </u>						
																<del> </del>

	BY	DATE	3	As Built	R.J.H.	11-72
MADE	T.E.M	1-17-68	2	General	J.G.V.	10-70
CHECKED	T.H.N.	2-68	1	Review Comm.	T.E.M.	6-68
IN CHARGE	J.G.D.		NO.	REVISION	8Y	DATE

# **Bridge 8**

Northbound and Southbound Powhite Parkway (VA-76) Parallel Bridges Over The James River, Kanawha Canal and CSXT Railroad Tracks

**Original Construction Plans for Bridge 8N and 8S** 

**Record Set Plans** 

GENERAL PLAN AND ELEVATION BRIDGE LAYOUT, GENERAL HOLES AND ESTIMATE OF QUANTITIES PROFILES AND PAVEMENT ELEVATIONS

TITLE

SOUTH ARUTMENT

NORTH ABUTMENT ABUTMENT DETAILS

PIER 1 PIER 2

PLER 3 (DELETED) PIER 4 (DELETED) PIER 5 PIER 6

PIER 7 PIER 8 PIER 9

PIER 10 PIER 12 PIER 13

PIER 14 PIER 15 PIER 16 PIER 17

SHOES

JOINT DETAILS

DECK DETAILS

(DELETED)

DRAINAGE SCUPPERS

32-33

35

36

38

43

FRAMING PLAN - UNITS 1, 2, 3 AND 4 FRAMING FLAN - UNITS 5 THRU 12 A FRAMING PLAN - UNITS 13, 14 AND 15 FRAMING PLAN - UNITS 16, 17 AND 18

DECK PLAN - UNITS 1, 2, 3 AND 4 DECK PLAN - UNITS 5 THRU 12 A

DECK PLAN - UNITS 13, 14 AND 15 DECK PLAN - UNITS 16, 17 AND 18

STRUCTUPAL STEEL DETAILS

ALUMINUM RAILING DETAILS

ELECTRICAL DETAILS APPROACH SLABS SLOPE PROTECTION BORING LOGS

COVER SHEET

SHEET NO.

TEMPTH 229000 OFFINAL

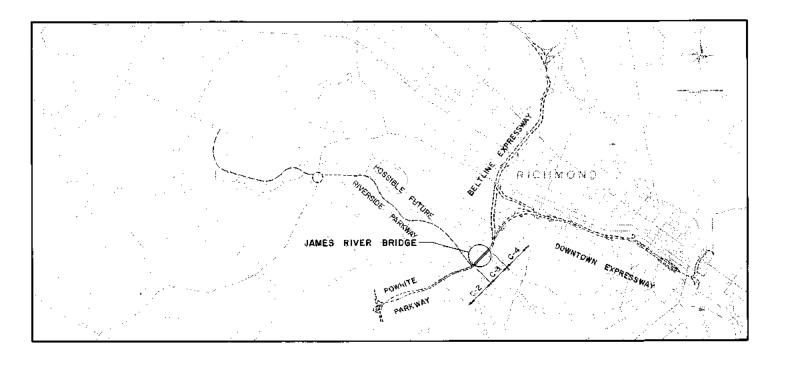
	RICHMOND EXPRESSWAY SYSTEM					
SECTION	PROJECT	SHEET NO	TOTAL			
3	JAMES RIVER BRIDGE	1	53			

## **RICHMOND METROPOLITAN AUTHORITY**

PLAN AND PROFILE OF PROPOSED

# RICHMOND EXPRESSWAY SYSTEM

# CHESTERFIELD COUNTY CITY OF RICHMOND JAMES RIVER BRIDGE



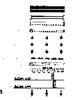
CONTRACT C-3

BRIDGE B-8

#### CONVENTIONAL SIGNS

TRAVELED WAY GUARD BAIL RETAINING WALL RAILROADS

ENIDGES
COLVERTS
DROP INLET
TROLLEY POLES
POWER POLES
TELEPHONE OR TELEGRAPH MARSH HEDGE WOODS



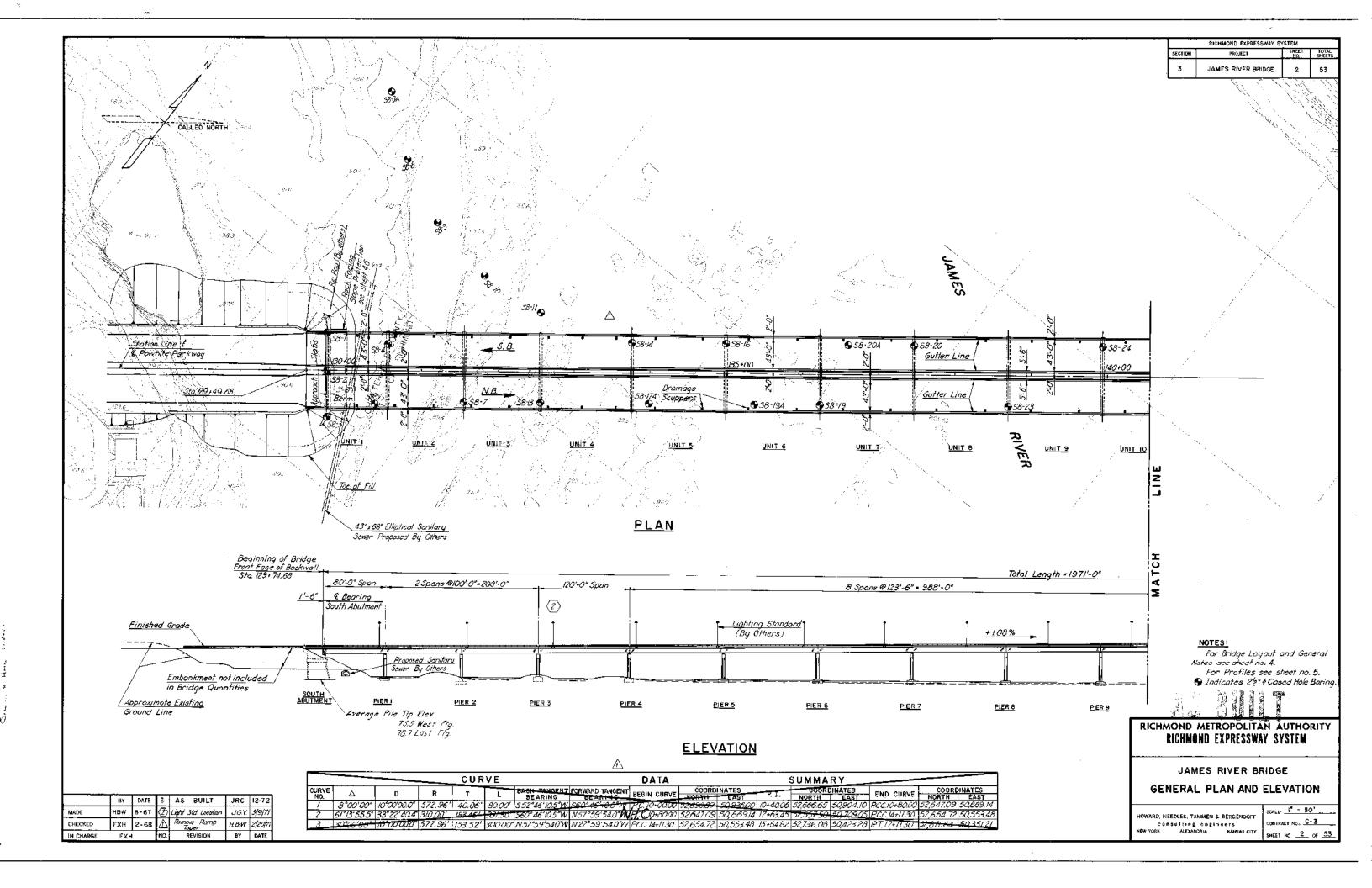
⚠ Index Revised J.G.V. 2-20-71

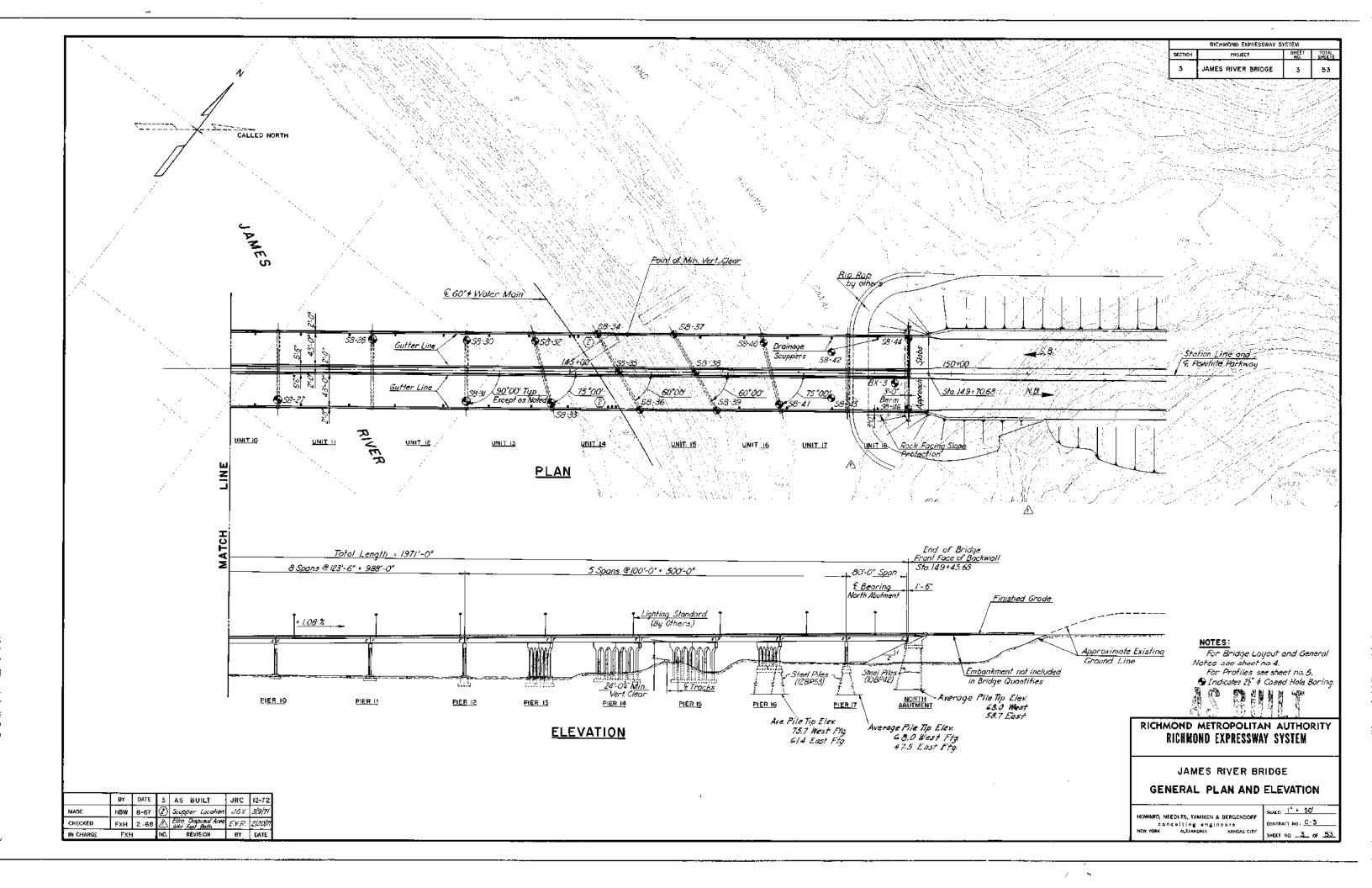
	SUBMITTED BY
Date	
-22-71	John P. Fowler ID
Date	HOWARD, NEEDLES, TAMMEN & BERGENDOFF

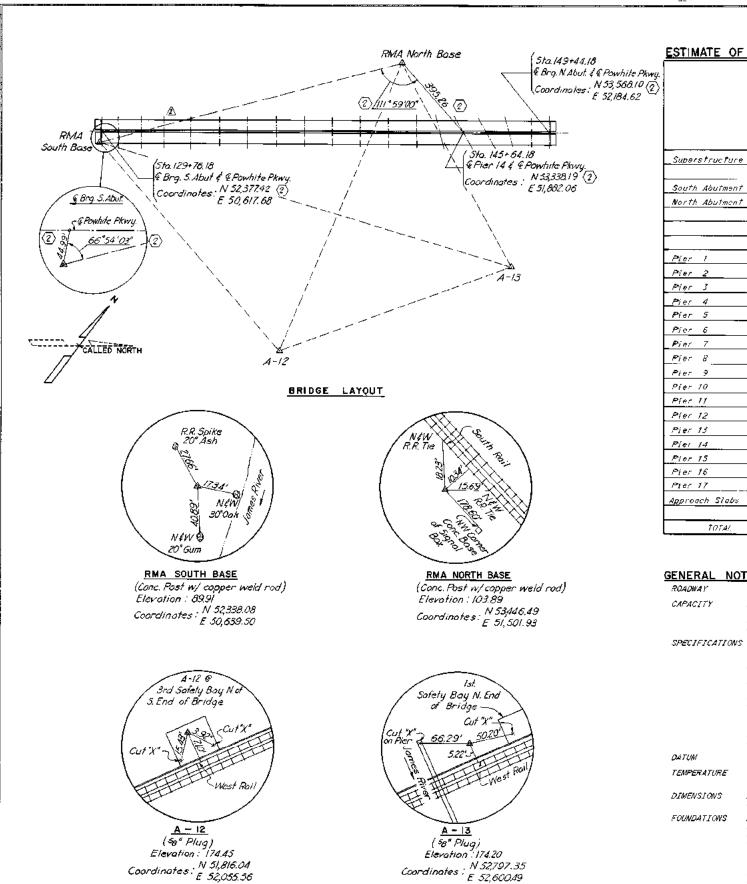
	RECOMMENDED BY
Dale	1. 6 Gan
[-22-7]	State of the state
Date	RICHMOND METROPOLITAN AUTHORITY

	APPROVED BY
/-24-7/ Date	Char a. Jan
	RICHMOND METROPOLITAN AUTHORITY

Plans Revised						
Sheet No	Date	Sheet No	Date			
1-6,9-16,28,29	Z-20·71					
32-38,41,43,46	2-20-71					
1,2,3,17,37,	3-9-7/					
38,39,40	3-9-7/					
1. 4,35,36,44	4-5-7/					
21	9-3-7/					
23	4.12.72					
45	3-/2-72					







BY DATE A As Built JAC 12-72 | HBW | H-67 | ② Bridge Loyout | H.B.W | 4|5|7| | FXH | 2-68 | ↑ | Quantities s | H.B.W. 2|20|7|

REVISION

CHECKED

IN CHARGE

STIMATE OF	QUANT	<u>ITIES</u>	/À													ι				
	Struct. Excov. of Piers 10 to 13 Incl.	Struct. Excav. Except Piers 10 to 15 Incl.	Porous Back fill	Steel Sheet Piling (Left in Place)	Steel Piles 108942	Steel Piles 128953	Concrete Class A3 Substructure	Concrete Class 43 Appr. Stabs	Concrete Class AA	Concrete Closs T3	Reinforcing Steel	Structural Steel	Aluminum Bridge Roiling	Dompproofing	6" Pipe Underdrain	Δ	Rock Facing Stope Profect. 6" Thick	Water Barings 4" Cased Hobs Incl. Sampling	NX Core Borings in Rock	Δ
	C. Y.	C. Y.	c. y,	S.F.		L.F.	C.Y.	C.Y.	C.Y.	C. Y.	4,50	Z.6a.	1.F.	S. Y.	L.F.		S. Y.	. L.F.	1.F.	
Superstructure				ļ					60598	<u> </u>	1,345,522	6310,579	7965							
	ļ.——	624	48	<del> </del>	15208	ļ.——	2950	<u> </u>	<u> </u>		/3465			142	/33		170	-	<u> </u>	<b>_</b>
South Abutment		608	48	+	3.7±3	+	286.7	<del>                                     </del>			13465			144	136		844	++	<del>                                     </del>	
North Abutment	<del>                                     </del>	000	40		3.25,0		206.7				73463			144	/36		074			
					-													†		
	_						****				44.50.4							.,		
Pier 1	<del>                                     </del>	923 🖺		-	<del></del>		270.9	₣——			44586						<del> </del>	<del>.                                    </del>	<del></del>	
Pier 2		358 ₾		-	ļ .		2626	<del></del>						<del></del>			<del> </del>			_
Pier 3	+	/23 A		ļ		ļ	274.5				44434						₩			Δ
Prier 4		97 ₫	<del></del> -				3//.8				44435		ļ—							Δ
Pier 5		156 ₾		<del></del>			324.2	-			44454						₩			A
Pier 6		132/	}	ļ			293.7	<u> </u>			44435	4							$\vdash$	Δ
Pier 7	<u> </u>	/ 33				<u> </u>	34/8	<u> </u>			44434									
Pier 8		105				↓	328.4				44435						─		$\vdash$	<del>-</del>
Pier 9	<del> </del>	154					3/8.3			<u></u>	44434						<del></del>			
Pier 10	245	ļ					382.7			51.0				ļ.,			ļ	1.2	38.5	
Pier 11	705	<u> </u>					547.5				74957						<u> </u>	12.5	114.6	
Pier 12	842	!					575.5				75469							47.6	41.4	
Pier 13	886	i				l	596.8	L		28.5	893/7							18.7	400	
Plet 14		845		1469			448.2			612	78522	<u></u>								
Pier 15		946		1994			4833			250	78 522									
Pier 16		656		2260		1465.7	3708			156.7	46374							-		
Pier 17	1	478		2197		2048.8	353.0			151.3	43678									

23 5.6

2678 5.808 96 7420 50498 35/45 7075.1 238.6 60598 473.7 2361,755 6310,579 7,965 286 269

GENERAL NOTES:

Twin Roodways 45'-0" clear,

Dead Load-Includes 15 lbs. per sq. ft. for future wearing

surface. Live Loods-HS20-44 looding and B.P.R. Modified Looding for Military Vehicles.

SENERAL - Virginia Department of Highway Road and Bridge Specifications, 1970.

DESIGN - A.A.S.H.O. Standard Specifications for Highway Bridges, 1961, modified by Special Design Provisions.

WELDING - 1969 Standard Specifications for Weided Highway and Railway Bridges of the American Welding Society.

ELECTRICAL - National Electrical Code, National Electrical Safety Code.

CONTRACT SPECIAL PROVISIONS

Specifications and Contract Special Provisions referred to above are necessary to make these plans complete.

CITY OF RICHMOND

The normal temperature referred to on the plans is 68°f. The temperature range for movement is 0°F to 120°f.

All dimensions are measured harizonfally and vertically unless otherwise nated.

All footing concrete shall be placed in the dry except where Class 13 Concrete is indicated on the plans or approved by the Engineer. Special attention is called to Sections 401.04 and 401.05 of the General Specifications, and to the Contract Special Provisions, concerning preparation of foundations for footings.

Because of the errotic nature of ground conditions at the site, elevations of the bottoms of the footings shown on the plans shall be considered approximate only. Should excavations at the time of construction reveal the foundations to be inadequate, redesign may be required.

5/46/

Concrete in superstructure shall be Class A4. All other concrete (except fremie seal concrete) shall be Class A3. All exposed edges and corners shall have a j" chamfer or fillef unless otherwise noted. Core in his method of vibration, the use of low-stump concrete, and/or other means shall be employed to prevent downgrade movement of newly placed slub concrete when the gradient is over 2%.

All reinforcing steel shall conform to A.S.T.M. A 615, Grade 40, reinforcing bar dimensions on the detailed drawings are to centers of bars unless otherwise nated. Clear distance between reinforcing steel and face of concrete shall be as noted on the plans. All bar laps shall be as shown on the plans.

STEEL NOTES

Structural steel shall conform to A.S.T.M. Specification 436 except as nated.

All field connections shall be made with i" diameter high strength bolts unless otherwise noted. Bolts shall conform to A.S.T.M. Specification A-325.

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSMAY SYSTEM

1014 80 2345 1

RICHMUND EXPRESSWAY SYSTEM

53

4

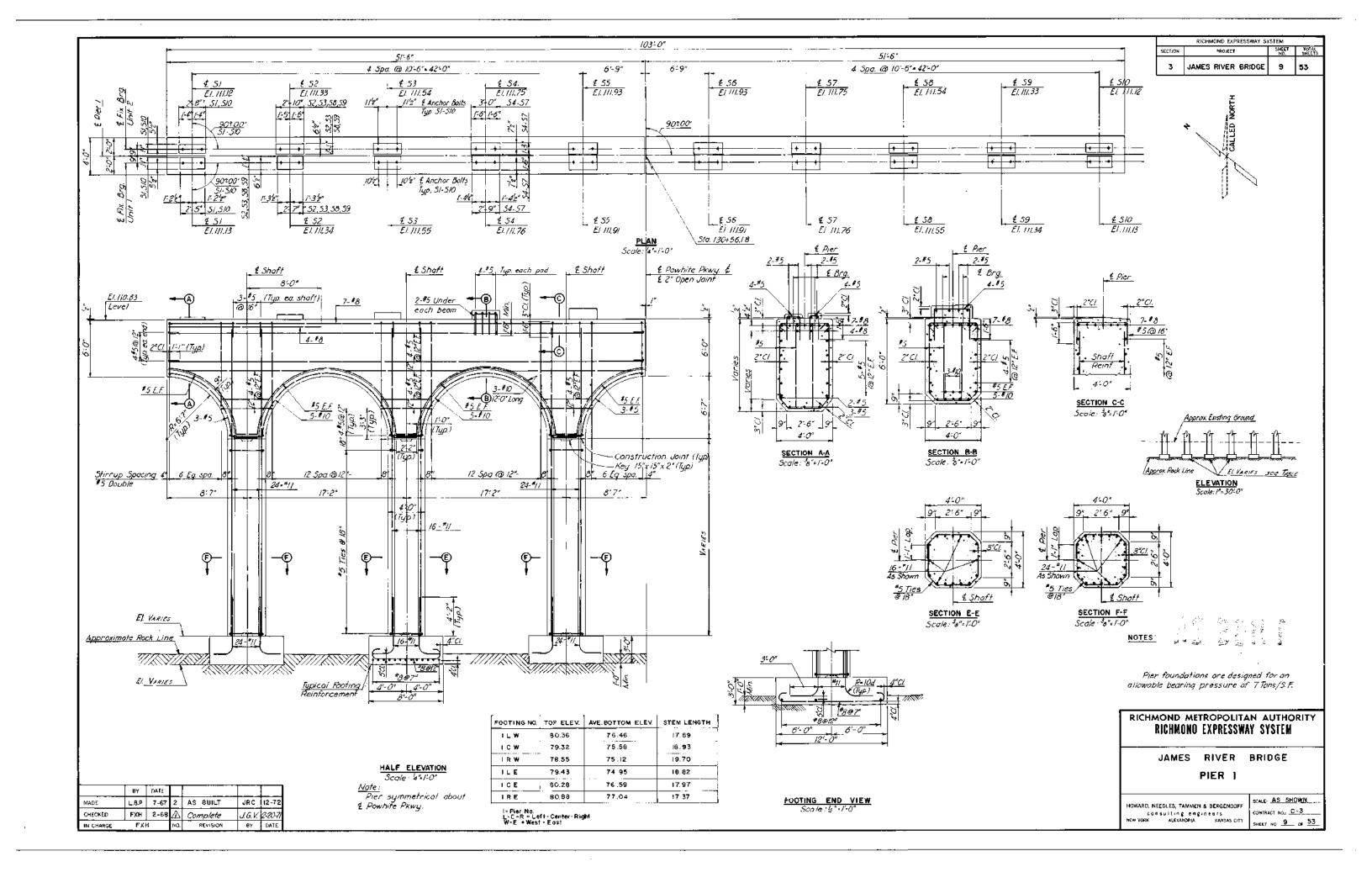
PROJECT JAMES RIVER BRIDGE

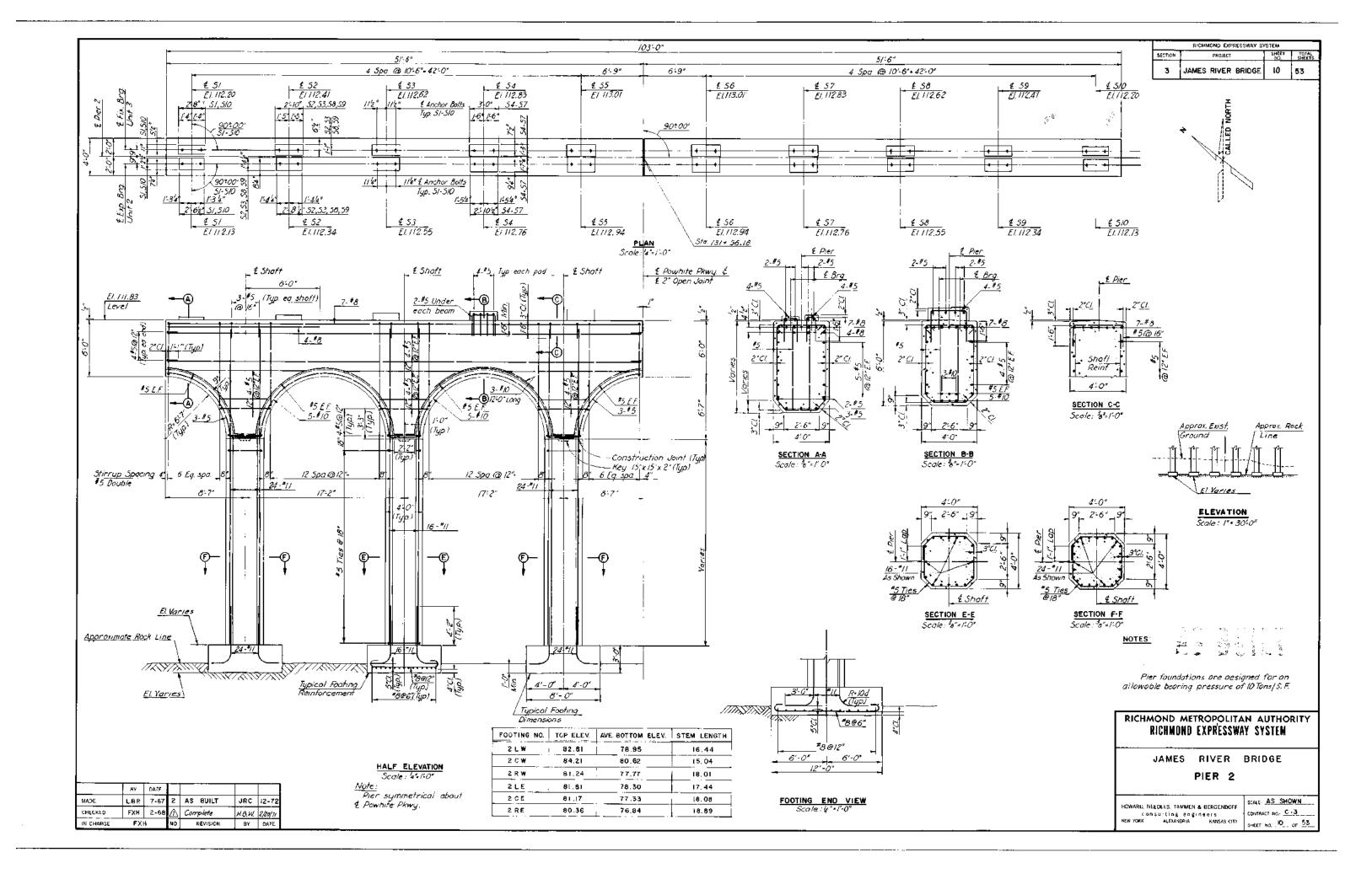
SECTION.

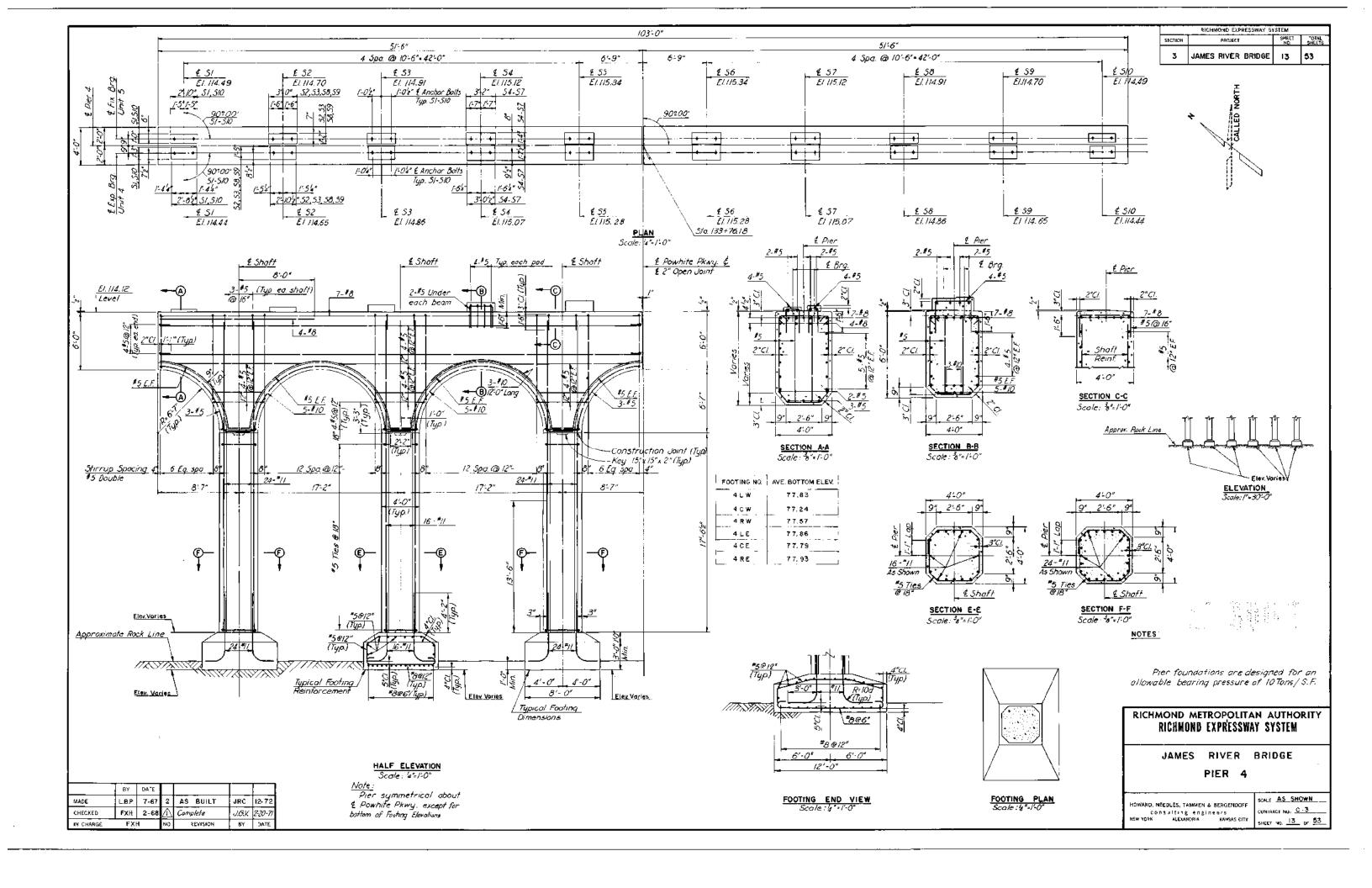
JAMES RIVER BRIDGE BRIDGE LAYOUT **GENERAL NOTES** ESTIMATE OF QUANTITIES

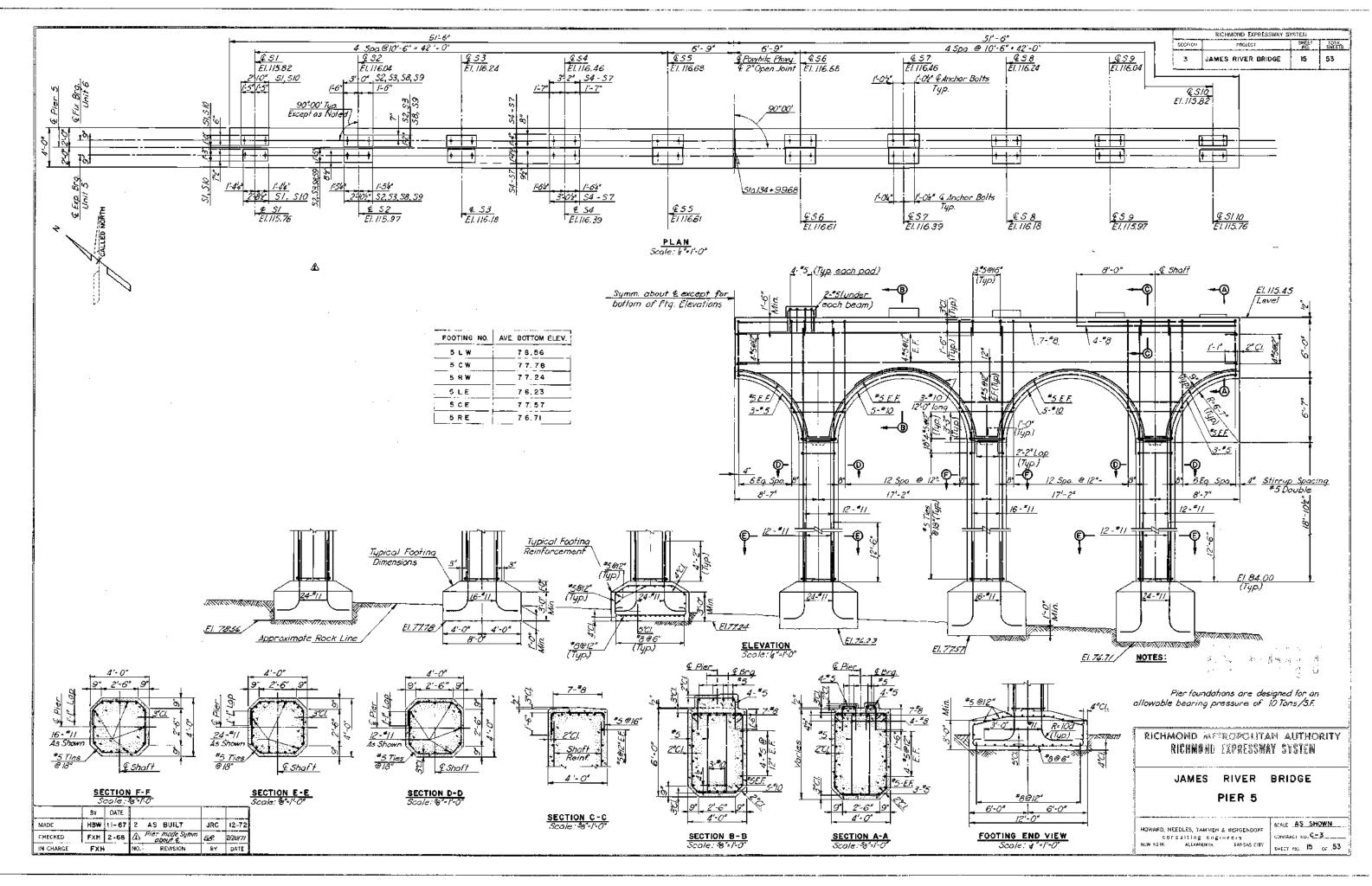
HOWARD, NEEDLES, TRAMBON & BERGENDOFF CONSUITATE ENGINEERS
NEW YORK GLEXAYORIA EARSAS CITY

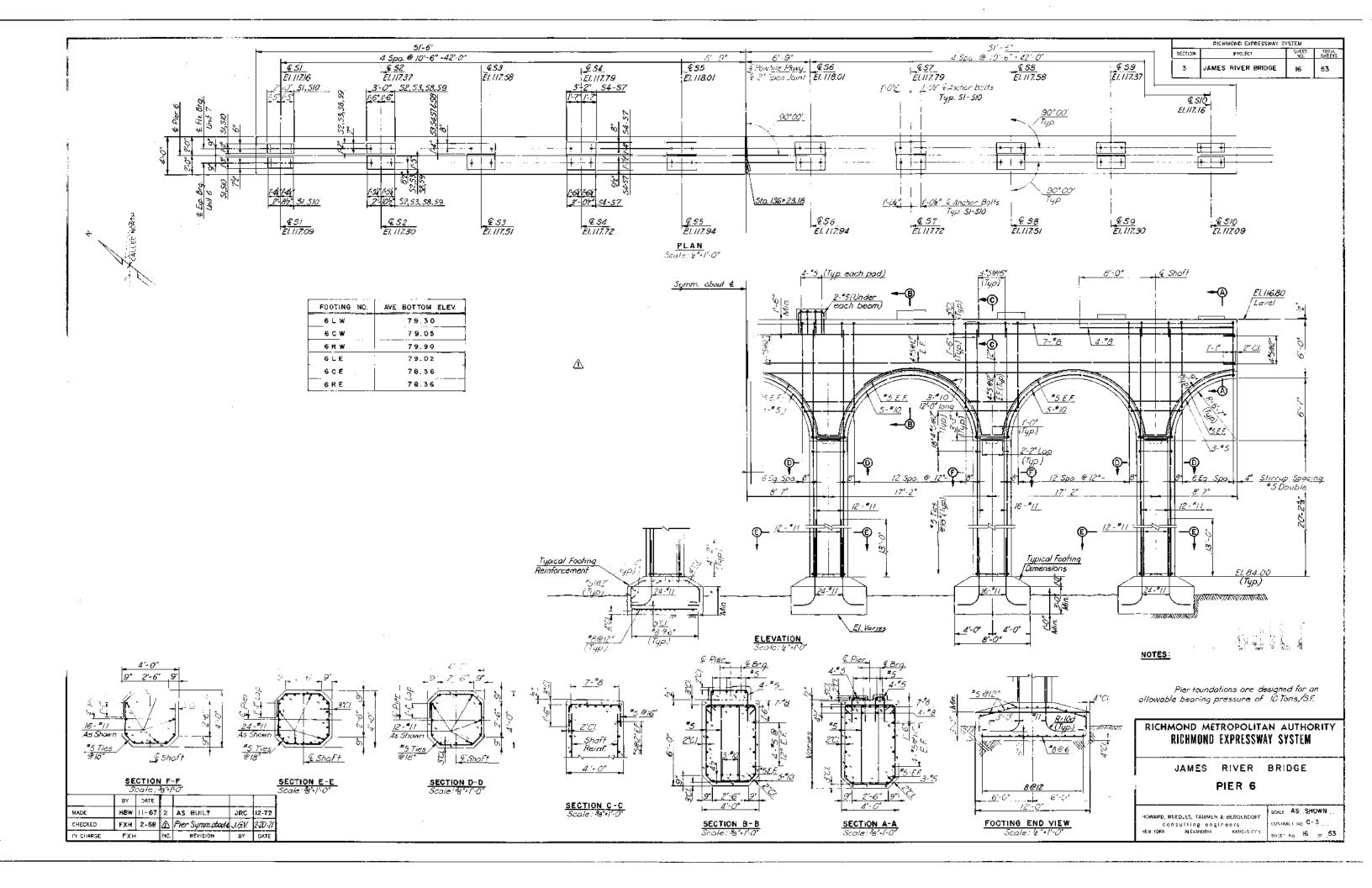
SCALE: NO SCALE SHICKET INC. . 4\_\_ or 53

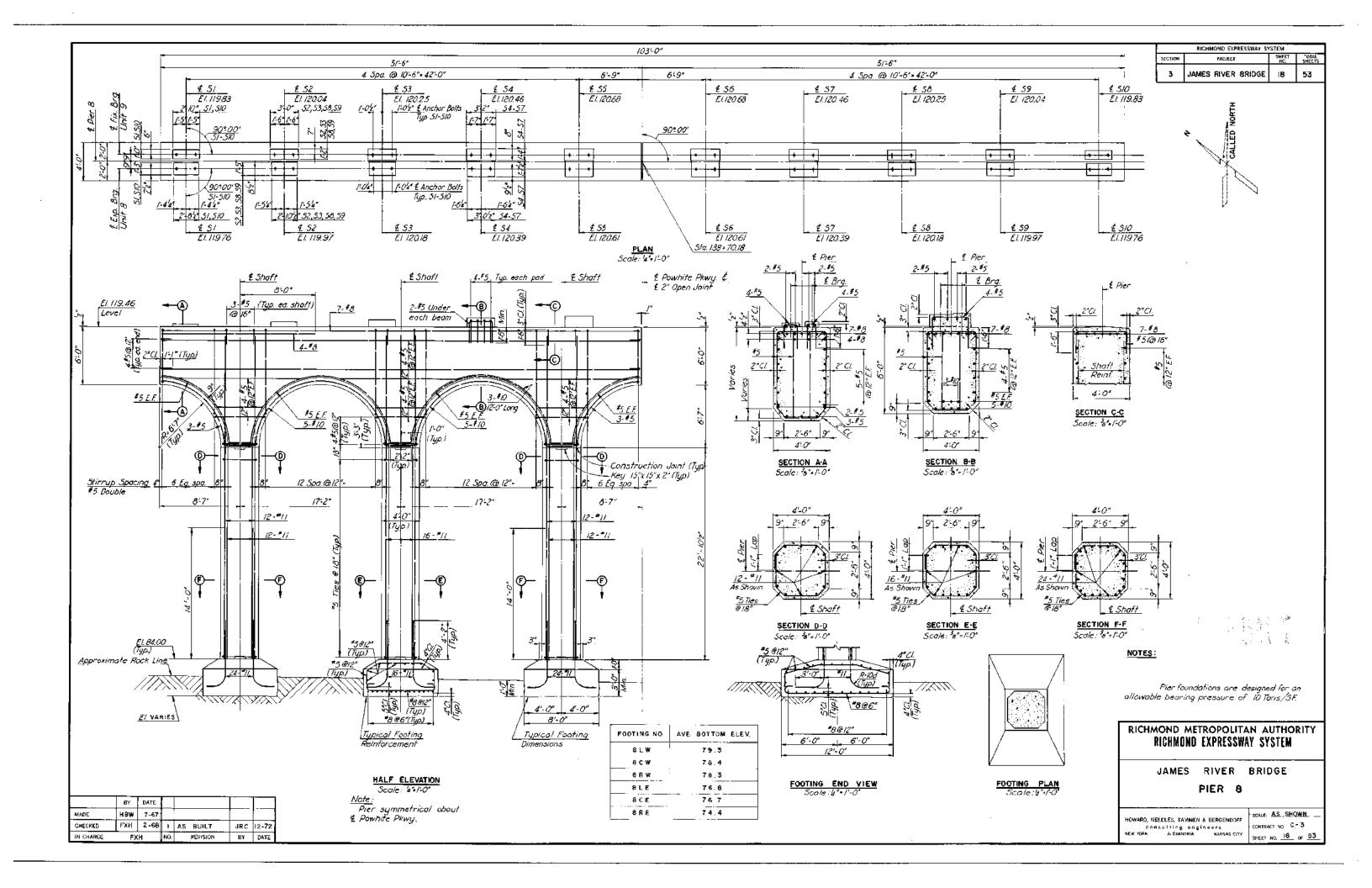


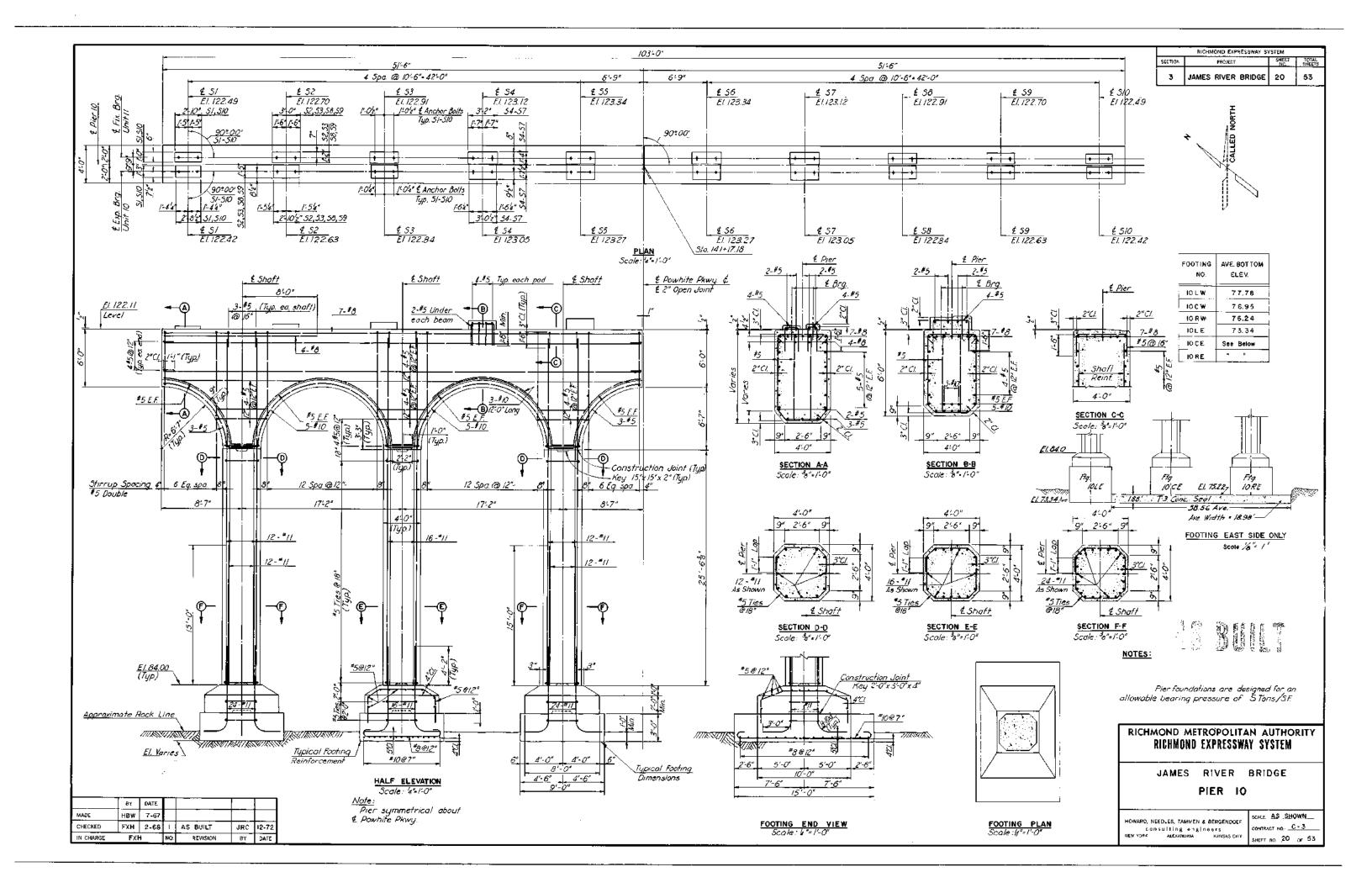


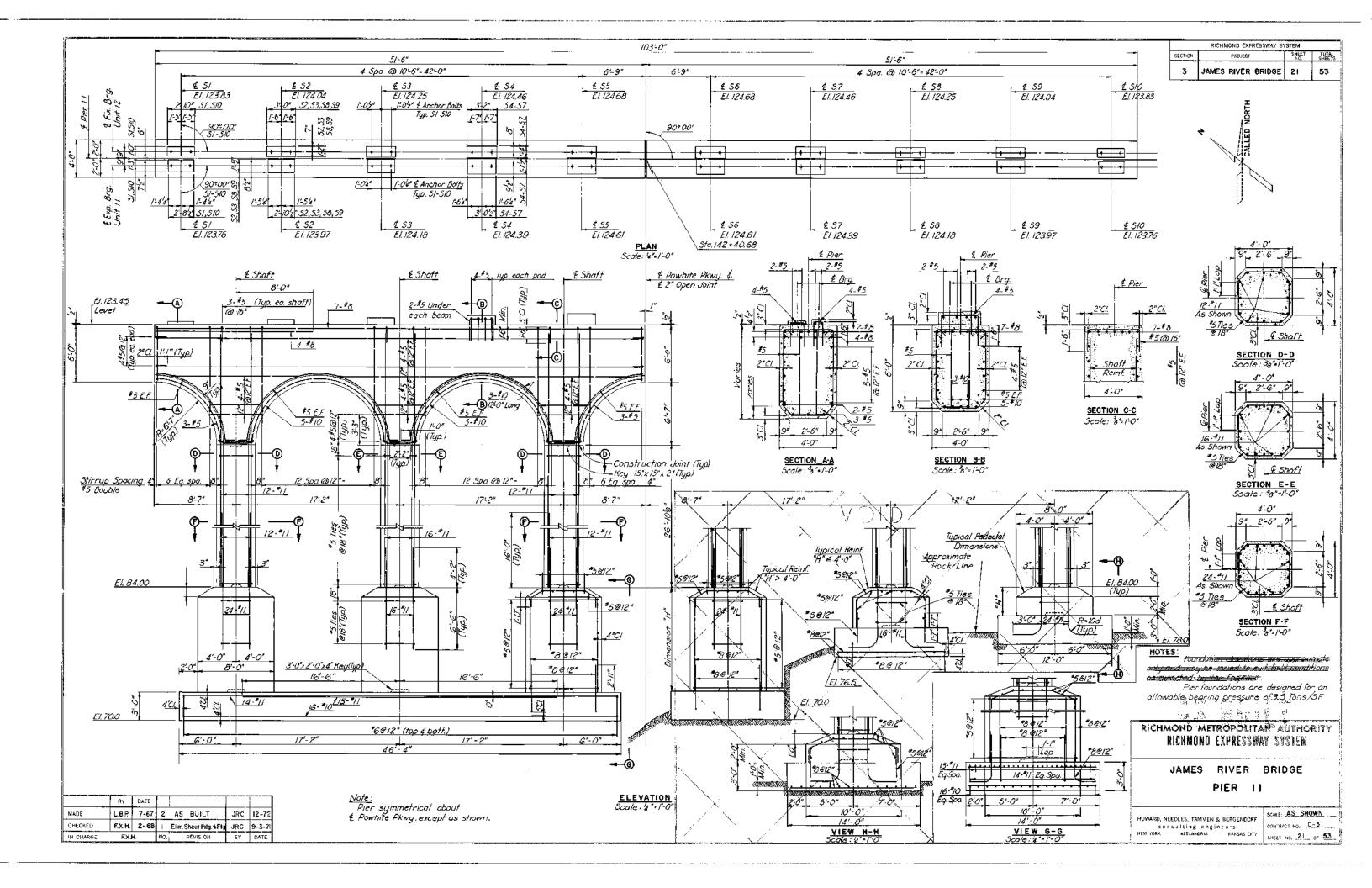


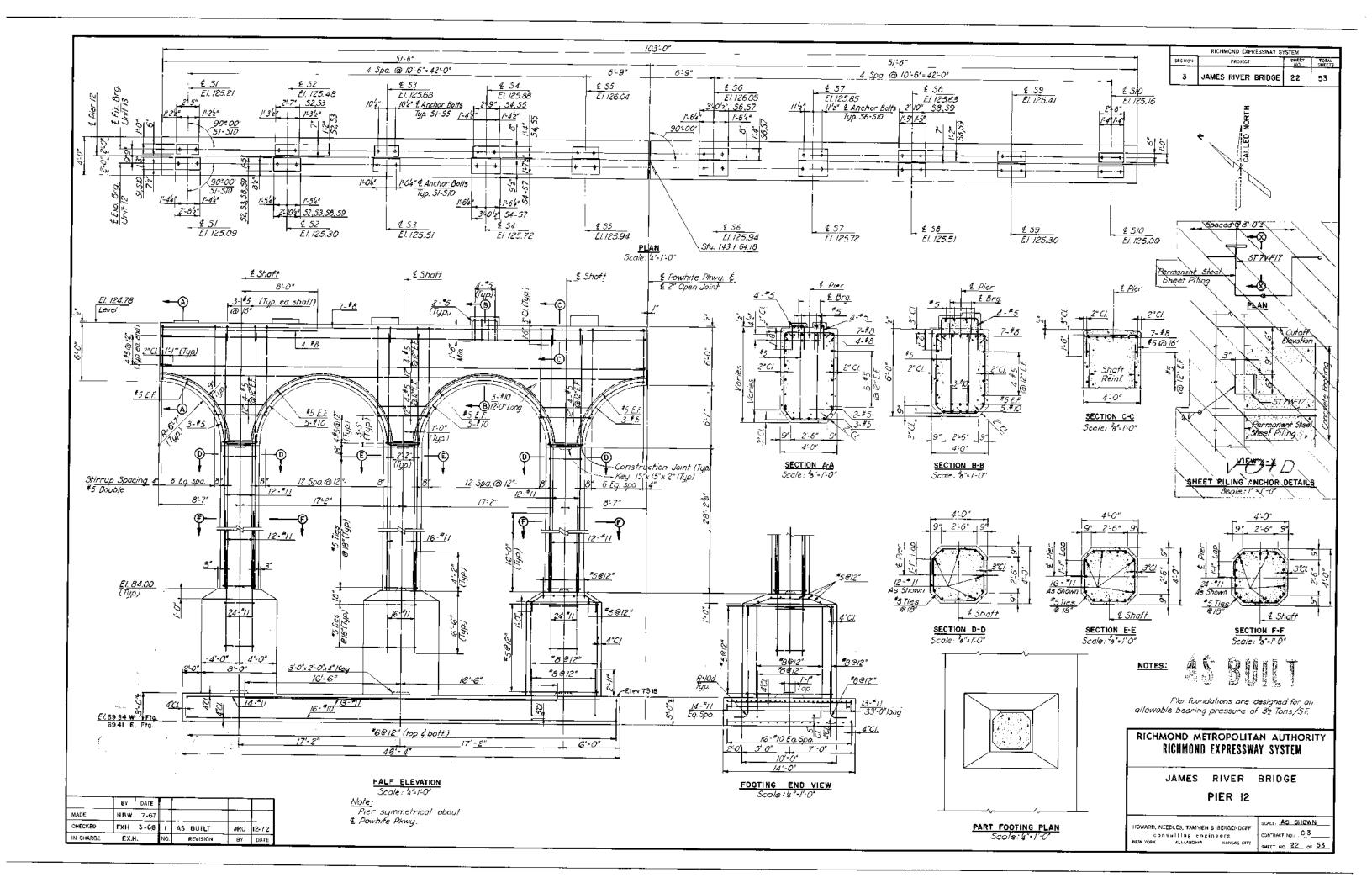


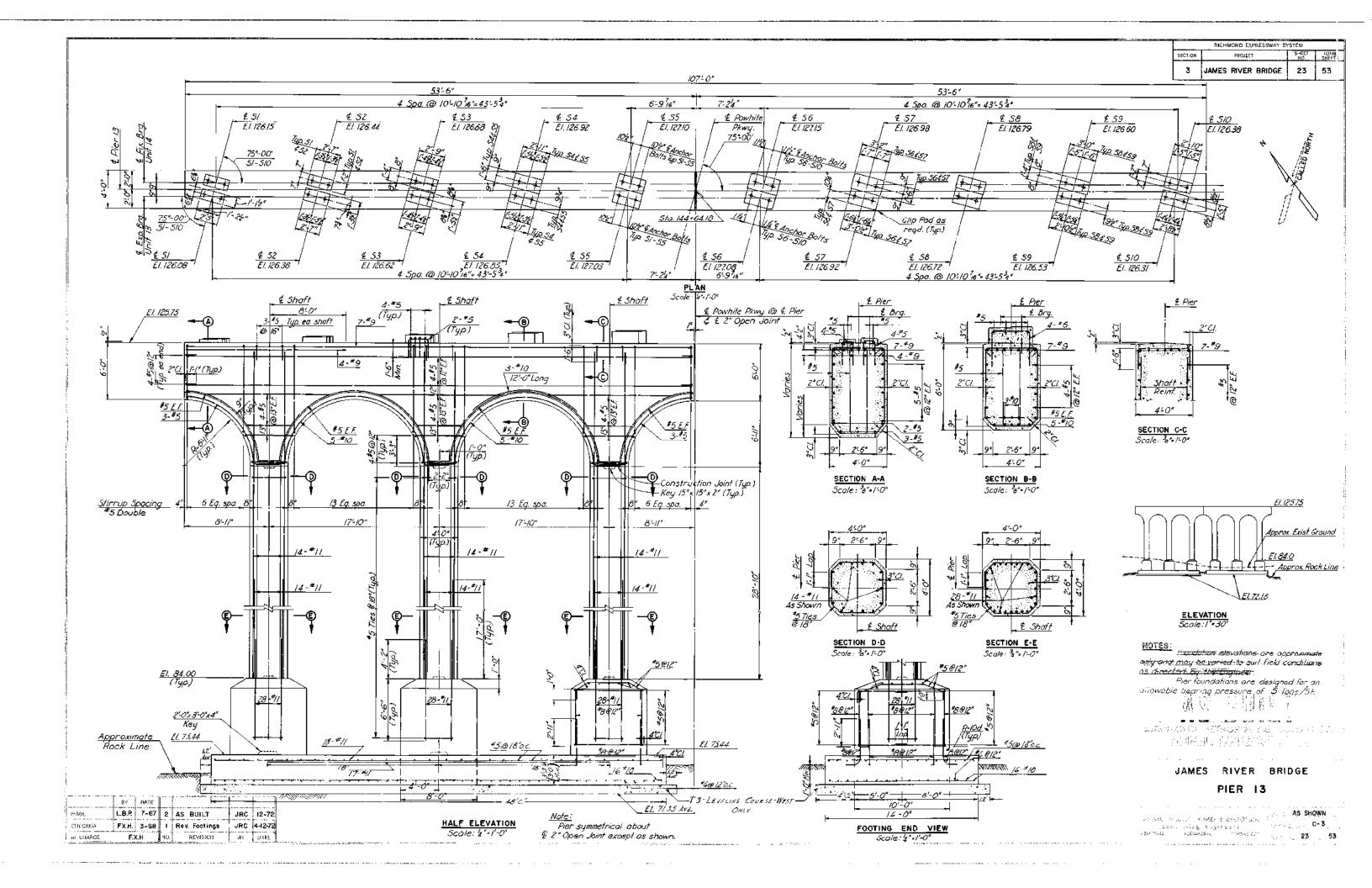


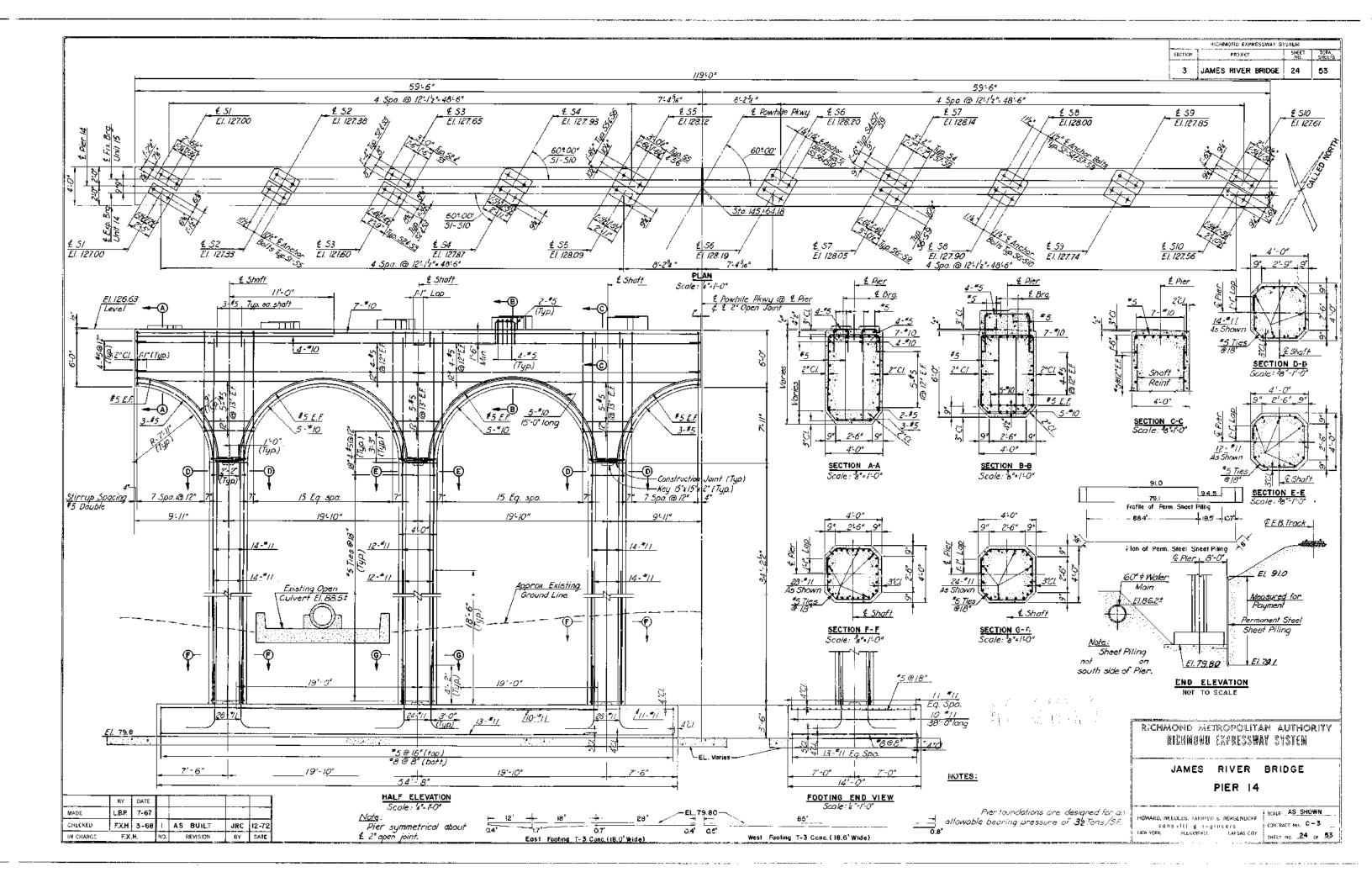


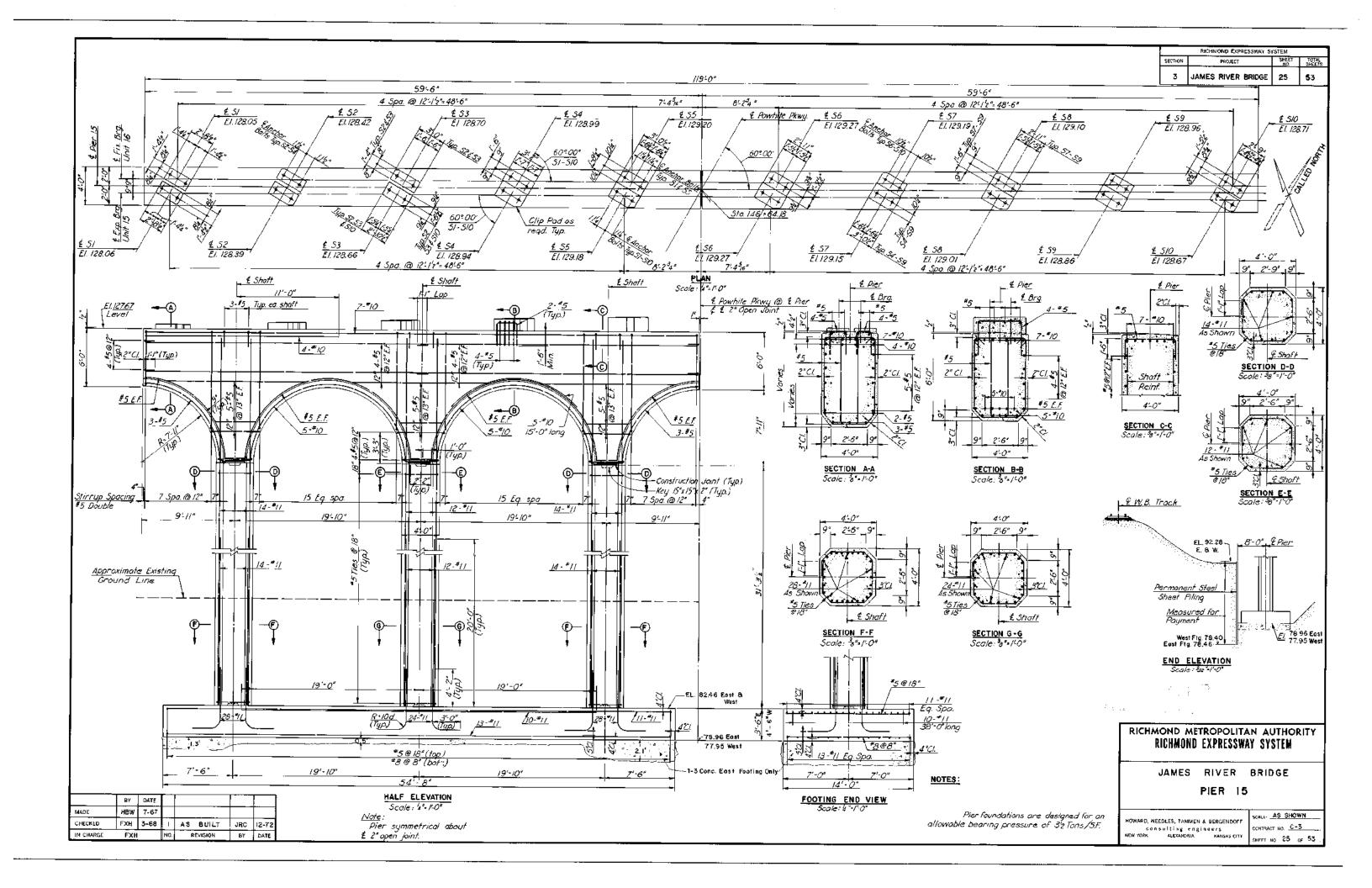


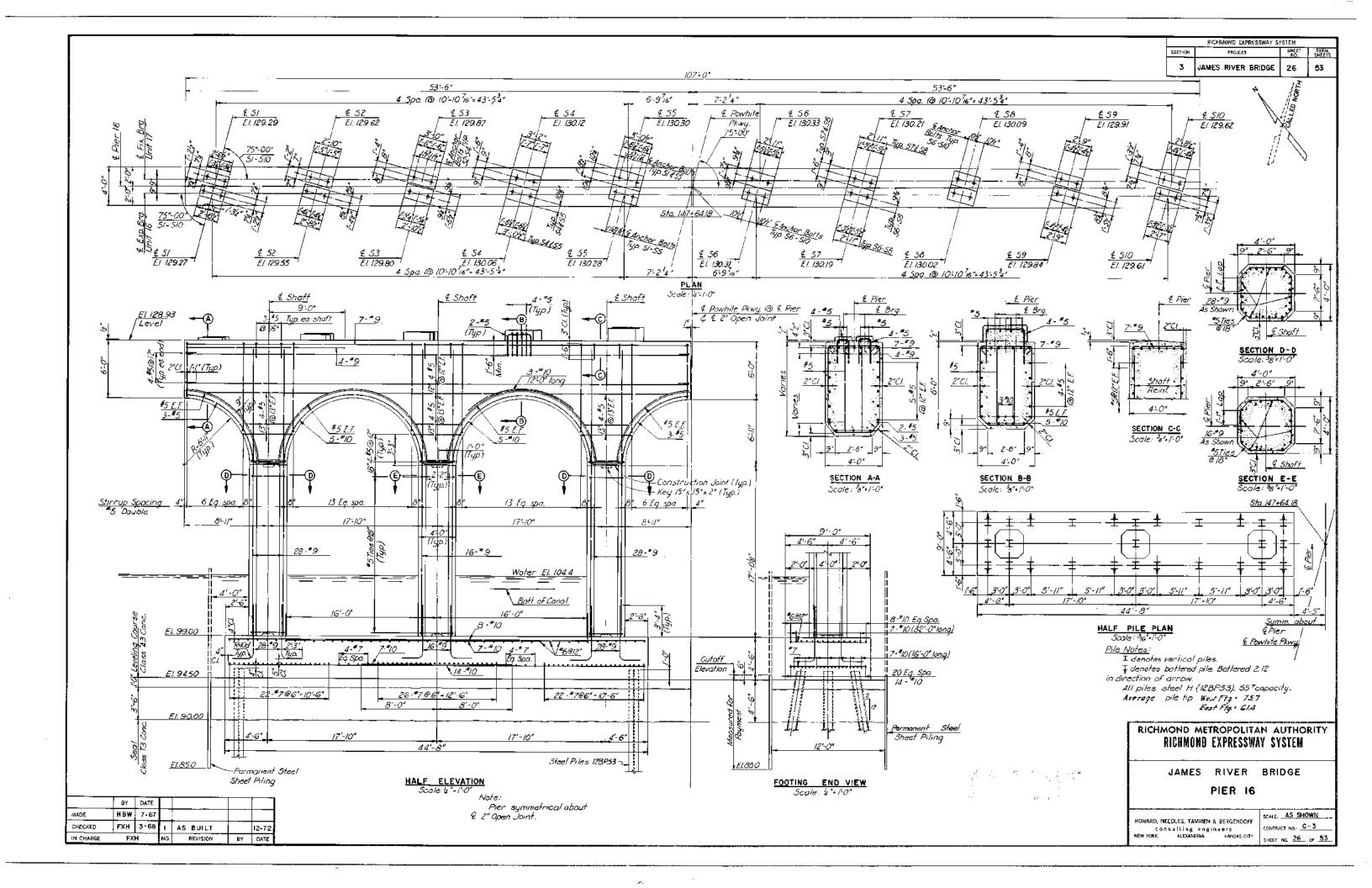


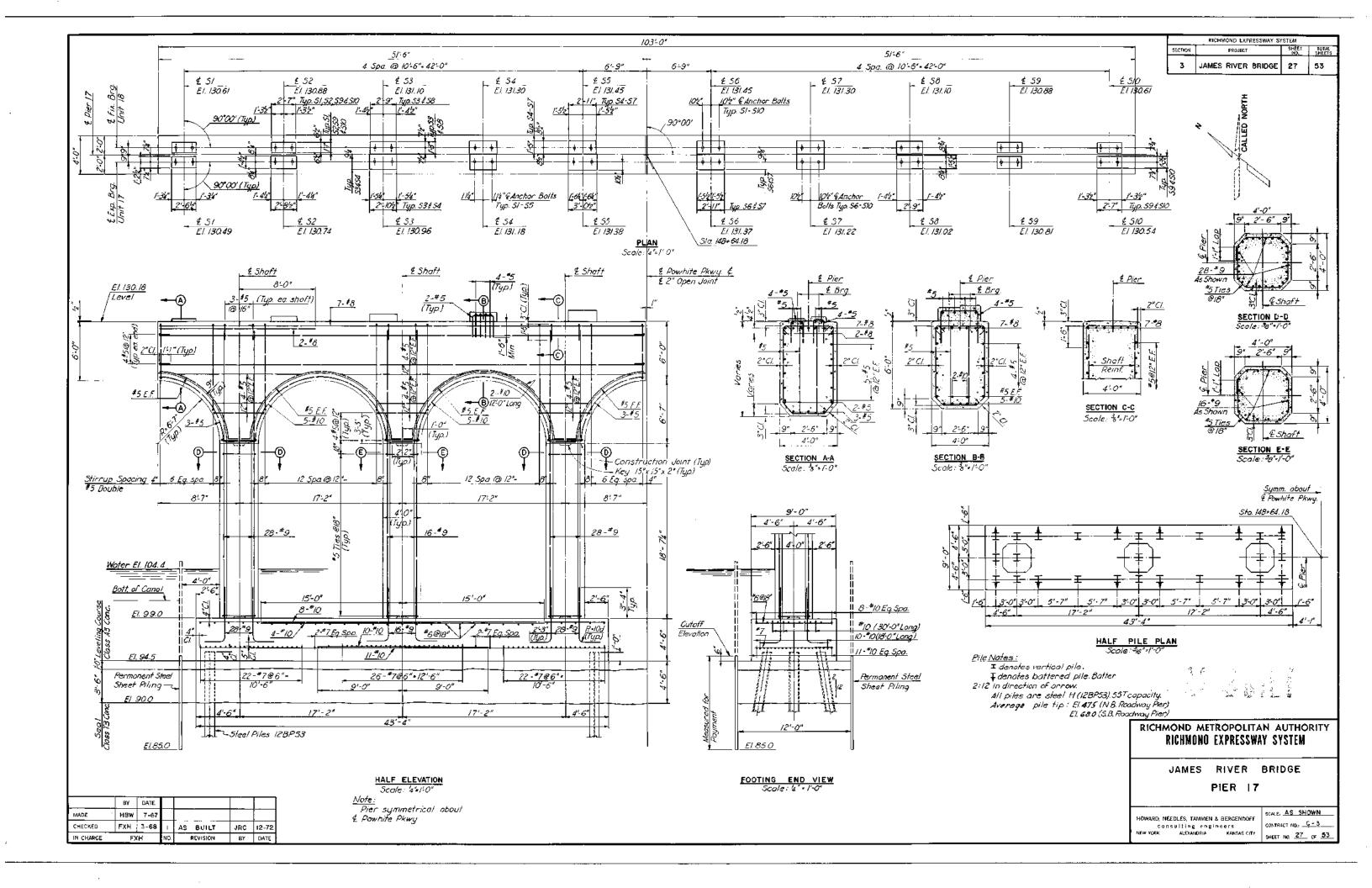


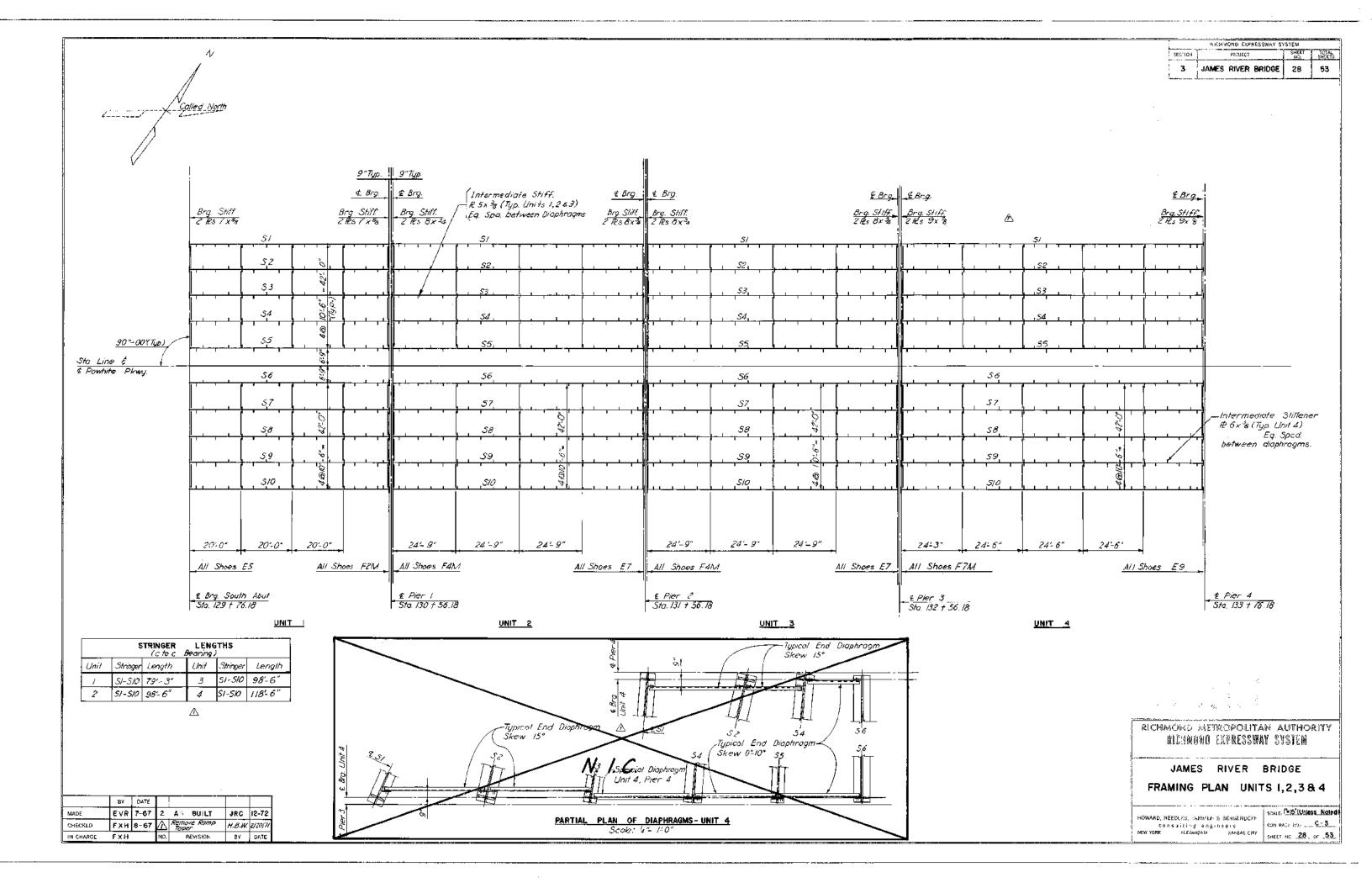


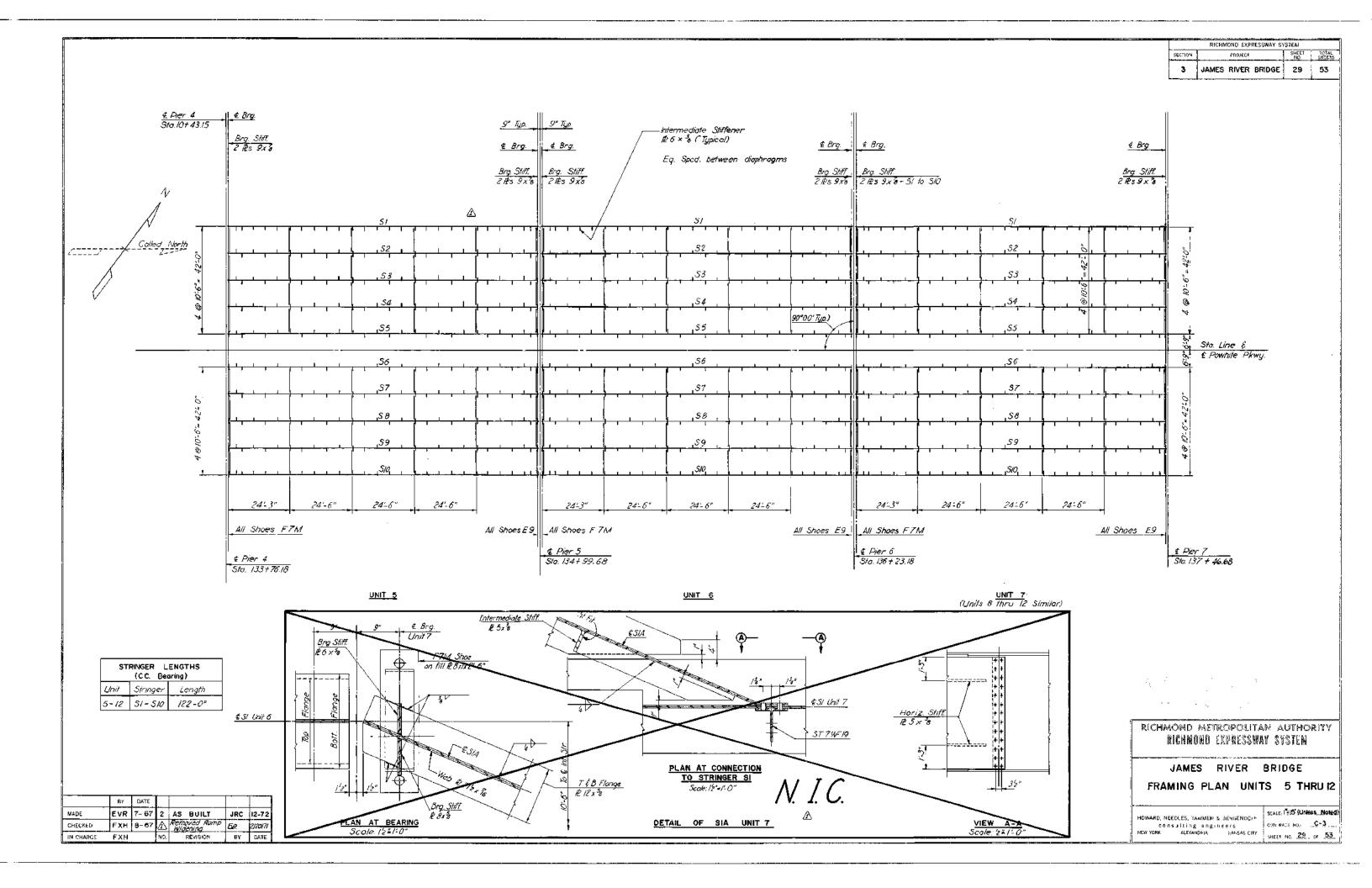




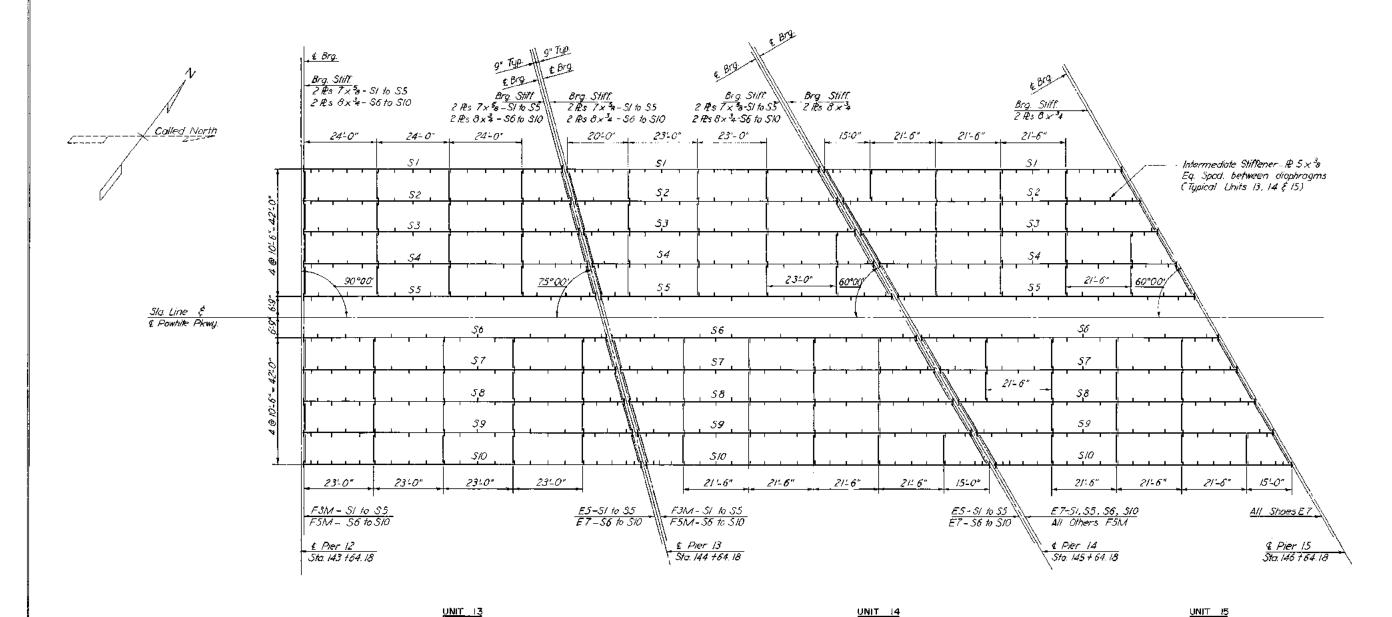












			STRINGI (	ER c to c Be	L <b>ENGT</b> HS <i>oring)</i>	3		
Unit	Stringer	Length	Unit	Stringer	Length	Unit	Stringer	Length
	5/	85'-4'5"		5 <b>8</b>	105'-10 %"		S5	961-34"
	52	88'-2"/6"	/3	59	108'-8"6"			100' -5 38"
	<b>\$</b> 3	91'-076"	(contd)	SIO	111'-6 16"	14		103'-876"
/3	54	93'-10 <sup>3</sup> s"		S/	83'-356"	(cont.d)	<i>\$8</i>	106'-11 <sup>5</sup> 6"
	55	96'-8"	14	52	85'-64"		59	/10'-2 <sup>5</sup> 6"
	56	/00' <b>-3<sup>3</sup>8</b> "	/ <del>*</del>	53	89'-9'4"		510	//3'-55g"
	.57	103"-1 <sup>1</sup> 8"		54	93'-0'4"	/5	51-510	98'-33%"

BY DATE EVR 7-67

FXH 8-67 | AS BUILT JRC 12-72

NO. REVISION EY CATE

MADE

CHECKED

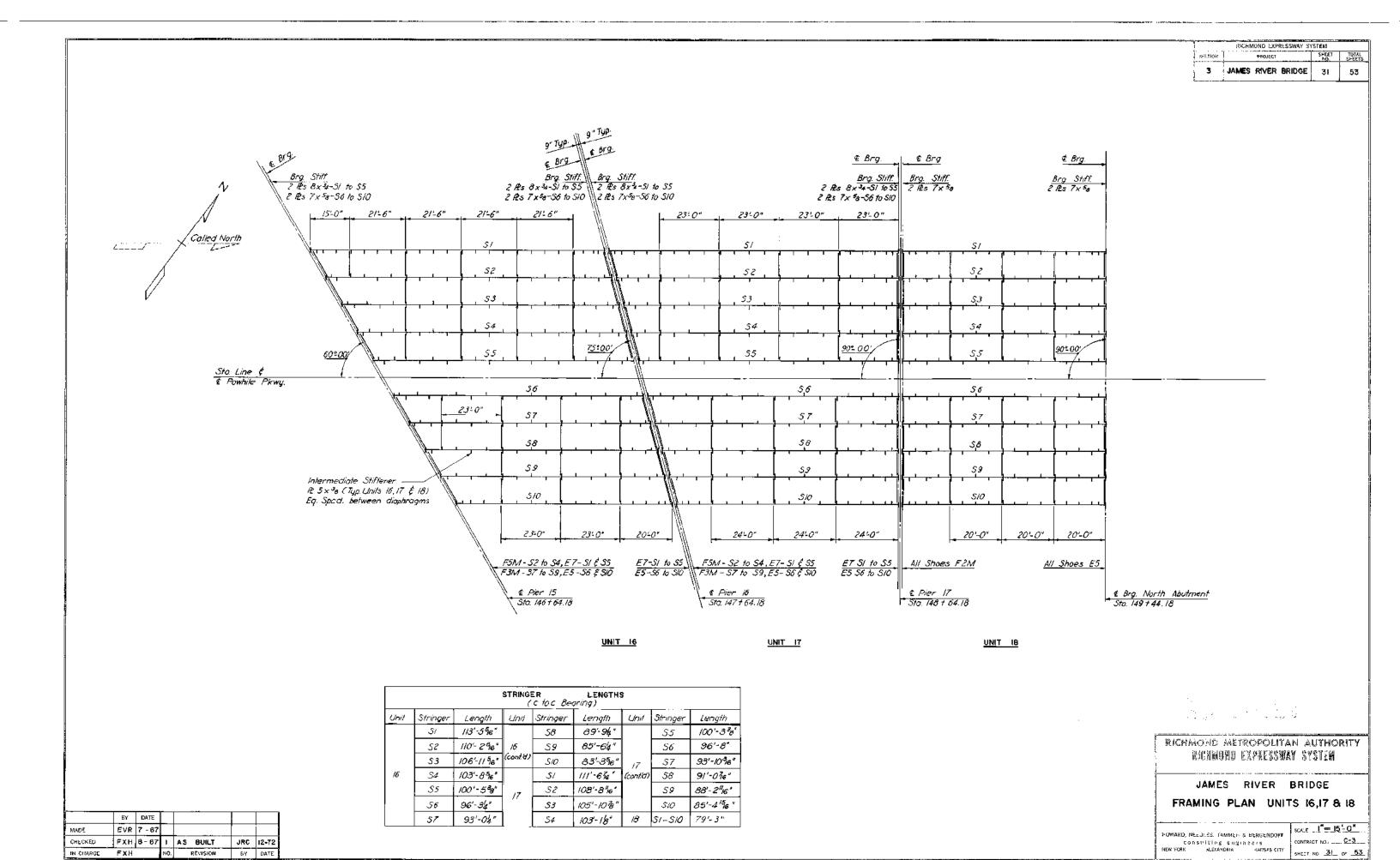
IN CHARGE FXH

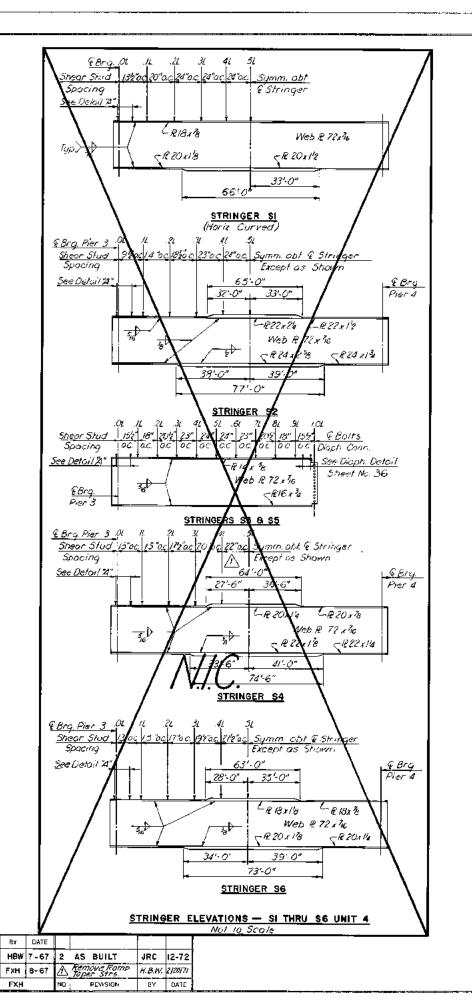
RICHMOND METROPOLITAN AUTHORITY ricimone express**way system** 

JAMES RIVER BRIDGE FRAMING PLAN UNITS 13,14 & 15

	HOWARD, NES	EDITES, TAMMAEN &	<b>HERGENDOFF</b>
i		នេះ នេះមាន ខេត្ត	eers
!	NEW YORK	ALEXANDRIA	WANSAS CITY

HOWARD, MESOLES, TAMMEN &	A HACENDARE	SCALE:   1 = 15-0"
constitue sugar	reers '	CON RAUS NO
NEW YORK REMANDRIA	WANSAS CITY	SHEET NO 30. OF 53.

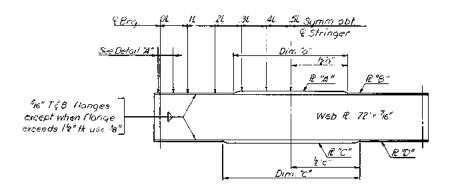




MADE

CHECKED

IN CHARGE



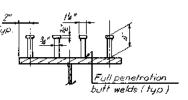
#### TYPICAL STRINGER ELEVATION Not to Scale

				STRIN	IGER SCH	IEDULE						
UNIT	STRINGER	fŁ"A"	Dim."a"	₽L"B"	RL"C"	Dim."c"	R_"D"			STUD	T .	<del></del>
├─	S5436	14 x 58	F.L.		10 7.	F. L.		10L-11L	.IL-2L /3を"	2L-3L	18/2°	20%
/	S1-54.57-510		E.L.	_	16x8	40'-6"	16 x 34	11/2"	13/2"	15'2'		207
	01-04,01010	14 x 12 ··	F. L	_	16 x 1	541.00	10 x 3	1392	132	4 199	10	24
		16x 2 -	7.1		10 112	52' 6"		13/2"	16"	18%		+-
Z	55 € S6	16x 3	F 1	·		60'-6"	19 X 6	12"	14"	18"	22"	24
	SI-S4.57-510			_	18 x 138		13 x 18	12"	14"	16"	19"	21"
<b>-</b>	JF-54531-510	16 x 3	F. L.	_	18 x 12	58'-6"	18 x 1	12"	14"	17/0		
3	55£56	<del>-</del>	<u> </u>			60'-6"	(4) 70	13	14"	/8"	22"	24
,	SI-S4.S7-S10	Exa	FL	_	18 x 1 €		18 * 78	12"			19"	21"
	いいかいいい	16, 4			18x12	58'-€"	18 x l	1/2"	14"	/6"	19	21
۱,	SI-54.57 <i>S10</i>	Sec.	<del>-{/eva/lions</del> 		Accr	741 611	20: 11.	1530-11	1.21.0	T 726.5	4 1374	696
4		18 x 1/2	68' 0"	18 x 16	70x13	70 '- 6"	20x14	1212"	1412"			23"
	S5 & S6	18 177	59° 0″	18 x 3	20x/8	72'-0"	20x14	122"	1412"	1812		
	<u> </u>	18.76	60' 0"-	10 - 20	201/2	777 0"		13/20	15t	20"		24
5	<u> </u>	18+1		6.7	20x/\$	75'-0"	20x1/g	14/21	1672	20/2		24.
	S5 & 56	16×14	66' 0"	16x8	20x Z	731-0"	20x/³8	155.	14%		24"	24"
	SI-S4, S7-SIO	181/8	68'-6"	18, 8	20x2	75'-6"	20x13g	125"	14%			
6	\$5656	18214	66'-0"	18/8	20x2	77'-0"	20x1-8	1212"	14%			24
	SI-S4,57-SIO	18118	68.5"	15 x 3	20x2	15.6"	20x13g	129"	14/2	172		23%
Treco	51- <b>54</b> ,57- <b>5</b> 10	17.13/	68-6"	18×8	20x 2	75'-6"	2011-8	12%	14%	_		231
12	<u> </u>	Posta	00.0	16x8	ZO+2	73'-0"	70x138	12.5.	148	17'2'		24
	51	18 1 58	F.L.		16 x18	51'-0"	16 x 4	11/2	/3"	15%		20
	52	14 1 8	FL		16x/4	5/'-0"	16x3	11/2"	1313.	15/2"		20
١.	55	10 / 2	FL	<u> </u>	16x18	55'-6"	16×8	119"	132	16'2'	182	20
13	S.s.	14 , 5%	F L		16x12	55' 6"	16x1	12"	13%	15%	182	20.
1		14 , 4	F_L	L	15x12	57-0"	16 x1	12"	14"	162	21"	24
14	56	1614	FL	<u> </u>	18.13	62.6	18x3	12"	14"	172		24
	57	1018	F. L.	<u> </u>	18 x /3	61'-0"	18x16	12"	14"	16"	19%	21"
	_ 5ë	1611	51-0*	16134	18113	66'-6"	18x18	124	14"	17"	19/2"	2112
	59	16x1	57'-0"	16x3	18x/8	67.0"	18 x 14	12"	14"	17"	20"	22
	SIO	16 118	63'-6"	16 x 3	1816	68'-6"	18x138	1212"	14"	16%		
15	St-54,57-510	18,34	FL.		18112	58' 6"	18x1	12"	14"	/6"	19"	21'
	<i>55 €</i> 50	16 x 4	EL	_	18x18	60-61	MXS	12"	14"	18"	22"	74
	SI	16 2 14	63.6	16.134	18.22	68.60	19 118	12/2"	14'	162	7 20%	22%
	52	10.1	57'-0"	16 2 34	18x18	67'-0"	18 11/4	12"	14"	17"	20"	22'
	5.3	F. et	5/1-0"	1624	18 113	<b>6</b> 64-6"	18×1%	12"	14"	17"	1921	21/2
16	54	16x8	F. 4.		18 x 18	6/-0*	18x16	12"	14"	16"	19/2	2/"
¢	55	16 1 30	8.L	_	18,18	621.0"	13 x 3	12"	14"	172	22"	24"
/7	<u> </u>	14 x 34	F. L	_	16 x 1/2	57'-0"	16x1	12"	14"	162	2/"	24'
l	57	14 x 3	F.L.		15x12	35'-0"	10x1	12"	132	15'2"	" /8'z'	20
[	58	14, 50	FL		16x/3	551-61	16 x 3	11/2*	13/2	16h	182	20'
	59	14 x 58	F.L.	-	16 x 14	51'-0"	16 x 8	1/2"	312	15%	18"	20
	5/0	14 x 58	F. L		16x18	5/1-0"	16 x 34	11/2"	/3"	15%	18"	20'
18	5/ 54,57-510	14 . 58	F.L.		16 x 1	40'.€"	614	119"	137	15/2	18"	20'
79	35¢35	14 x 58	FL	_	16 x &	84	_	12"	137	1 16"	182	20/2

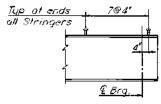
### NOTES:

For General Notes see sheet no 4. For Joint Details see sheet no 35. For Shoe Details see sheet no. 34. Supper locations shown on Deck Plans. For Scupper Details see sheet no. 36. For stringer lengths see Framing Plans.

	RICHMOND EXPRESSWAY SYSTEM						
SECTION	PROJECT	SHEET NO.	TOTA, SHICETS				
3	JAMES RIVER BRIDGE	32	53				



SHEAR STUD DETAIL Not to Scale



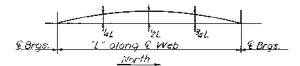
DETAIL "A" Not to Scale

### SHEAR STUD NOTE:

Capacity = 3406 lbs. per stud. The controctor may, if he elects, use three "8" diameter studs at the same longitudinal spacing in lieu of four 44" diameter studs shown.

Stud rows shall be placed parallel to the main deck reinforcing.

Stieur stud spacing shown is maximum spacing.



CAMBER DIAGRAM

### NOTE TO FABRICATOR:

fabricated with an upward camber Stringers amounting to (see table). This will provide approximate compensation for deflection under full dead load.

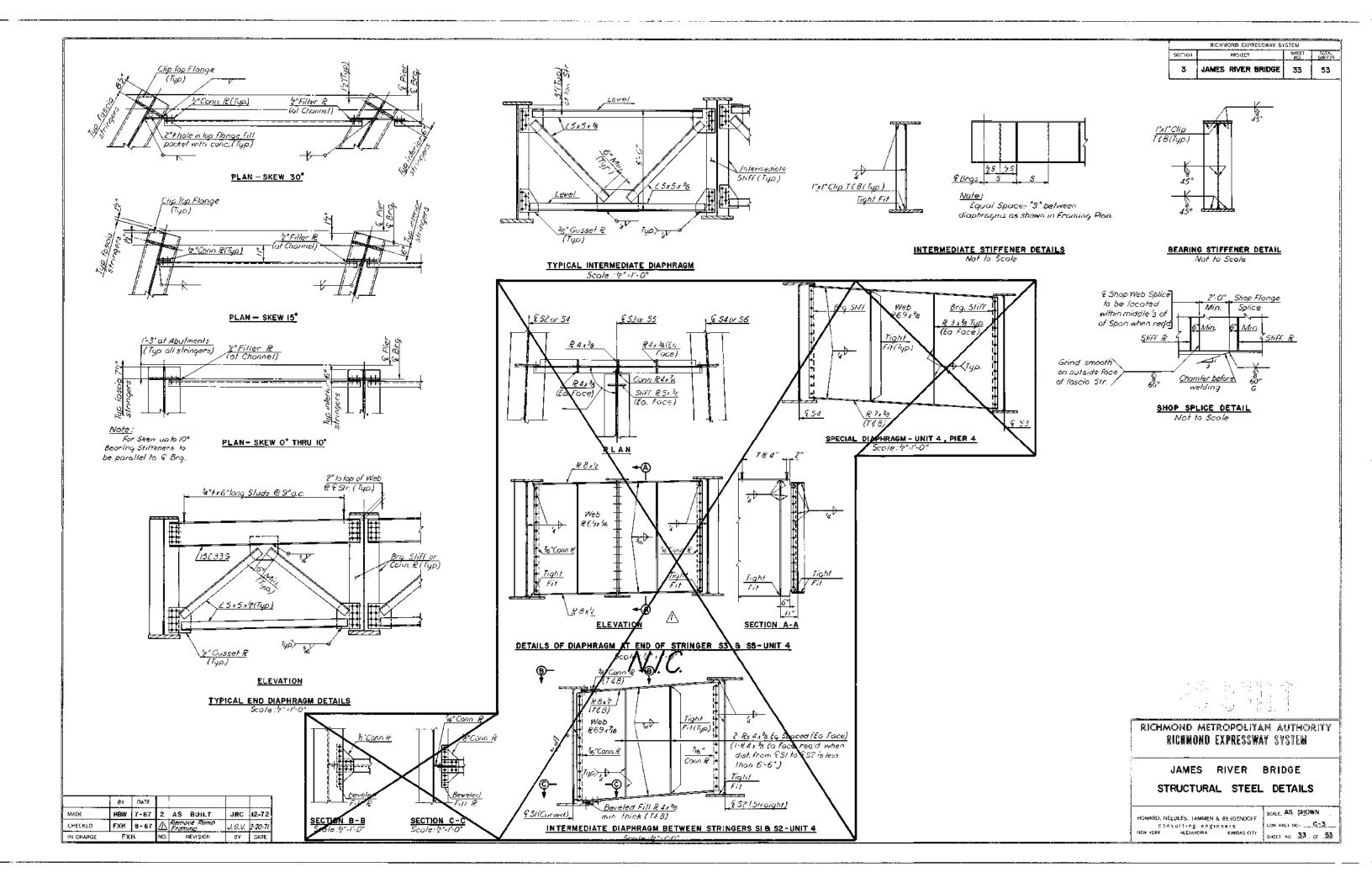
CAME	ER	SCHE	DULE				
UNIT	STRINGER	4834	12	ÜNIT	STRINGER	4834	وا
/	\$1 - 5/0	34"	/"		56	130	/3"
		14"	13,"	13 \$	57	1/2"	28
2		14."	159*	14	58	/5e"	24"
	51-510	/3g*	14"	٠٠ ا	59	/58"	24"
3		14. "	13/"		\$10	18"	212"
	51-510	/3g"	/8"	15	51-510	/38"	16"
4	SI-SIO	2"	23,5		S/	/ <sup>7</sup> 8"	2'2"
5	S5£\$6	2'8"	3"	1	52	134"	238
	51-54,57:30	28	28"	1	53	/%	24"
6	<i>\$5€\$6</i>	218"	3"	16	54	1/2"	28"
	SI-54,57:SIO	2'8"	27"	¢	<i>ა</i> 5	/ <sup>3</sup> 8"	18"
7 1bru	SI-54,57-\$10	2'a*	23	17	56¢57	14"	134"
12	55 ¢56	2'8"	3*	1	58	/e"	1/2"
	SI.	- 중*	//3"	1	<i>59</i>	/"	/ਤੌ8"
13	<i>\$2</i>	1"	/38″		5/0	ক''	/8"
13 & 14	_53	18"	1/2"	18	\$I+\$IO	34"	/"
14	54 455	14"	134"	٥			

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

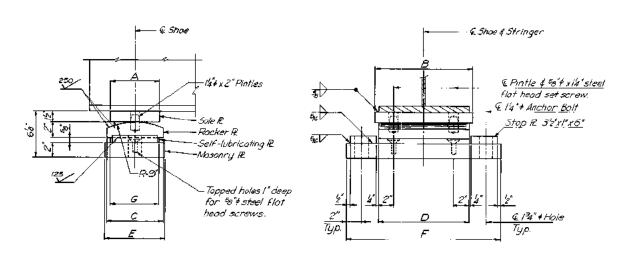
JAMES RIVER BRIDGE STRUCTURAL STEEL DETAILS

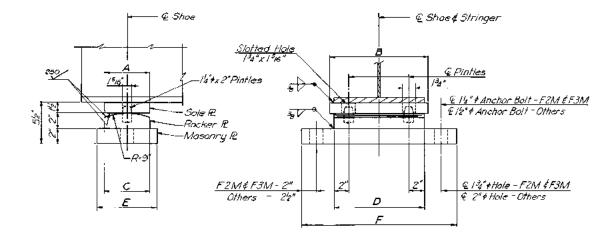
HOWARD, NEEDLES, TAMMEN & BERGENDOFF Consulting engineers
NEW YORK ALEXANDAIA HANSI

SCALE: AS SHOWN COMPRACT NO. C - 3 HANSAS CITY SHEET NO. 32 OF 53



RICHMOND EXPRESSWAY SYSTEM								
SECTION	PROJECT	SHEET NO.	TOTAL SHEETS					
3	JAMES RIVER BRIDGE	34	53					



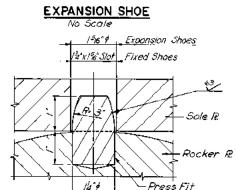


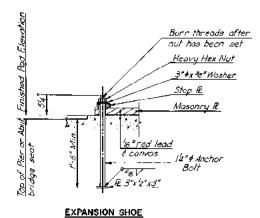
SIDE ELEVATION

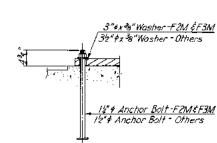
END ELEVATION

SIDE ELEVATION

END ELEVATION







FIXED SHOE

Shoe Notes:

Top of masonry plates, bottom of rocker plates and top and bottom of sole plates planed, straightened or atherwise treated to secure true level surfaces.

Contact surfaces noted on the plans with finish symbols shall be finished in accordance with the American National Standards Institute surface roughness requirement, as defined in ASA BA61. Surface Houghness, Wayness and Lay, Part I.

The plates comprising the expansion shoes shall be set so as to be truly centered under full dead load of a temperture of 68°F.

Concrete pads shall be formed integral with abutment or pier and not less than 8' or more than 4" above finished elevation. Dress down pads by rubbing, grinding or as otherwise approved by the Engineer, to true level surfaces at the finished elevation.

Another bolt assemblies shall conform to ASTM. A 307 and shall be hot dip

galvonized conforming to A.S.T.M. A-153.

Templates shall be used to accurately set the anchor bolts.

Material for shoes (exclusive of self-lubricating plates) shall be high strength low alloy structural steel conforming to A.S.T.M. Specification A-588.

Material for self-lubricating plates shall be Leaded Tin Bronze conforming to A.S.T.M. Specification 822, alloy D modified to the extent that 1.5 to 25 percent lead is allowable. Shoes shall be included with structural steel item for payment.

### PINTLE DETAIL Scale: 34" + 1"

ANCHOR BOLT DETAIL

Anchor Bolt for Fixed Shoes same as Anchor Bolt for Exp Shoes except as shown,

FIXED SHOE

EXPANSION SHOES	A	В	С	D	Ε	F	G	FIXED SHOES	Α	в	С	D	E	F
	(A)	157.00	134	3/3/6	13.41	Sister	18/1	F2M	6"	1'-52"	6"	1'-42"	7"	2-1
E5	6"	1-52	9*	1-48	92"	2-1"	7"	F3M	6"	1'-5%	6"	1-42	8"	2-1
		1,024	3	1.80	736	11/20	Biri	F4M	6"	1-70	6"	1-6"	7"	2-4
E7	6"	/'-7"	95"	/'-6"	102"	2:22	7½"	F5M	5"	1-1"	6"	/'-6"	8"	2'-4
<u> </u>	1194	17.04	Dist.	X4X	1331	3.34	1836	KEAN!	181	XX	134	1484	1144	36.8
<u>[9</u>	6"	/'- <b>9</b> *	10"	/-8"	//°	2.45"	72*	F7M	6"	1'-9"	6"	/ <del>'-</del> 8"	8"	2'-6
<del>- []</del>	· 6"	12 /12	10"	1, 10,	111	2.5	<del>. 7</del> [~	<del></del>	6#	11 714	6"	1110	8"	21.0
11/2/3/11/1	1181	13/2/	1341	26.7%	138 m	2.88	1	(F.00W)	18/4/	Sex.	Bri	35/24	11:5/11	8.2
E/#	5.	2" ("	1'-0"	2'0"	11.1"	21.00	0/42							,,,,,
				•				_						$\overline{}$
								_						

			_			
	ВУ	DATE	Γ			
MADE	HBW	5-67	2	AS BUILT	JRC	12-72
CHECKED	FXH	8-67	$\Delta$	Delete Shoes .	H.B.W.	7/20/71
IN CHARGE	FXH		NO.	REVISION	BY	DATE

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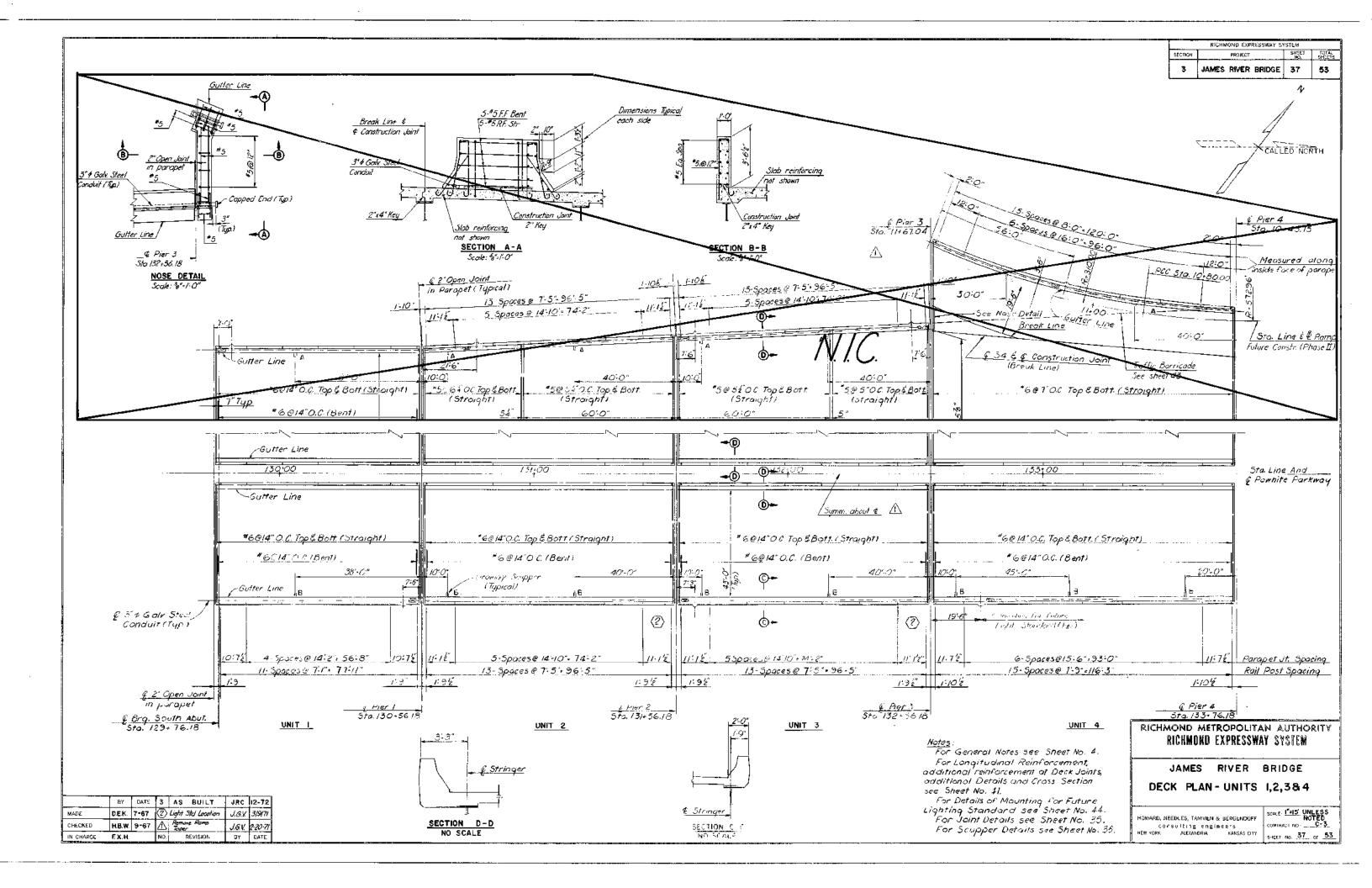
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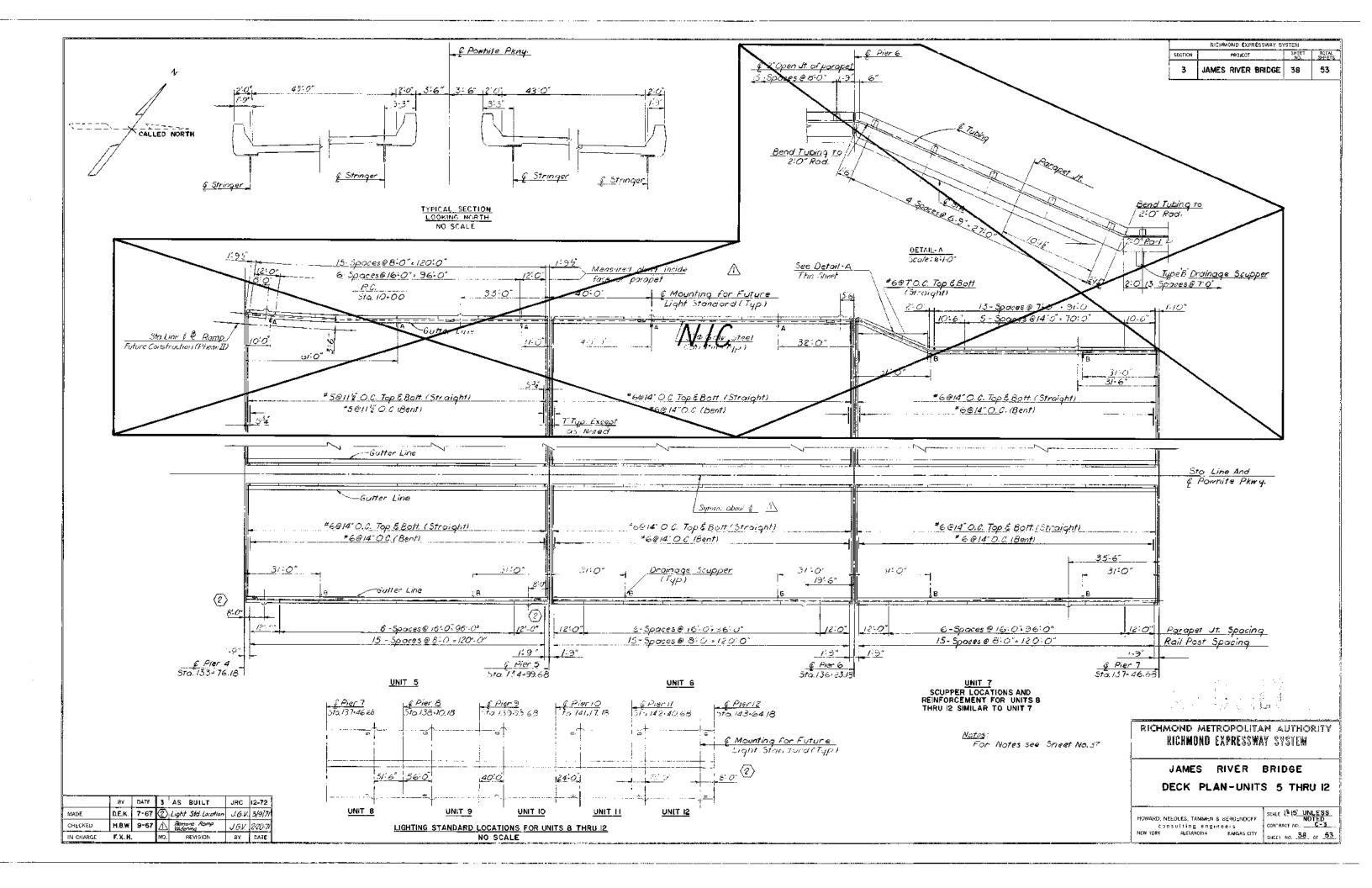
## RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

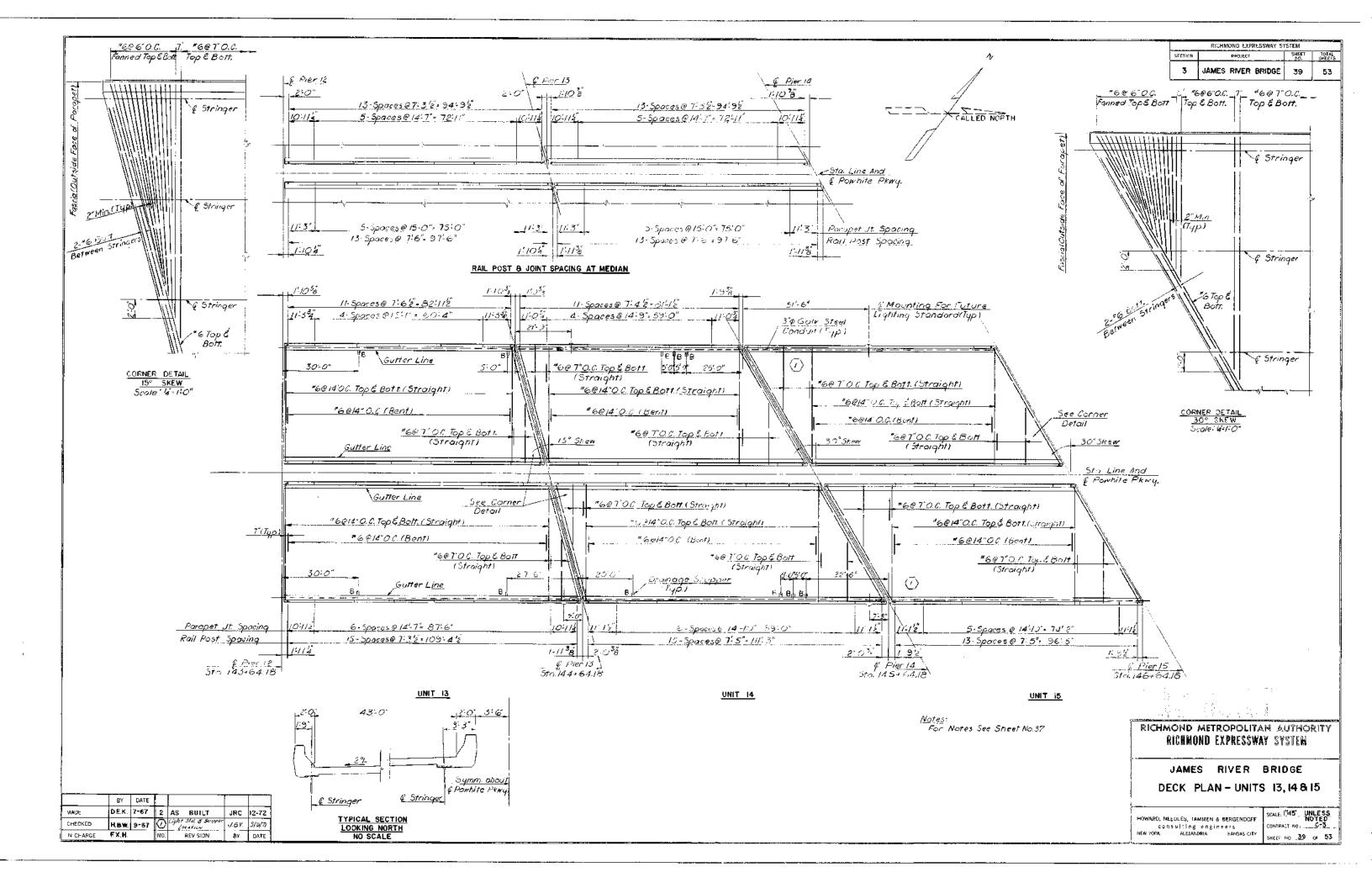
JAMES RIVER BRIDGE SHOES

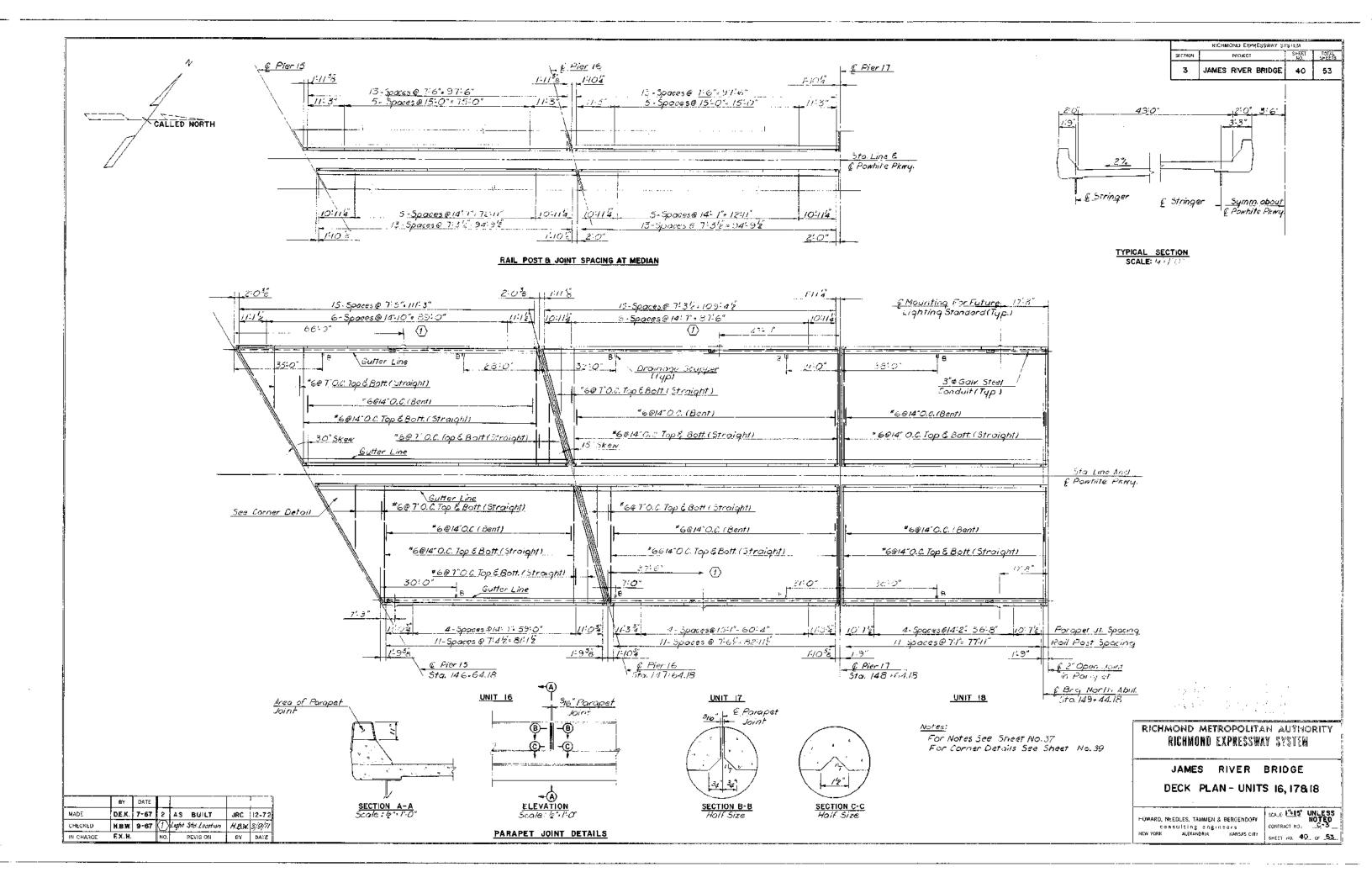
HOWARO, NEEDLES, TAMMEN & BERGENDOFF
CONSULTING ENGINEERS
NEW YORK ALEXANDRIA KANSAS CITY

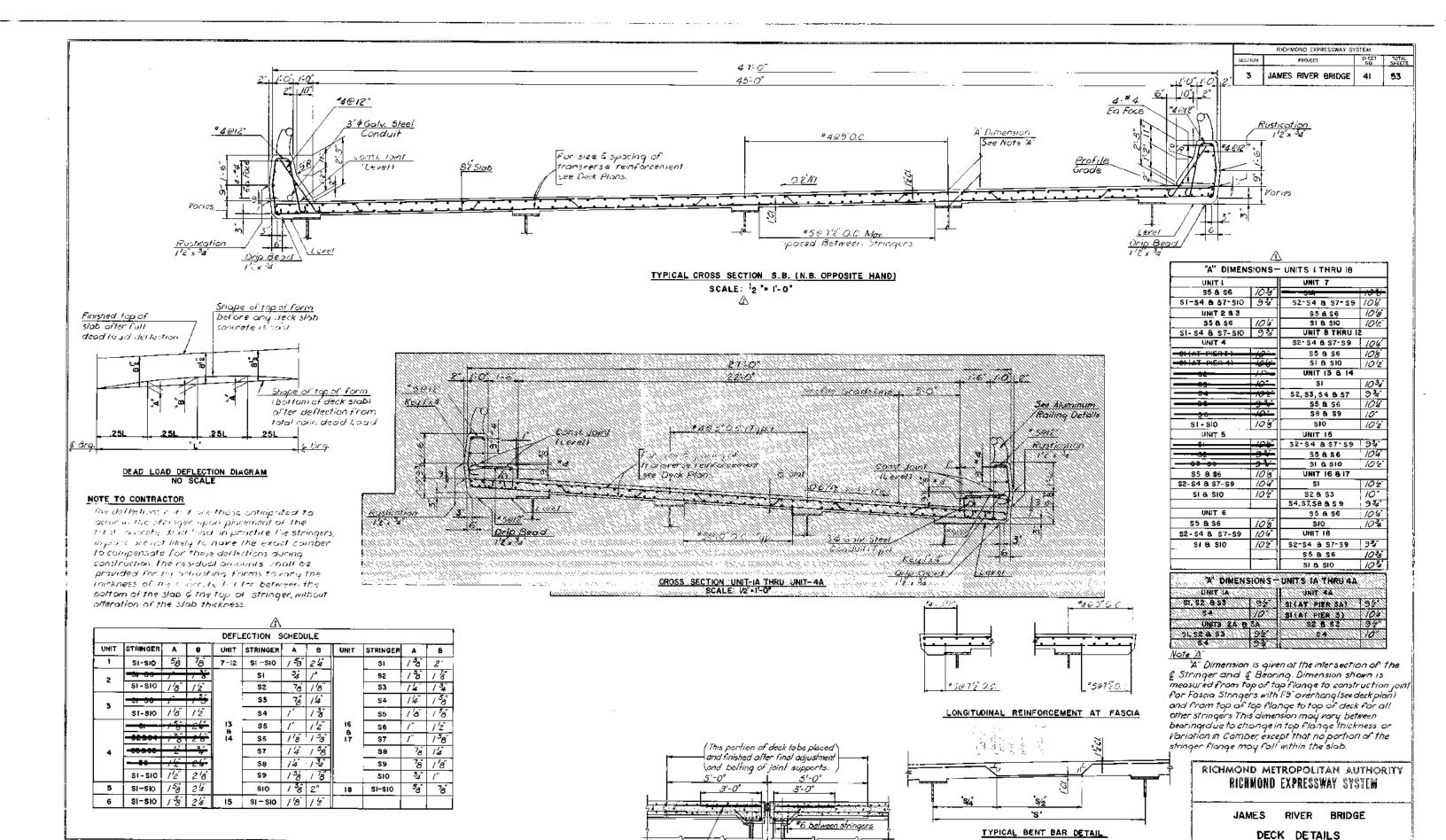
SCALE. AS SHOWN
CONTRACT NO: . C.3.
SHEET NO. 34 OF .53











#5@12"

D.E.K. 7-67 2 AS BUILT

H.B.W. 9-67 A X- Sections of Outlier lines

IN CHARGE EX.H.

JRC 12-72

EP. 2/20/71

between stringers

TYPICAL SECTION AT JOINT

<u>Notes:</u> For Notes see 5heet No.37.

All Bar elearances to conc. Pace are 2"unless nated

HOWARD, NEEDLES, TAMMEN & BERGENDOFF

COnsulting engineers

NEW YORK ALEXANDRIA KANSAS CITY

SHEET NO. 41. OF 53

## Bridge 8

Northbound and Southbound Powhite Parkway (VA-76) Parallel Bridges Over

The James River, Kanawha Canal and CSXT Railroad Tracks

**Widening Construction Plans for Bridge 8N and 8S** 

**Record Set Plans** 

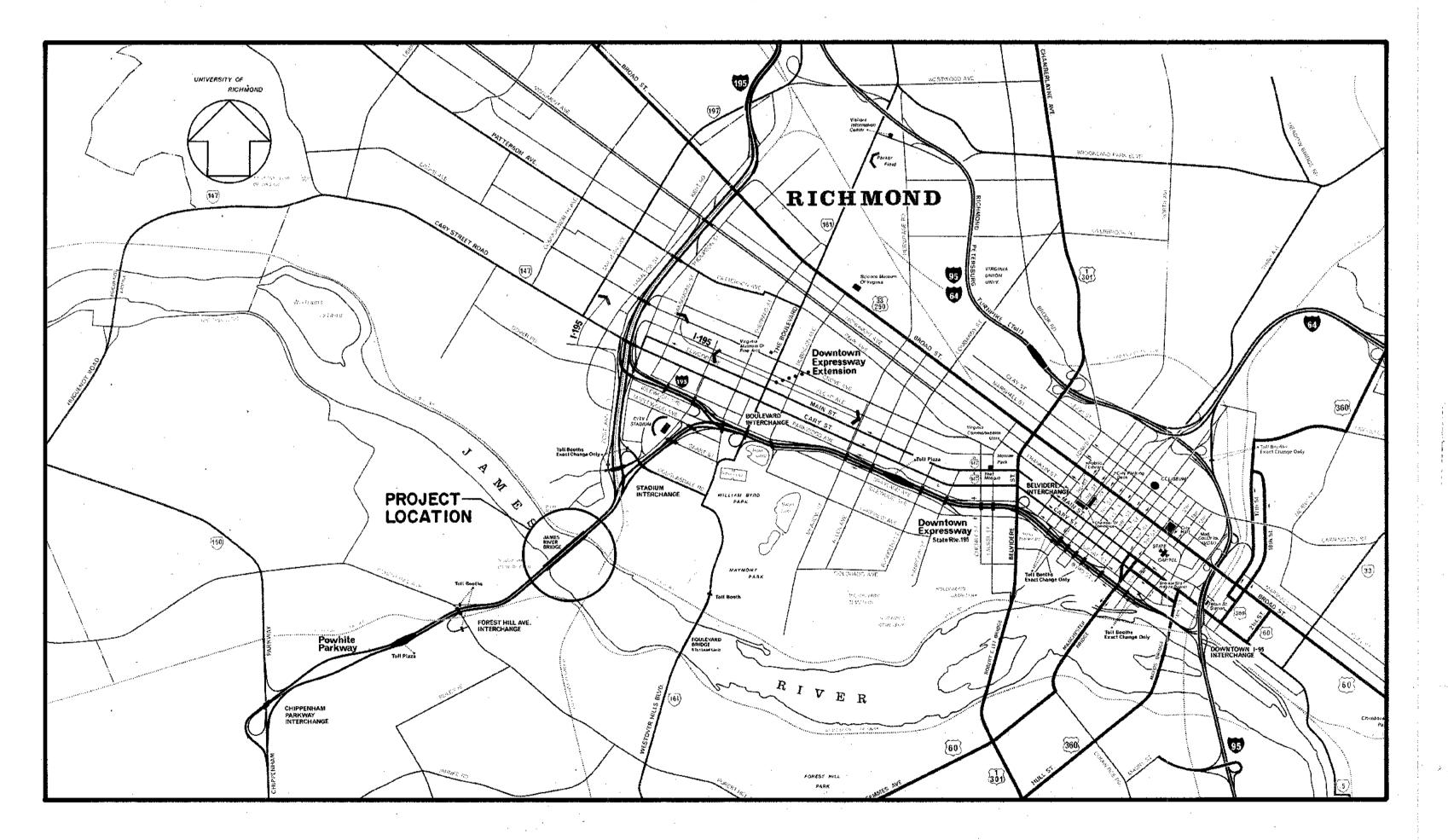
# RICHMOND EXPRESSWAY SYSTEM

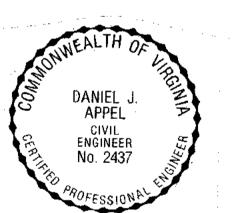
PROPOSED WIDENING

# JAMES RIVER BRIDGE

### TITLE SHEET NO. GENERAL PLAN AND ELEVATION GENERAL NOTES AND ESTIMATED QUANTITIES SOUTH ABUTMENT DETAILS NORTH ABUTMENT DETAILS FLARED TERMINAL WALL DETAILS 12-28 MISCELLANEOUS SUBSTRUCTURE DETAILS CAMBER DIAGRAMS AND SCHEDULE TYPICAL CROSS SECTION AND PARAPET DETAILS 51-65 DECK PLANS 66-67 LIGHTING STANDARD AND ELECTRICAL DETAILS DEAD LOAD DEFLECTION DIAGRAM AND SCHEDULE JOINT DETAILS DRAIN ASSEMBLY DETAILS 70 ALUMINUM RAILING DETAILS BAR LIST 72-89 BAR BENDS SLOPE PROTECTION APPROACH SLAB DETAILS 93-99 BORING LOGS CONSTRUCTION SEQUENCE AND METHOD MAINTENANCE AND PROTECTION OF TRAFFIC PARTIAL EMBANKMENT PLAN TYPICAL ROADWAY SECTION EMBANKMENT CROSS SECTION 104-105 EROSION CONTROL 106

INDEX OF SHEETS





	SUBMITTED BY
Date	
3-28-87	
Date	HOWARD, NEEDLES, TAMMEN & BERGENDOF

	RECOMMENDED BY
Date	
3.29-87 Date	GENERAL MANAGER, RICHMOND METROPOLITAN AUTHORITY

	APPROVED BY
3-29-87	
Date	CHAIRMAN, RICHMOND METROPOLITAN AUTHORITY

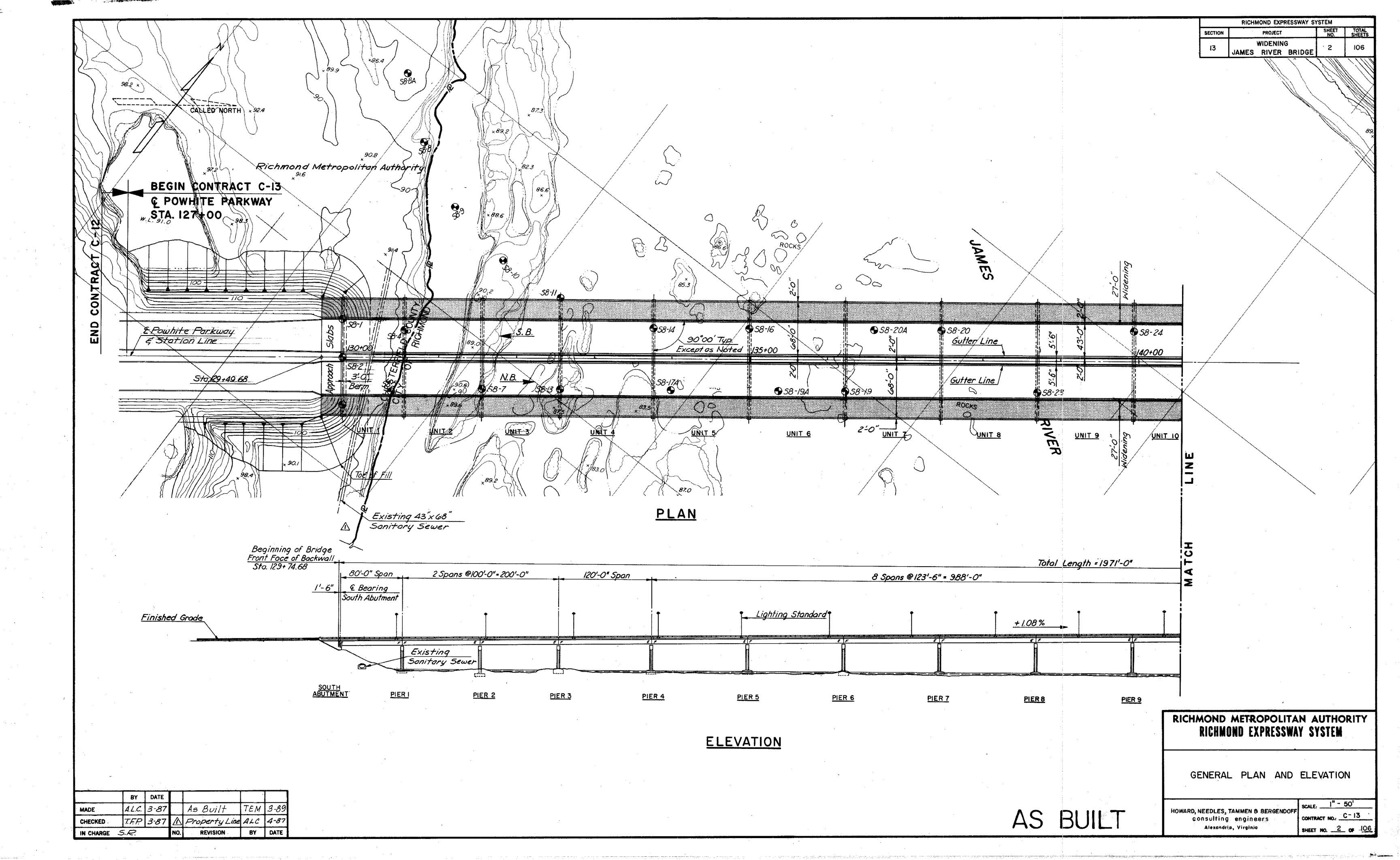
Plans Revised													
Sheet No.	Date	Sheet No.	Date										
		,											
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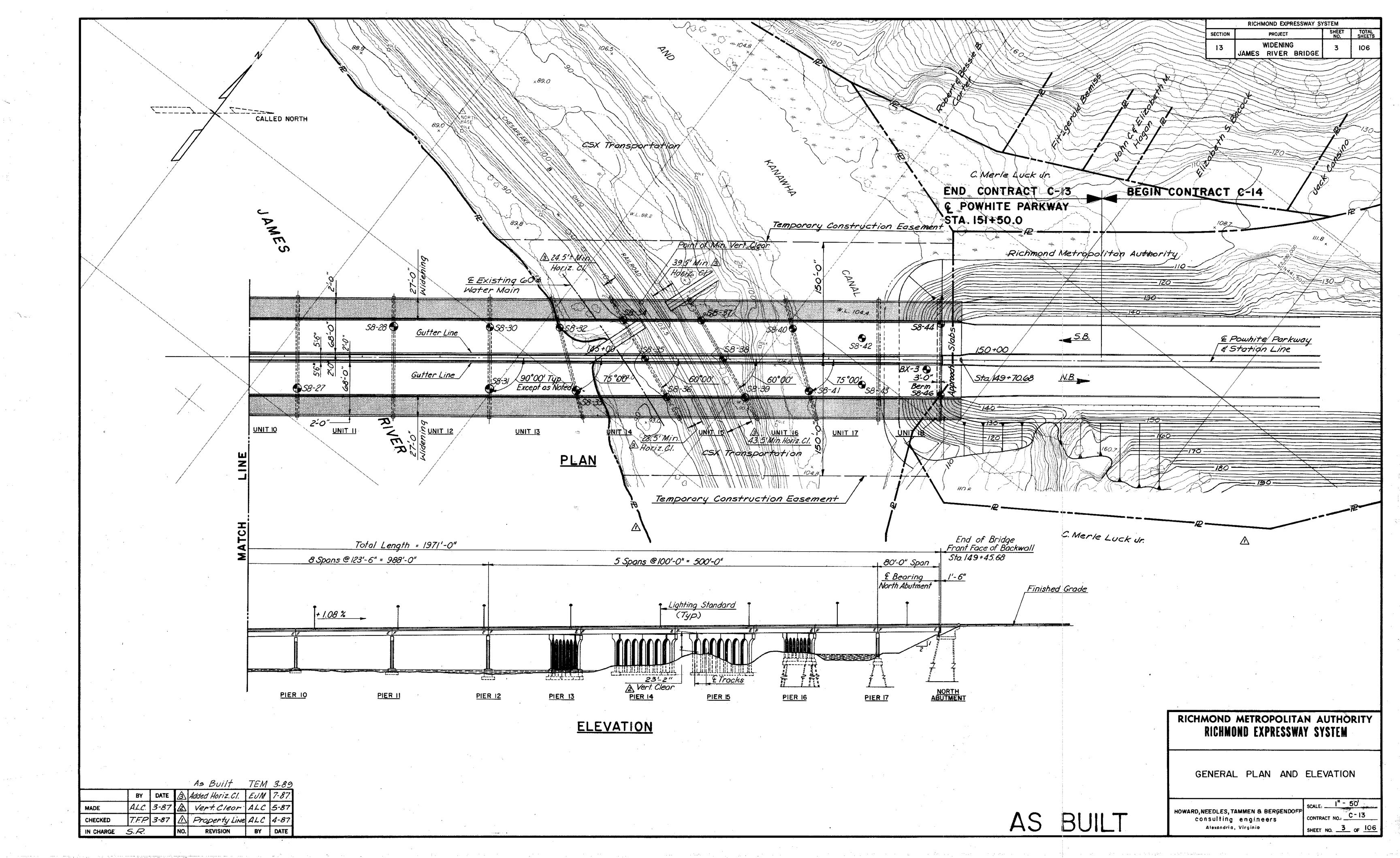
CONVENTIONAL SIGNS

STATE LINE CITY, TOWN OR VILLAGE RIGHT OF WAY LINE... FENCE LINE TROLLEY POLES FENCED PROPERTY LINE GUARD RAIL... HEDGE. RAILROADS GROUND ELEVATION. BASE OR SURVEY LINE. GRADE ELEVATION. POLES WITHIN CONSTRUCTION LIMITS....

CONTRACT C-13

AS BUILT





						•				,		,																					13	JAMES	IDENING RIVER BE		4
									<del> </del>				· · · · · · · · · · · · · · · · · · ·				<u></u>	ESTIM/	TE OF	QUANTITIES	<del>*</del>						· .				····	-					
	CONSTRUCTION SURVEY	EMBANKMENT	TEMPORARY SILT FENCE	FILTER CLOTH	STRUCTURE EXCAVATION AT	STRUCTURE	POROUS BACKFILL	STEEL SHEET PILING	STEEL PILES 108P42	DRIVING TEST FOR STEEL PILE 108P42	CONCRETE CLASS A3 SUBSTRUCTURE	CONCRETE CLASS A4 APPROACH SLABS CONCRETE	CLASS A4	CONCRETE	REINFORCING STEEL	EPOXY COATED REINFORCING STEEL		ORY RIPRAP CLASS I	RIPRAP FILTER CLOTH BEDDING	DAMPPROOFING	UNDERDRAIN 6"DIA.	MOBILIZATION	OFFIC				PARTIAL DEMOLITION OF EXISTING SUBSTRUCTURE	PARTIAL DEMOLITION OF EXISTING SUPERSTRUCTURE	TEMPORARY CAUSEWAYS AND ACCESS ROADS		ELASTOMETRIC EXPANSION DAM, 0-2"	-					
	L.S.	C.Y.	L.F.	S.Y.	C.Y.	C.Y.	C.Y.	S.F.	L.F.	L.F.	C.Y.	C.Y. C	X.	L.F.	LBS.	L.BS.	L.S.	S.Y.	S.Y.	S.Y.	L.F.	L.S	s. MTI	н.			L.S.	L.S.	L.S.		L.F.				4		
UPERSTRUCTURE												30.	91.6	4038	3	604,840	1	×					······································						ļ		2750						
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PIER II					134						220.4				26080														<u> </u>				 ļ <del> </del>				
PIER 12					95						226.0				26640							 							<u> </u>				<u> </u>				
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APPROACH SLAB												99.2			15100					,		 											ļ <sup>1</sup>				

2341 336

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### GENERAL NOTES

WIDTH:

TOTALS

WIDENING OF 27'-0" TO THE EAST AND WEST OF THE

EXISTING STRUCTURE.

4670 1735 23

SPAN LAYOUT:

18-SIMPLE SPAN, STEEL PLATE GIRDER WITH SPANS OF

2540 728

2-80 FEET, 7-100 FEET, 1-100 FEET AND 8-123.5 FEET.

CONSTRUCTION TOLERANCES AND CONSTRUCTION METHODS.

CAPACITY:

HS20-44 LOADING AND ALTERNATE MILITARY LOADING. DESIGN LOADING INCLUDES 15 P.S.F. FOR FUTURE WEARING SURFACE AND 20 P.S.F. ALLOWANCE FOR

SPECIFICATIONS:

CONSTRUCTION: VIRGINIA DEPARTMENT OF HIGHWAYS AND TRANSPORTATION ROAD AND BRIDGE SPECIFICATIONS, 1982.

DESIGN:

AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, 1983, INCLUDING INTERIM SPECIFICATIONS, 1984, AND

VDHT MODIFICATIONS.

STANDARDS:

VIRGINIA DEPARTMENT OF HIGHWAYS AND TRANSPORTATION

ROAD AND BRIDGE STANDARDS, 1982.

ALL DIMENSIONS ARE MEASURED HORIZONTALLY AND VERTICALLY UNLESS OTHERWISE SPECIFIED.

THESE PLANS ARE INCOMPLETE UNLESS ACCOMPANIED BY THE SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS INCLUDED IN THE CONTRACT DOCUMENTS.

4038 356,060 606,020

ALL STRUCTURAL STEEL SHALL BE A36, EXCEPT AS NOTED.

4454 4715 48 29466 992 30916

FINISH PAINT SHALL BE LIGHT GREEN, FEDERAL STANDARD 595-24227.

CONCRETE IN SUPERSTRUCTURE SHALL BE CLASS A4. ALL OTHER CONCRETE, EXCEPT TREMIE SEAL CONCRETE, SHALL BE CLASS A3.

ALL DEFORMED REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60.

ALL REINFORCING BAR DIMENSIONS ON THE DETAILED DRAWINGS ARE TO CENTER OF BARS EXCEPT WHERE OTHERWISE NOTED AND ARE SUBJECT TO FABRICATION AND CONSTRUCTION TOLERANCES.

ALL FOOTING CONCRETE SHALL BE PLACED IN THE DRY EXCEPT WHERE CLASS T3 CONCRETE IS INDICATED ON THE PLANS OR APPROVED BY THE ENGINEER.

BECAUSE OF THE ERRATIC NATURE OF THE GROUND CONDITIONS AT THE SITE, ELEVATIONS OF THE BOTTOM OF THE FOOTINGS SHOWN ON THE PLANS SHALL BE CONSIDERED APPROXIMATE ONLY. DURING CONSTRUCTION SHOULD EXCAVATION OPERATIONS REVEAL THAT THE FOUNDATION IS INADEQUATE, REDESIGN MAY BE REQUIRED AT THE DISCRETION OF THE ENGINEER.

STEEL PILES 10BP42 ARE DESIGNED FOR A CAPACITY OF 55 TONS PER PILE. ALL PILES SHALL BE DRIVEN TO PRACTICAL REFUSAL.

PRIOR TO THE COMMENCEMENT OF ANY WORK THE CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS AND ELEVATIONS OF THE EXISTING STRUCTURE AND REPORT TO THE ENGINEER ANY DISCREPANCIES BETWEEN FIELD MEASUREMENTS AND MEASUREMENTS SHOWN ON THE CONTRACT PLANS.

A ELEVATIONS TAKEN FROM CITY OF RICHMOND DATUM.

180 190

\* For additional quantities see Sheet Nos. 67 \$ 101,

RICHMOND EXPRESSWAY SYSTEM

PROJECT

SECTION

\*\* The total weight of structural steel, 3,541,900 lbs, includes the shoes.

2750

## RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

GENERAL NOTES AND

ESTIMATED QUANTITES

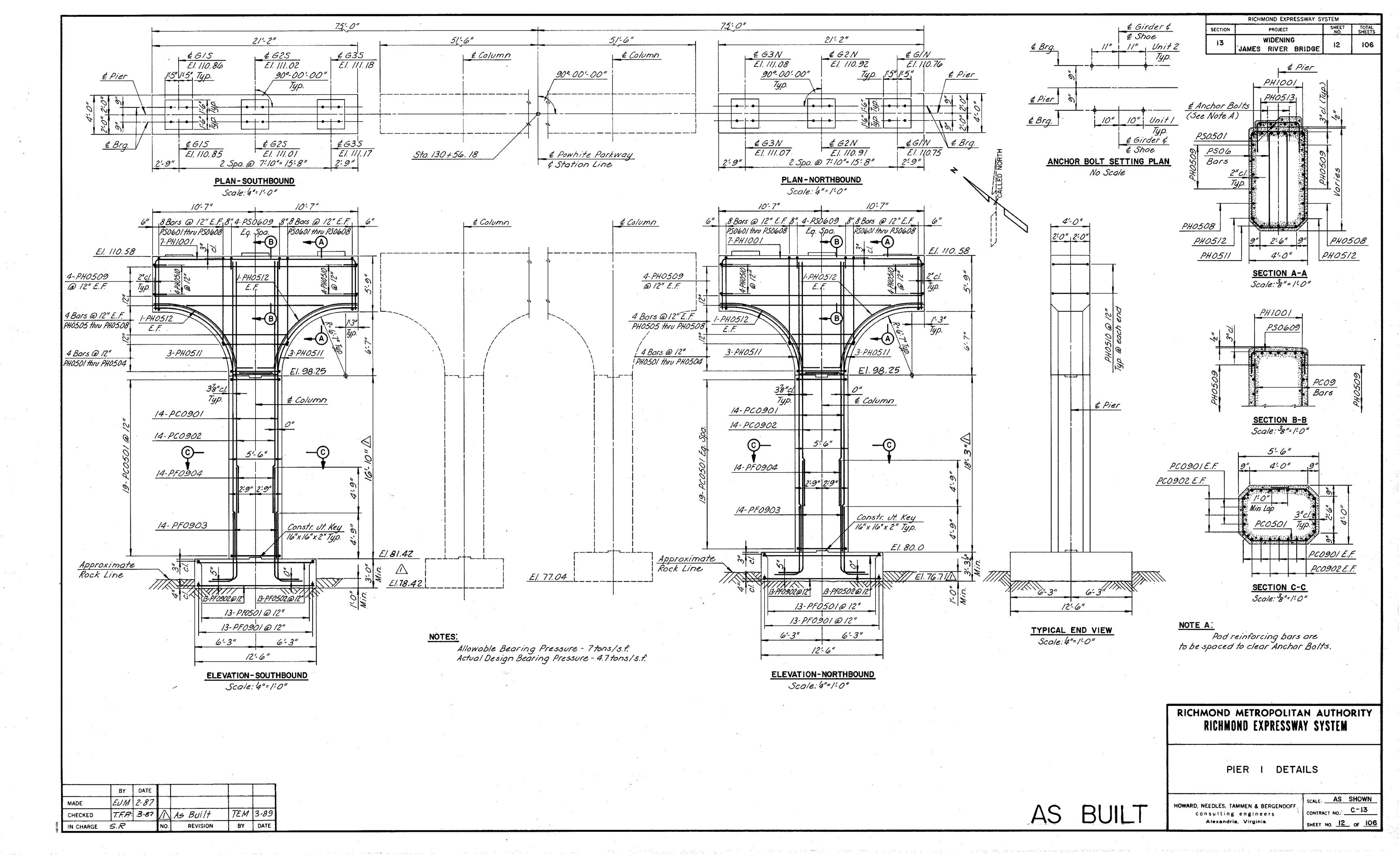
HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers

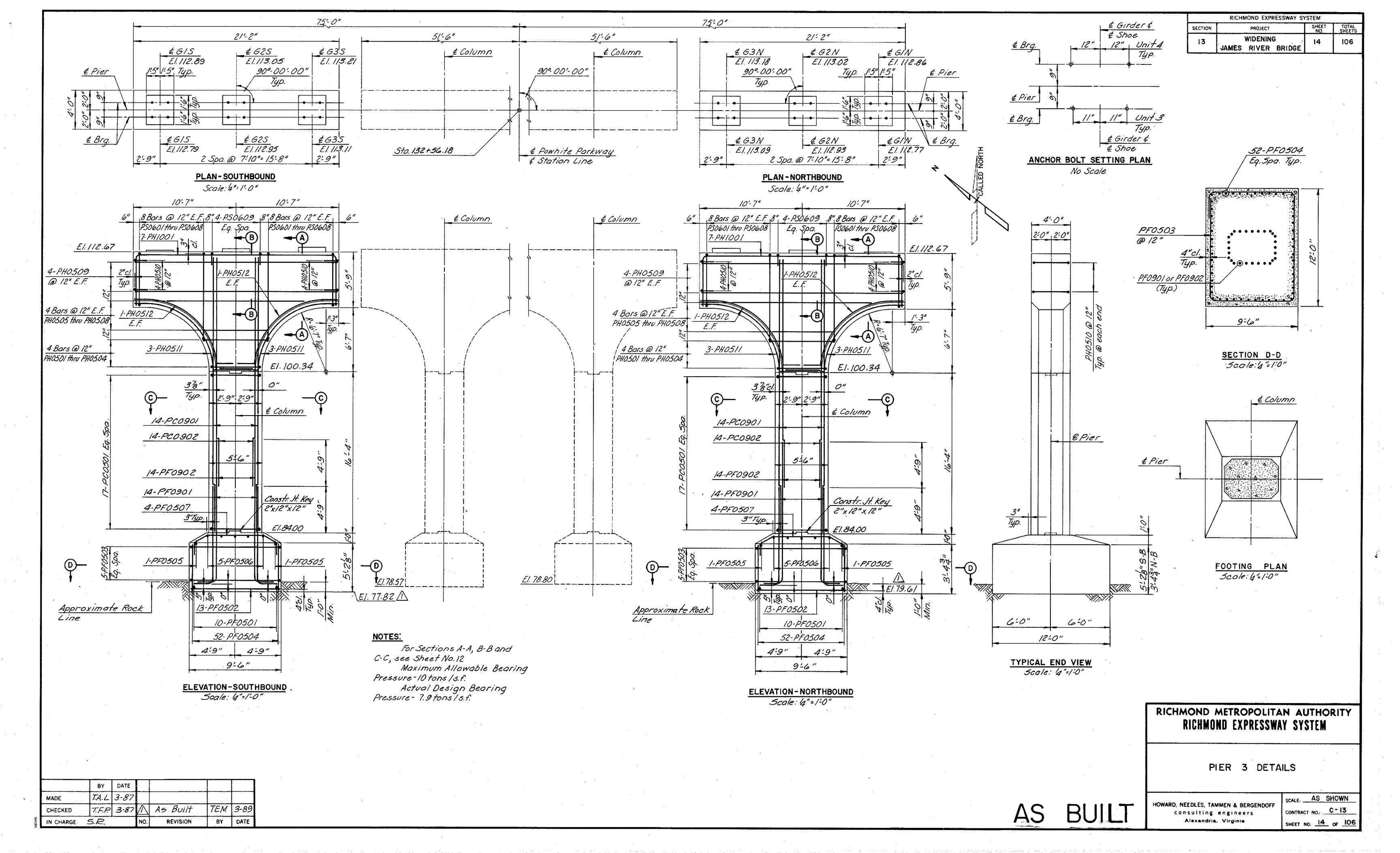
SCALE: AS SHOWN CONTRACT NO.: C - 13 Alexandria, Virginia SHEET NO. 4 OF 106

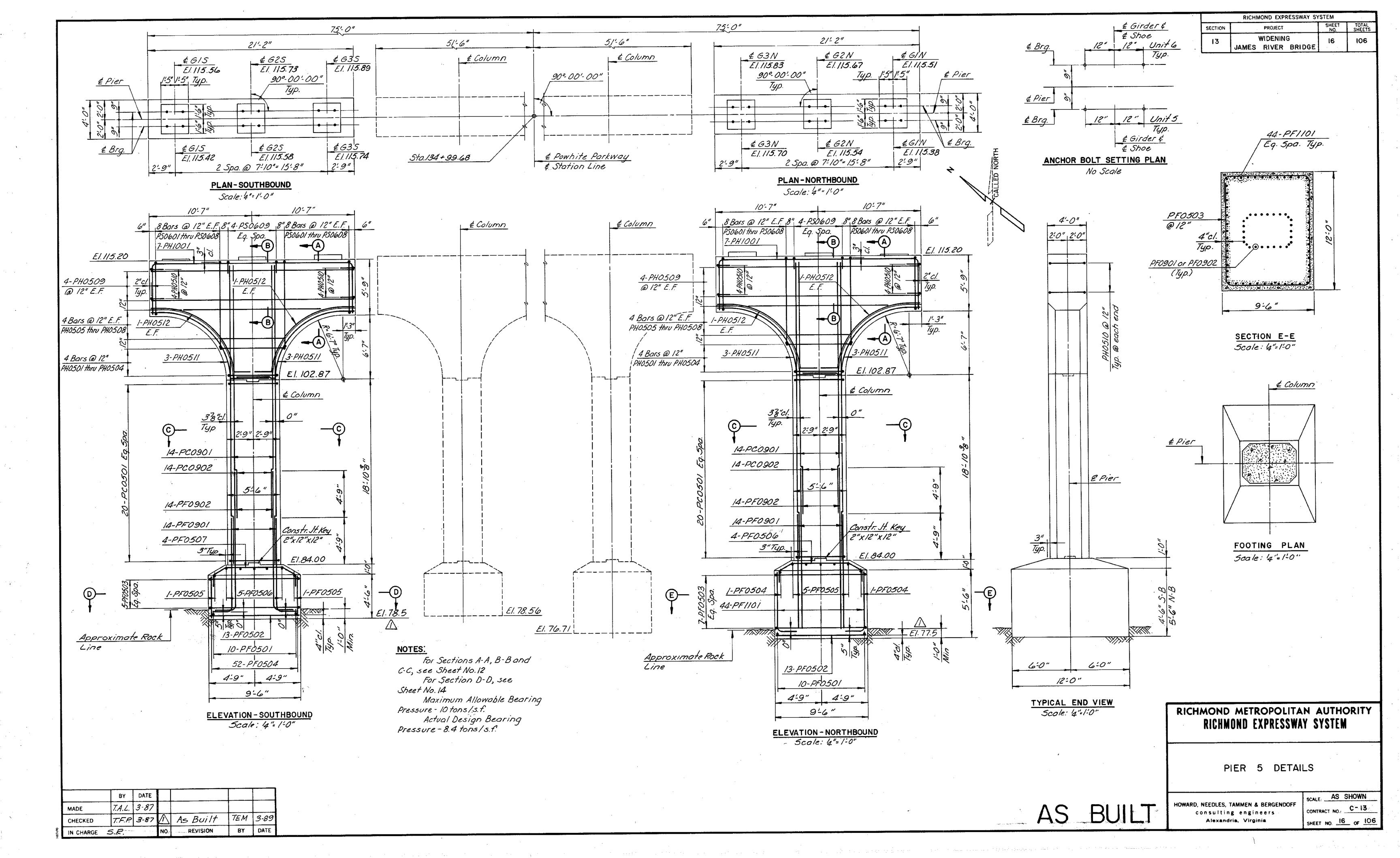
A As Built TEM 3-89

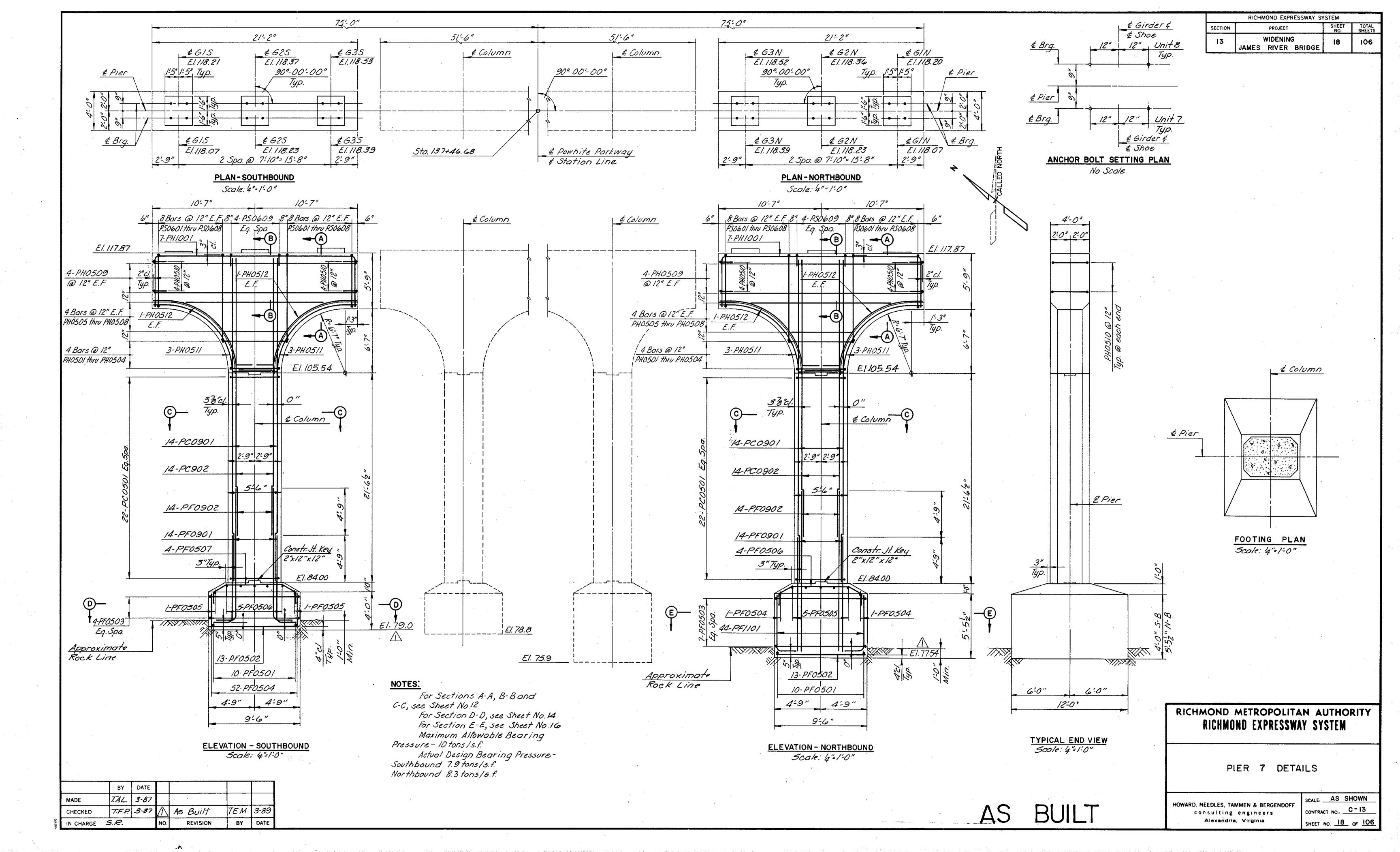
BY DATE A Quantities ALC 4-87 ALC 3-87 & Quantities ALC 4-87 MADE T.F.P. 3-87 A Quantities & ALC 4-87 CHECKED DATE REVISION IN CHARGE S.R.

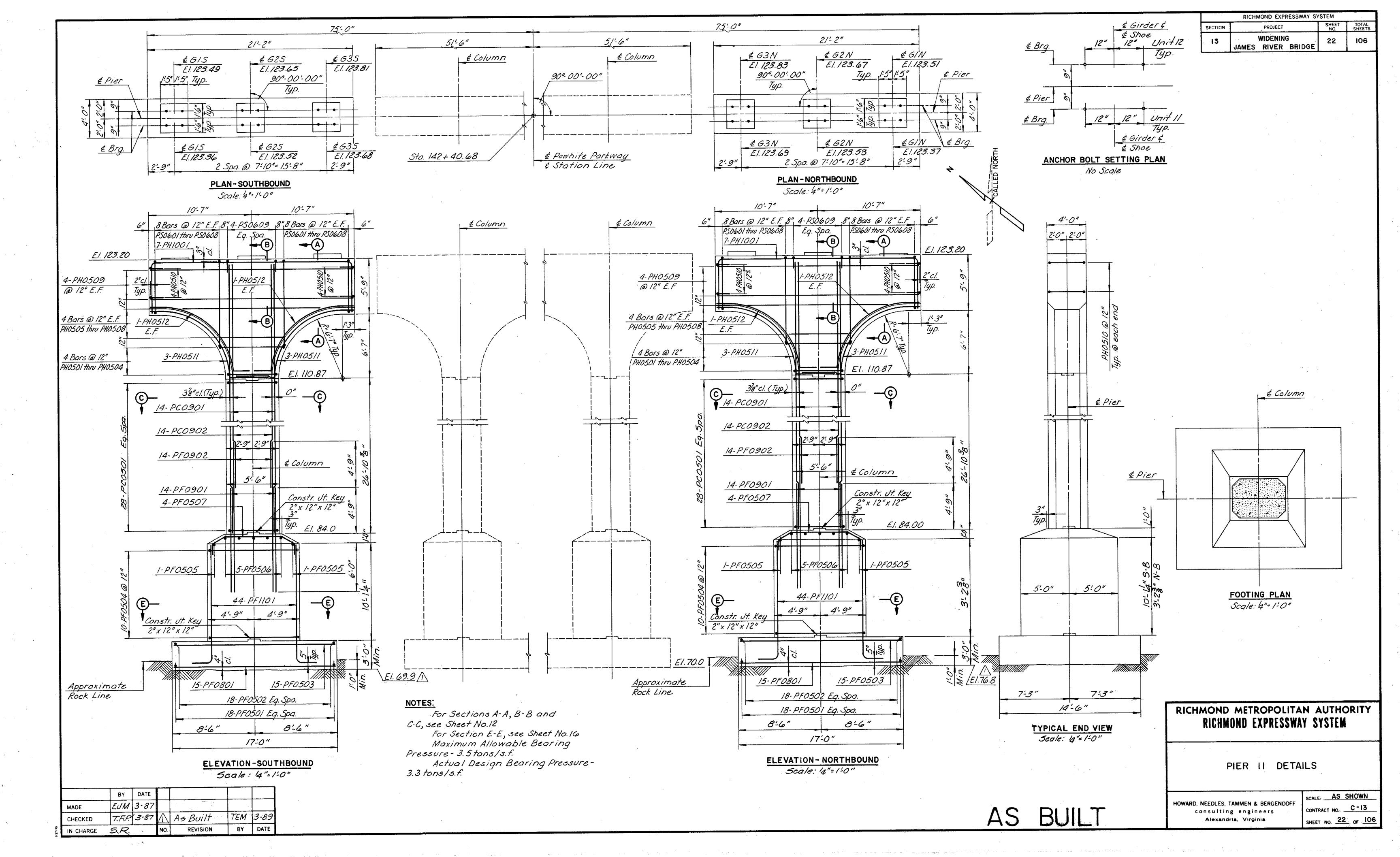
AS BUILT

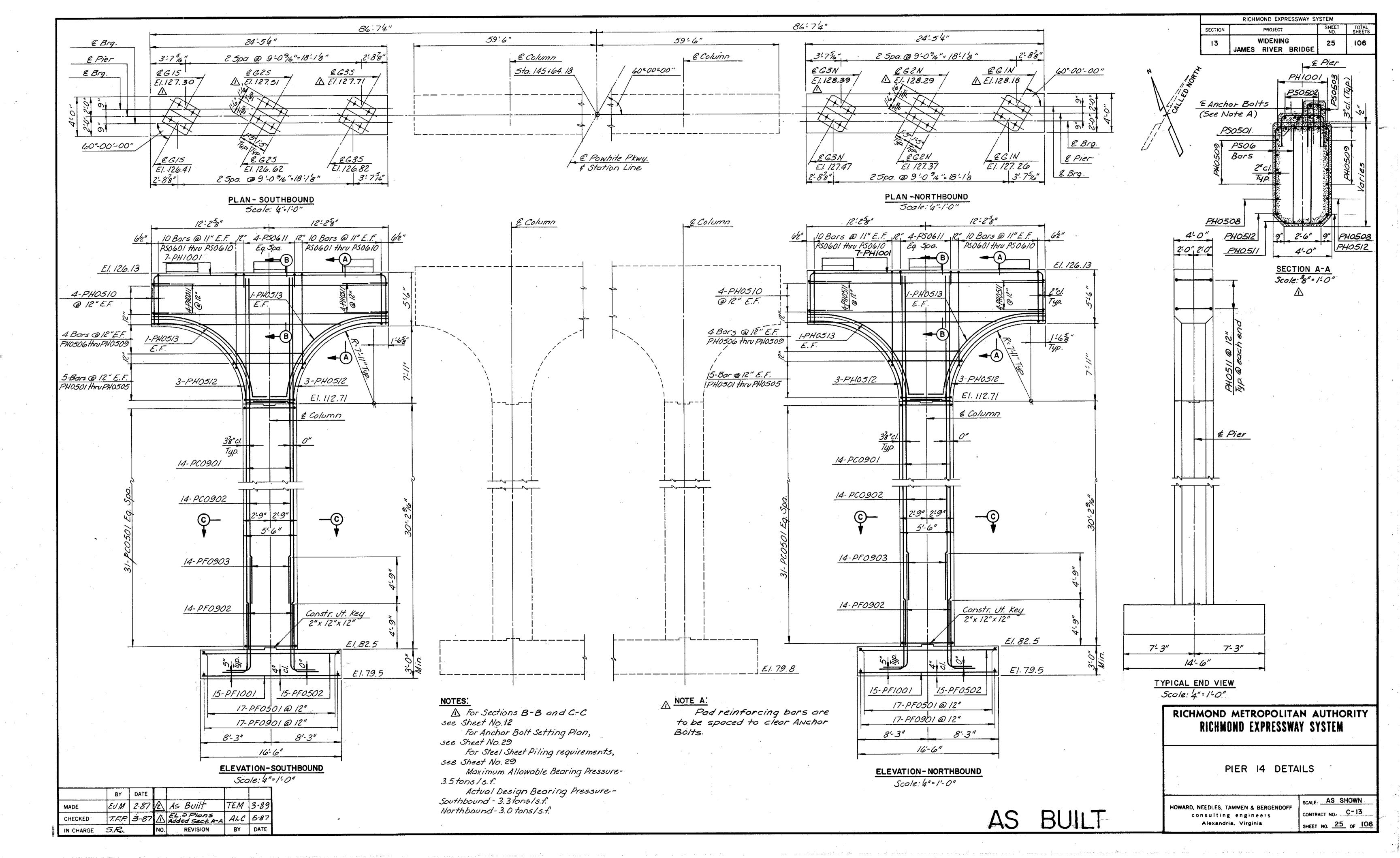


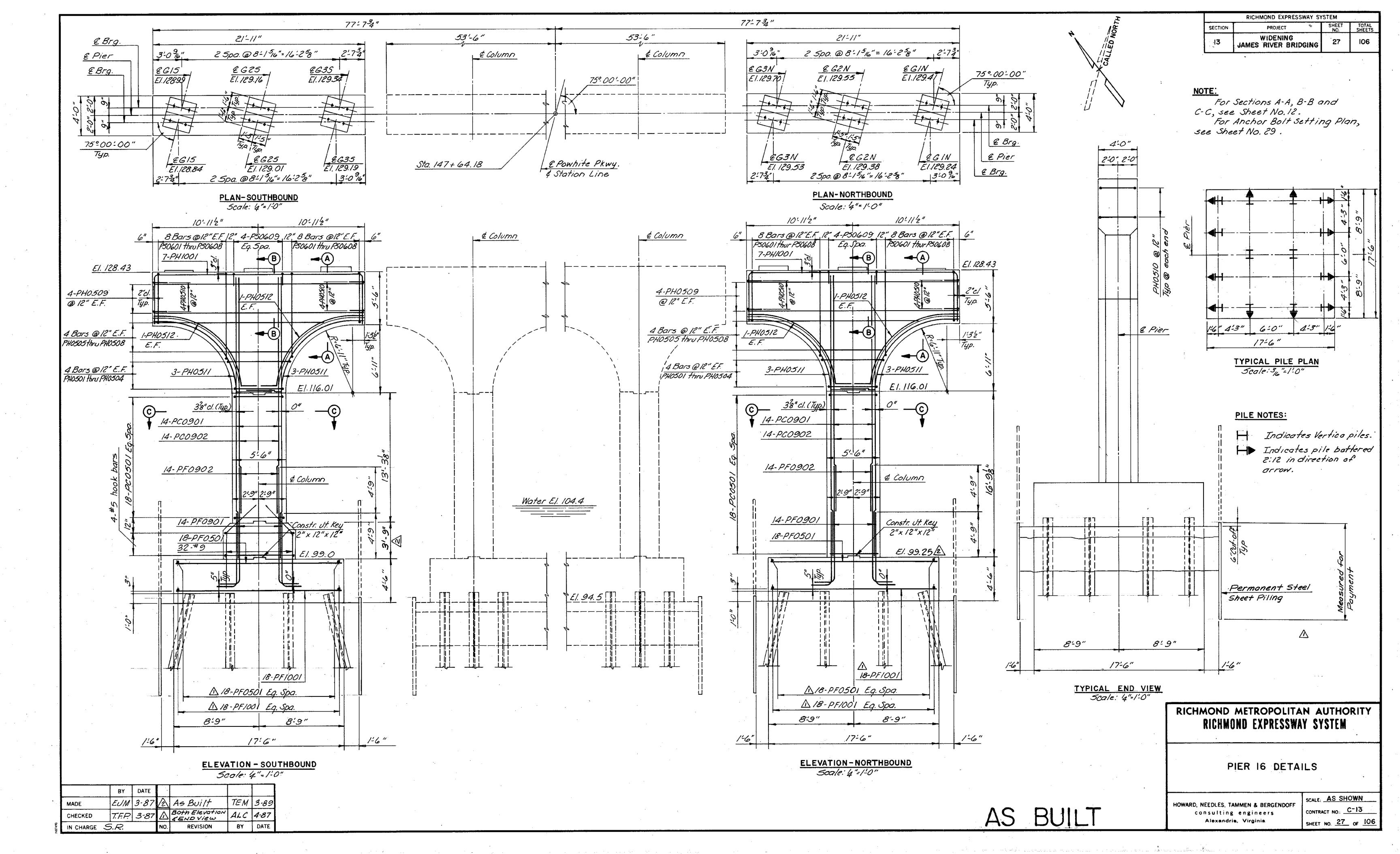


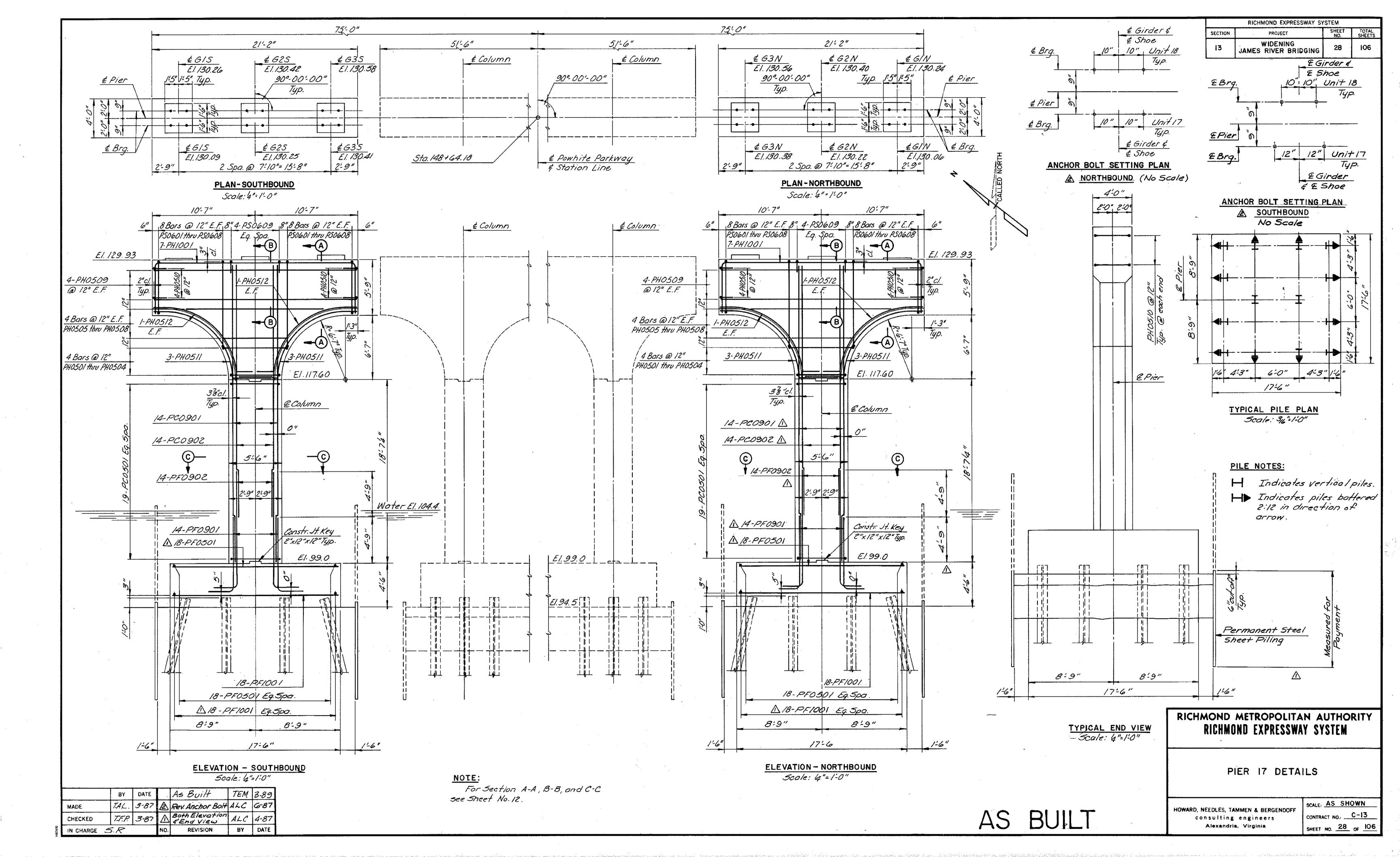


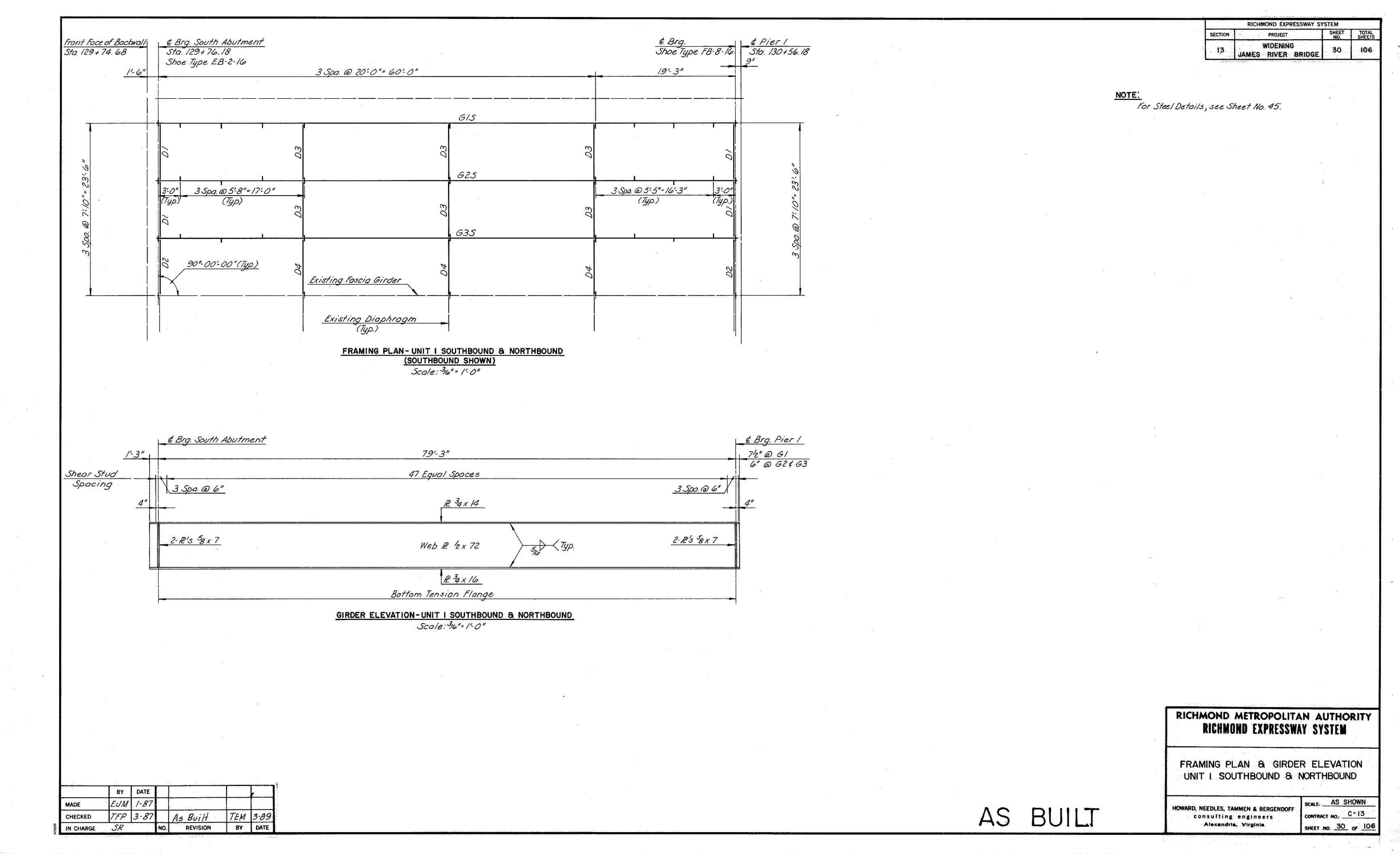


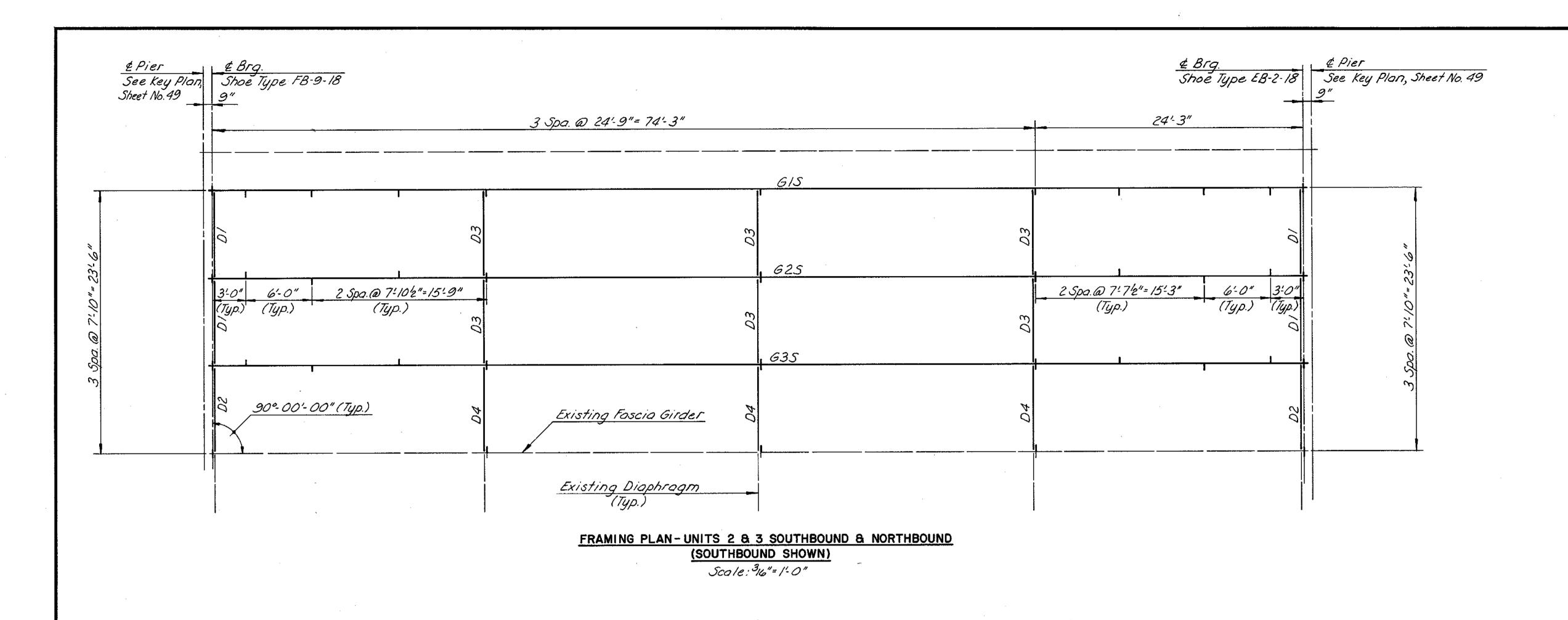


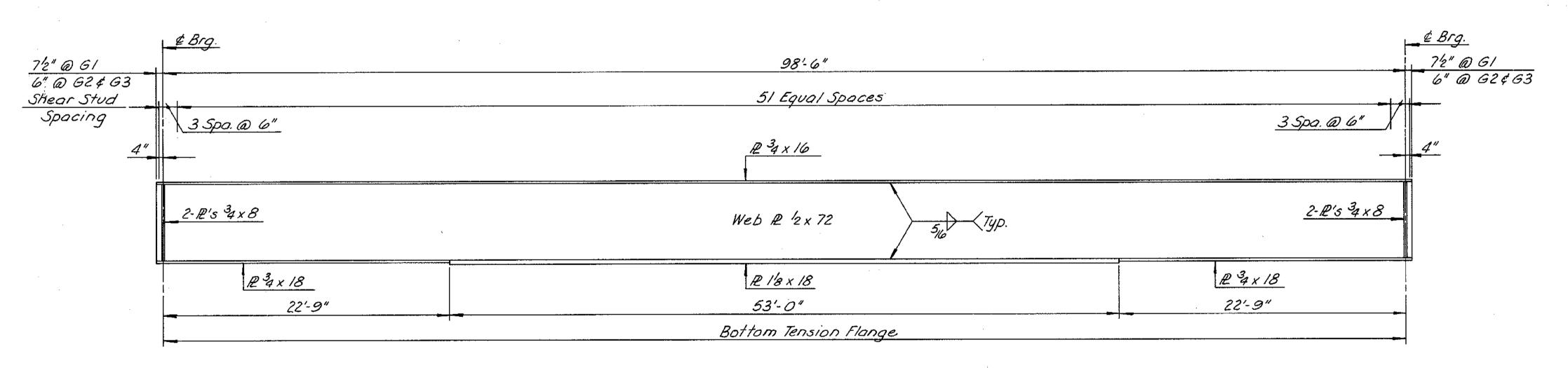












GIRDER ELEVATION-UNITS 2 & 3 SOUTHBOUND & NORTHBOUND

Scale: 316" = 1'-0"

<u>.</u>					•		
		BY	DATE				
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	CHECKED	TFP	3-87		As Built	TEM	3-89
ŝ	IN: CHARGE	SR	•	NO.	REVISION	BY	DATE

AS BUILT

RICHMOND EXPRESSWAY SYSTEM

SECTION PROJECT SHEET TOTAL SHEETS

WIDENING JAMES RIVER BRIDGE 31 106

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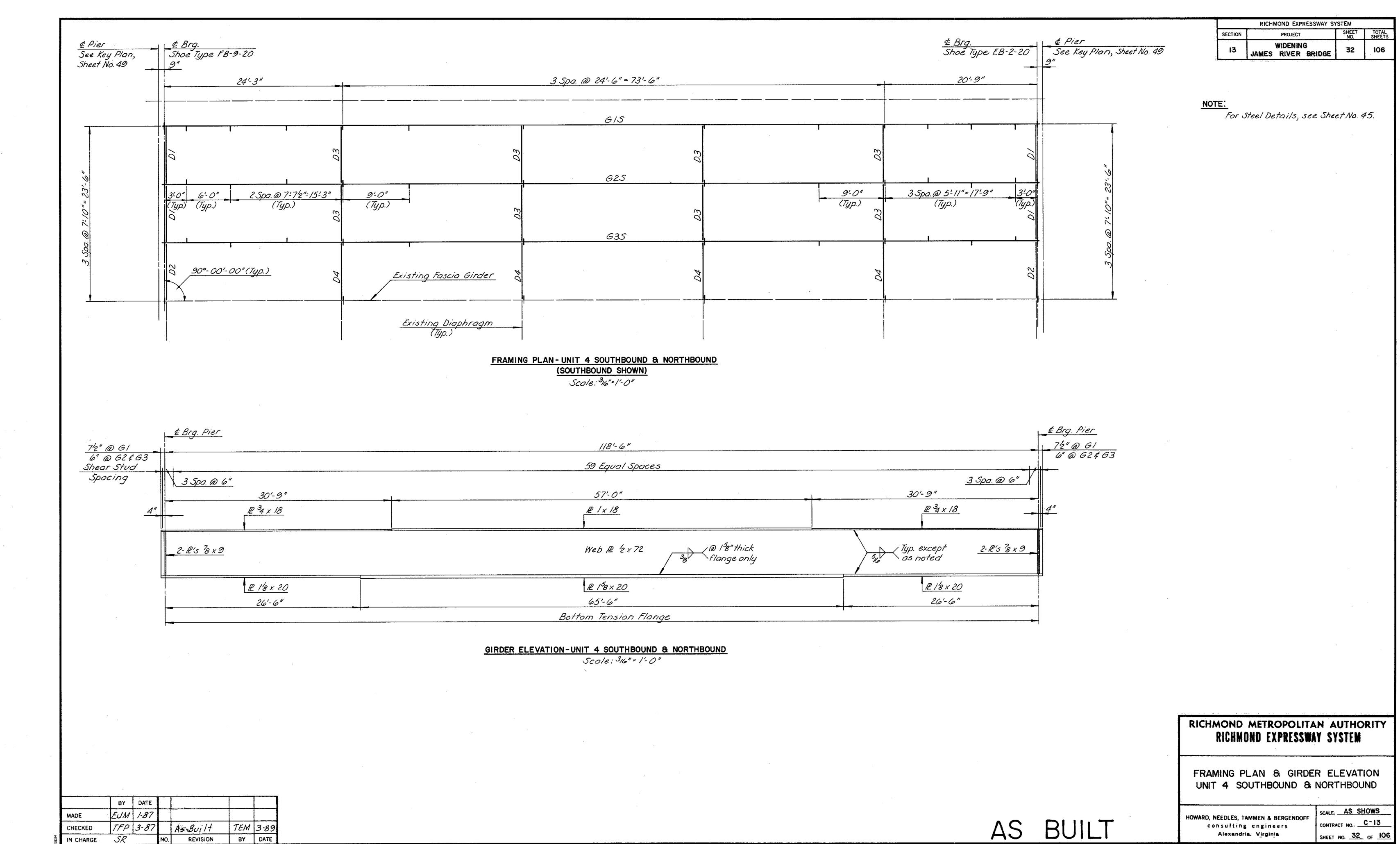
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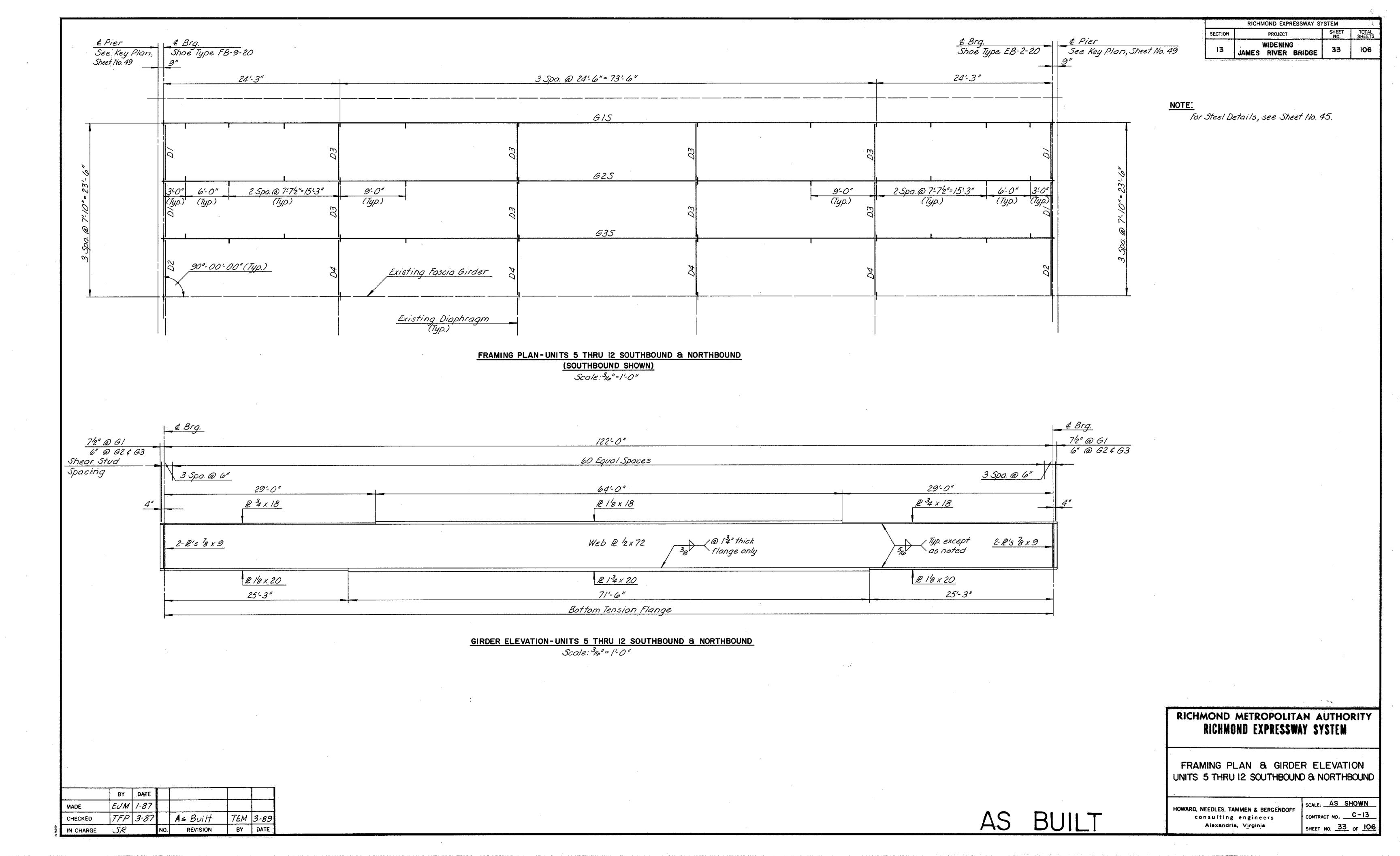
RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

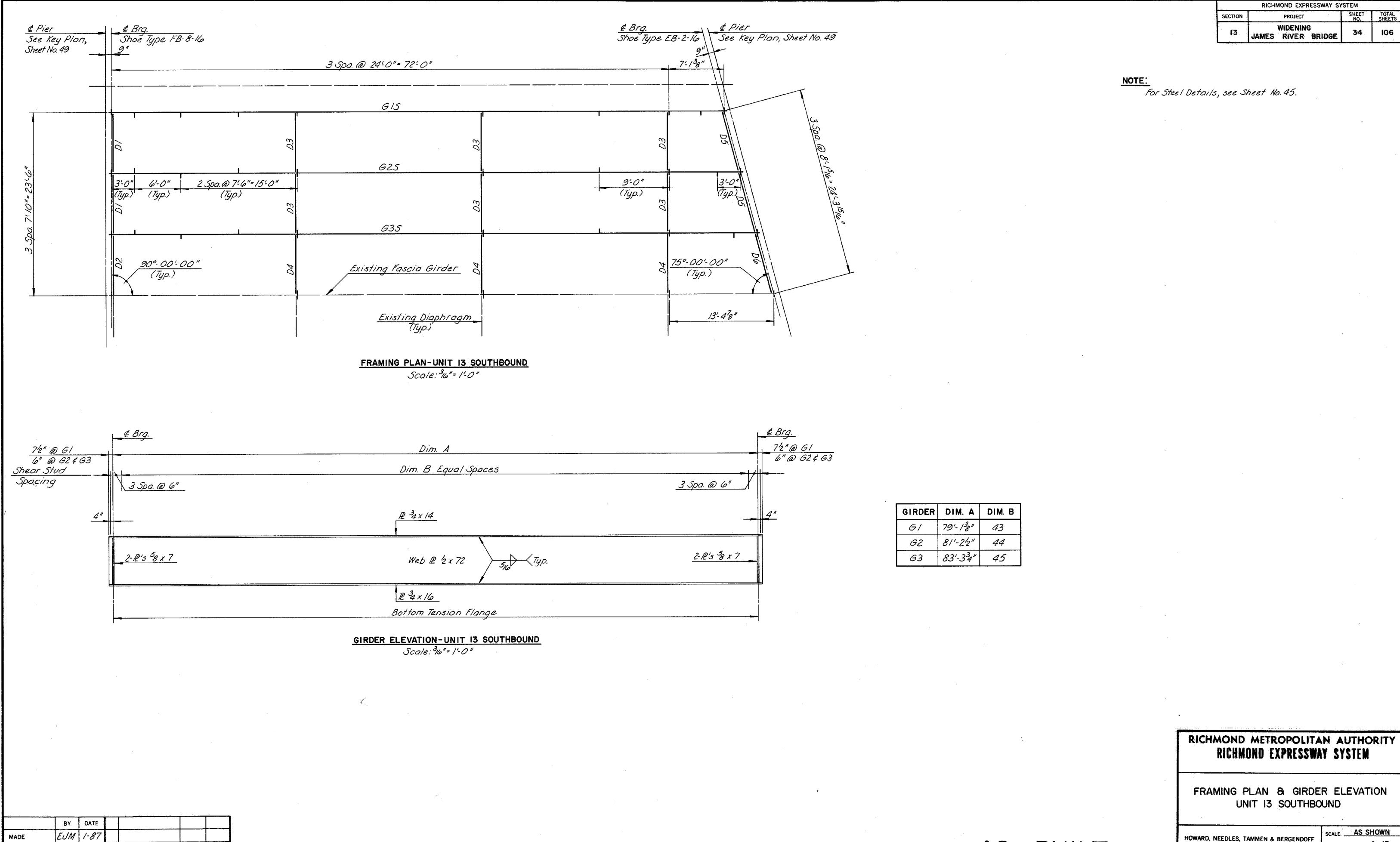
FRAMING PLAN & GIRDER ELEVATION
UNITS 2 & 3 SOUTHBOUND & NORTHBOUND

HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers
Alexandria, Virginia

SCALE: AS SHOWN
CONTRACT NO.: C-13
SHEET NO. 31 OF 106







 TEM
 3-89

 BY
 DATE

TFP 3-87

SR

IN CHARGE

As Built

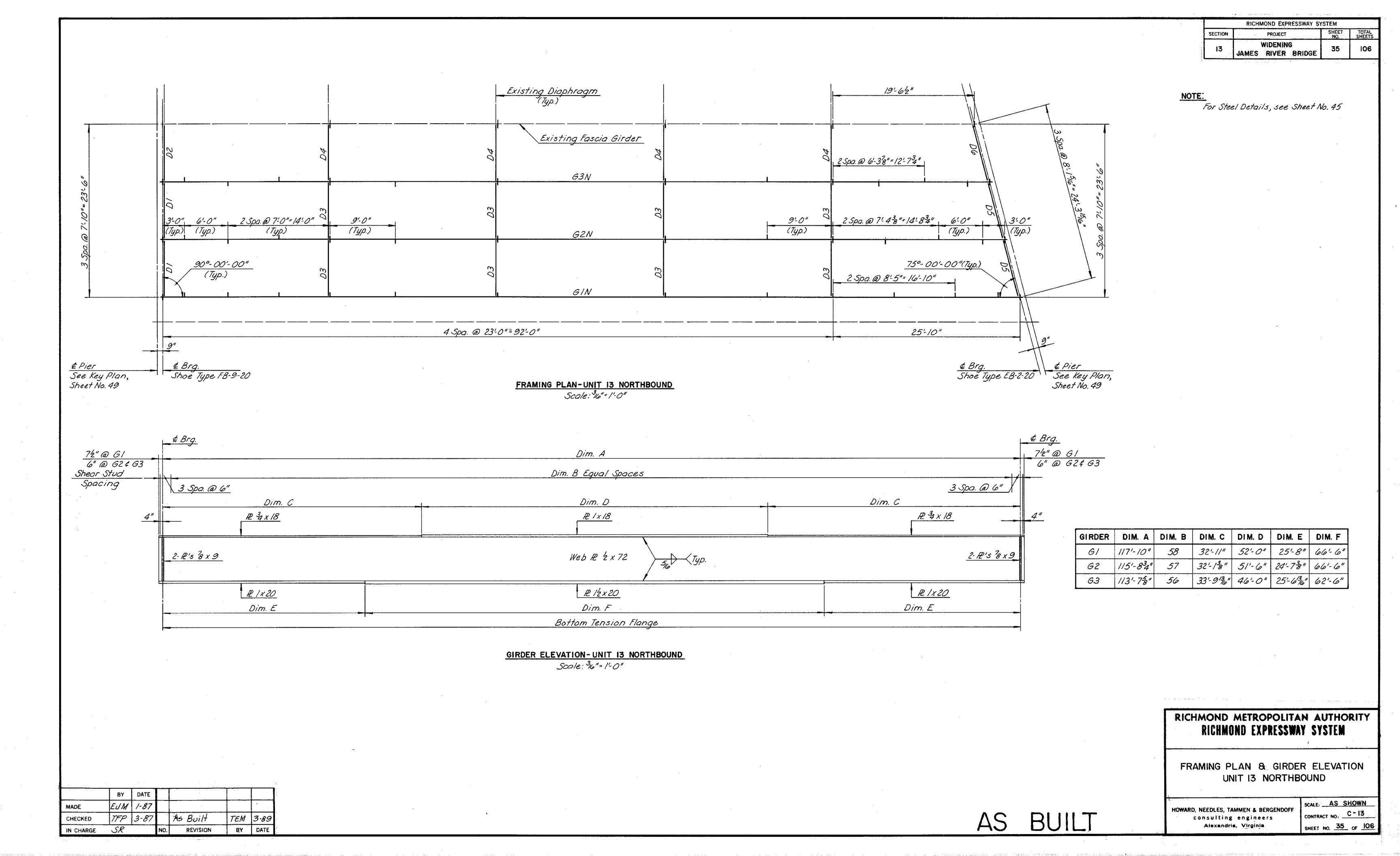
REVISION

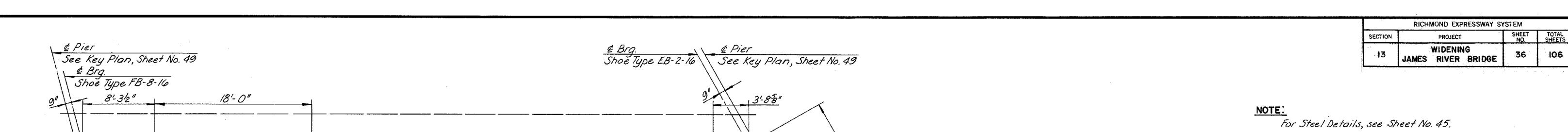
AS BUILT Alexandria, Virginia

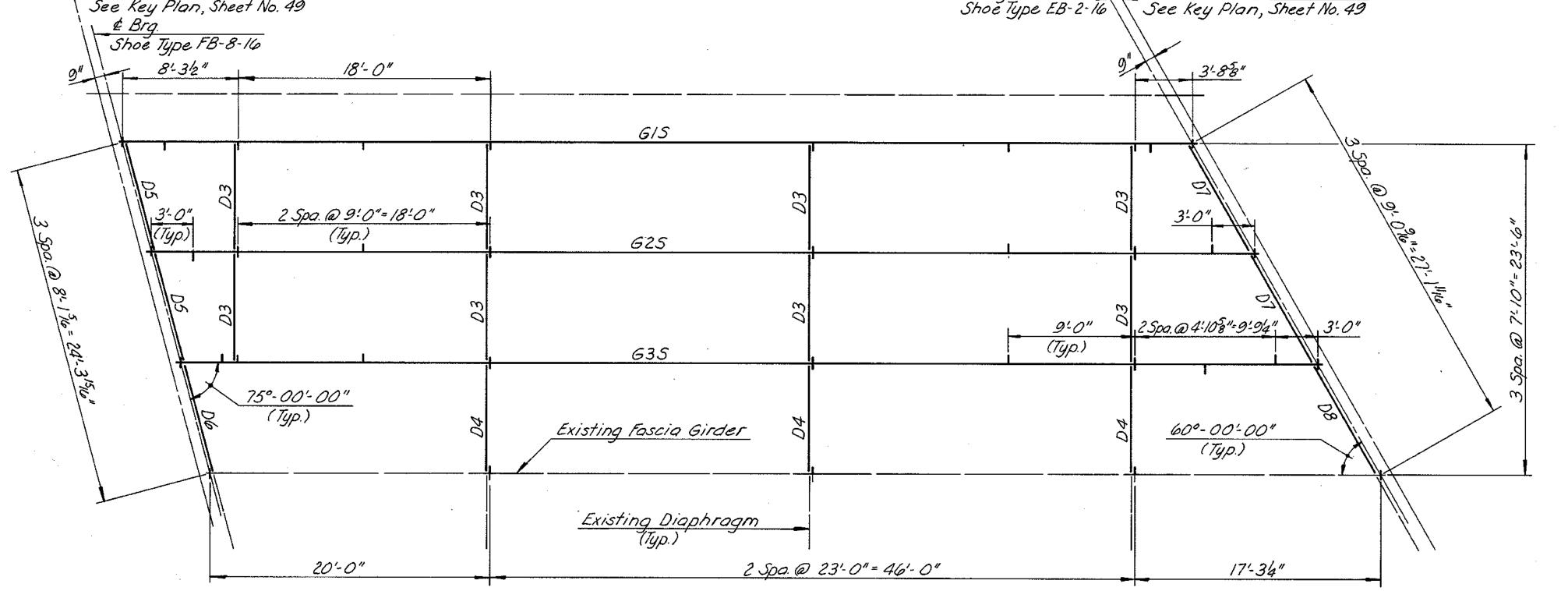
SCALE: AS SHOWN HOWARD, NEEDLES, TAMMEN & BERGENDOFF CONTRACT NO.: C-13 consulting engineers

SHEET NO. 34 OF 106

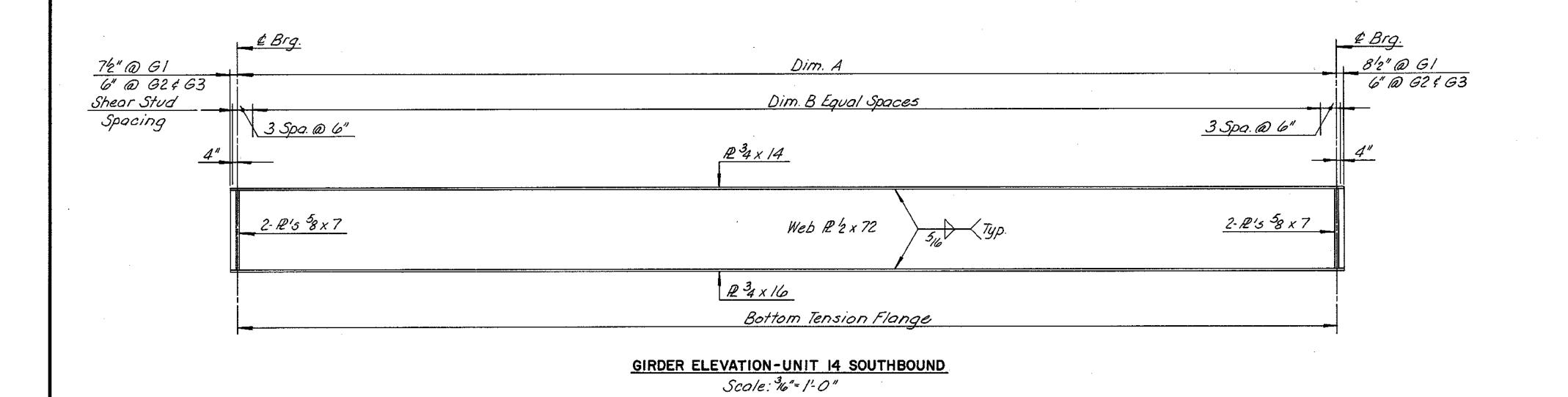
34







### FRAMING PLAN-UNIT 14 SOUTHBOUND Scale: 36"= 1-0"



GIRDER	DIM. A	DIM. B
G1	76'-0"	43
G2	78'-5'8"	44
<i>6</i> 3	80'-104"	46

# RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

FRAMING PLAN & GIRDER ELEVATION
UNIT 14 SOUTHBOUND

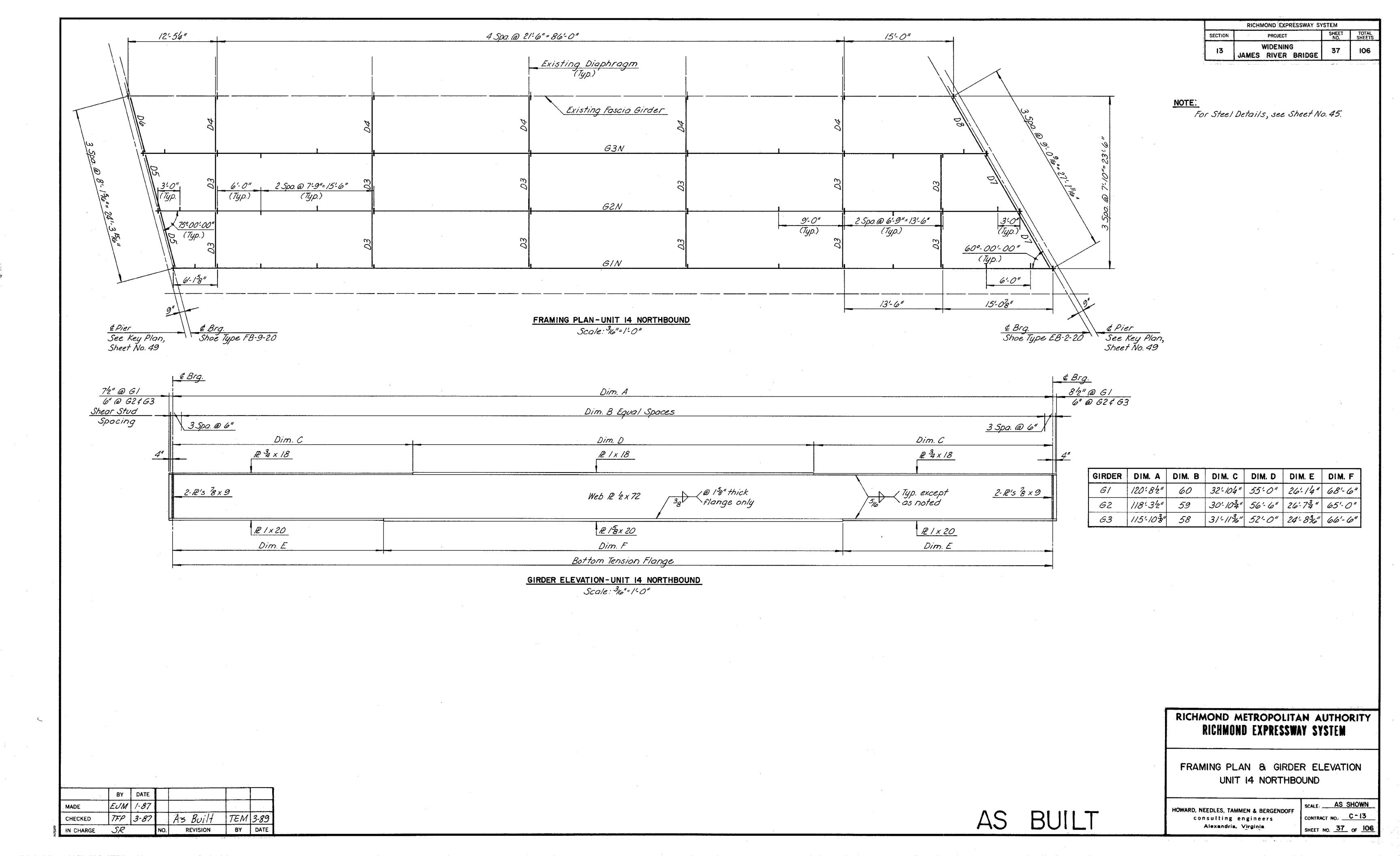
HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers
Alexandria, Virginia

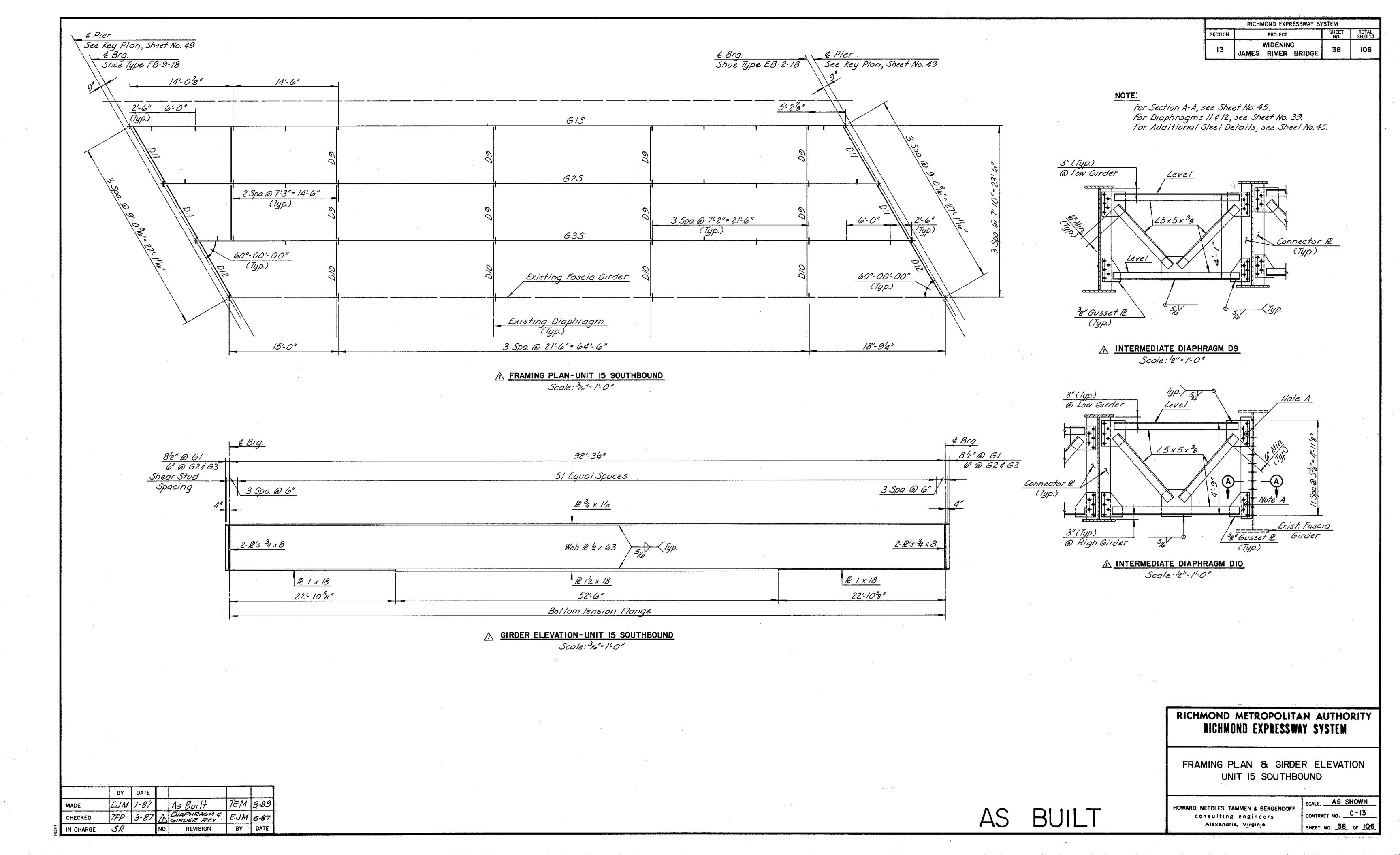
SCALE: AS SHOWN

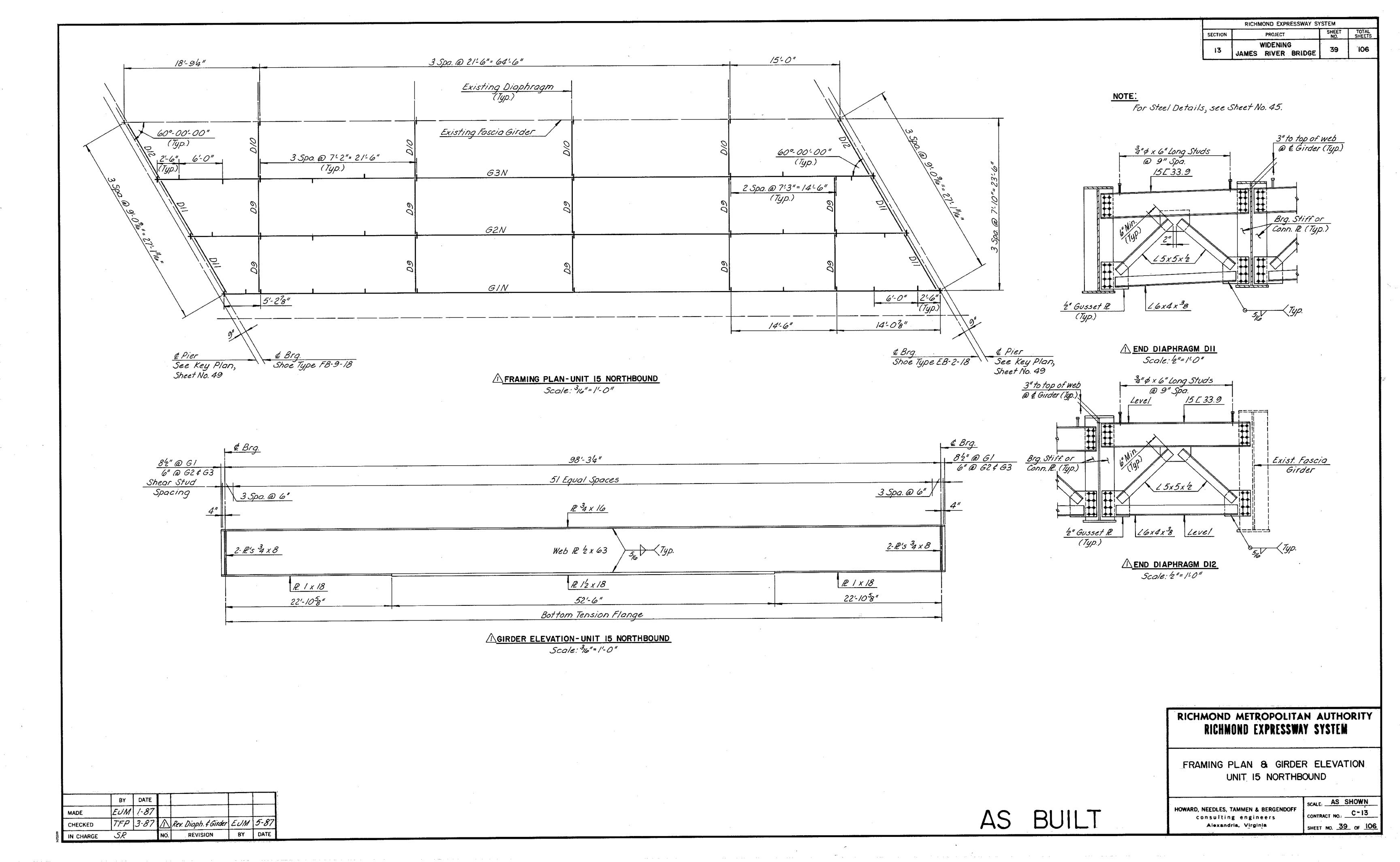
CONTRACT NO.: C-13

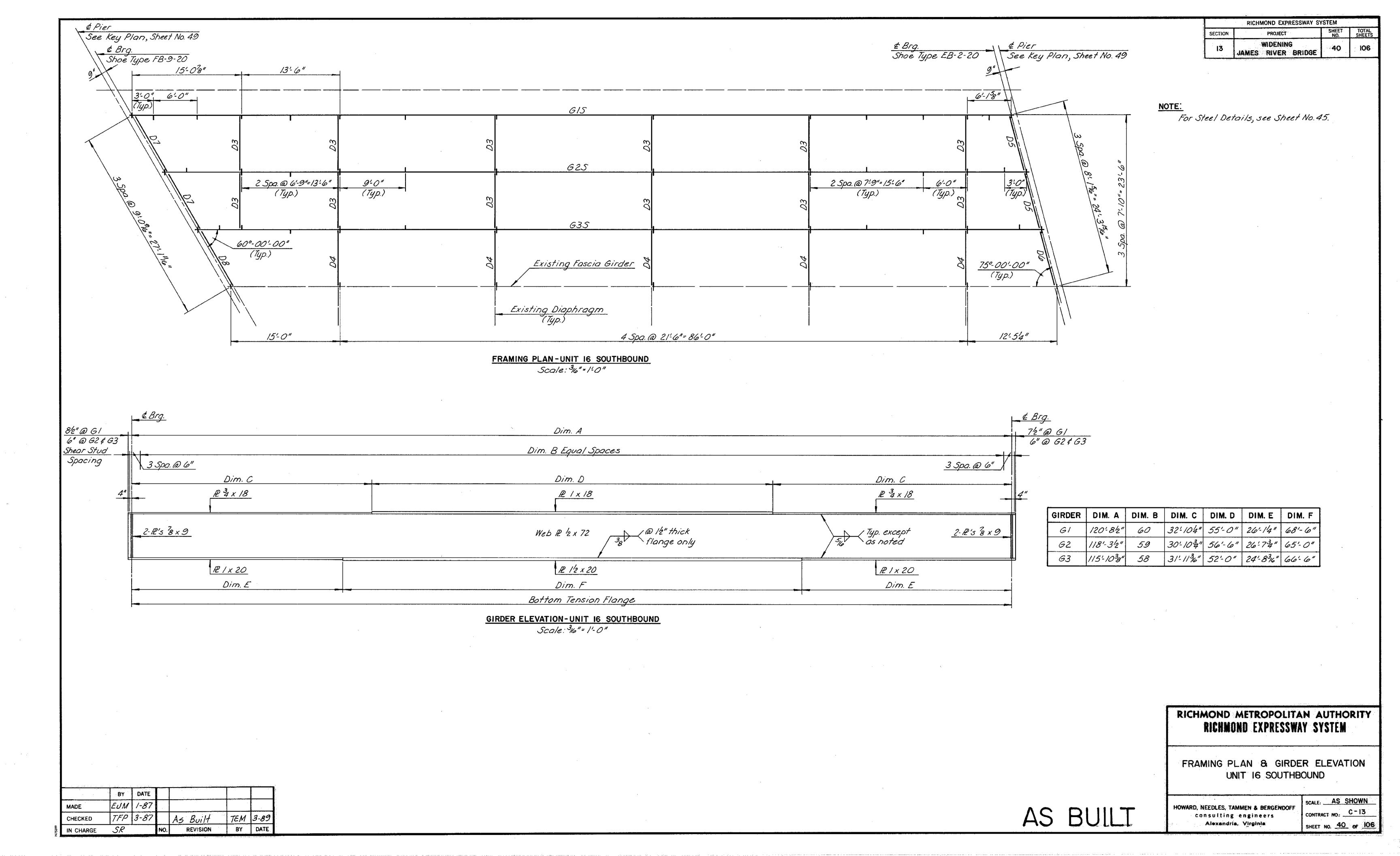
SHEET NO. 36 OF 106

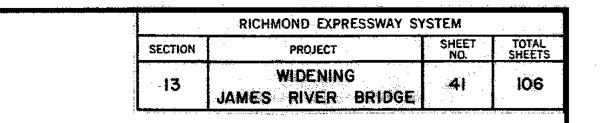
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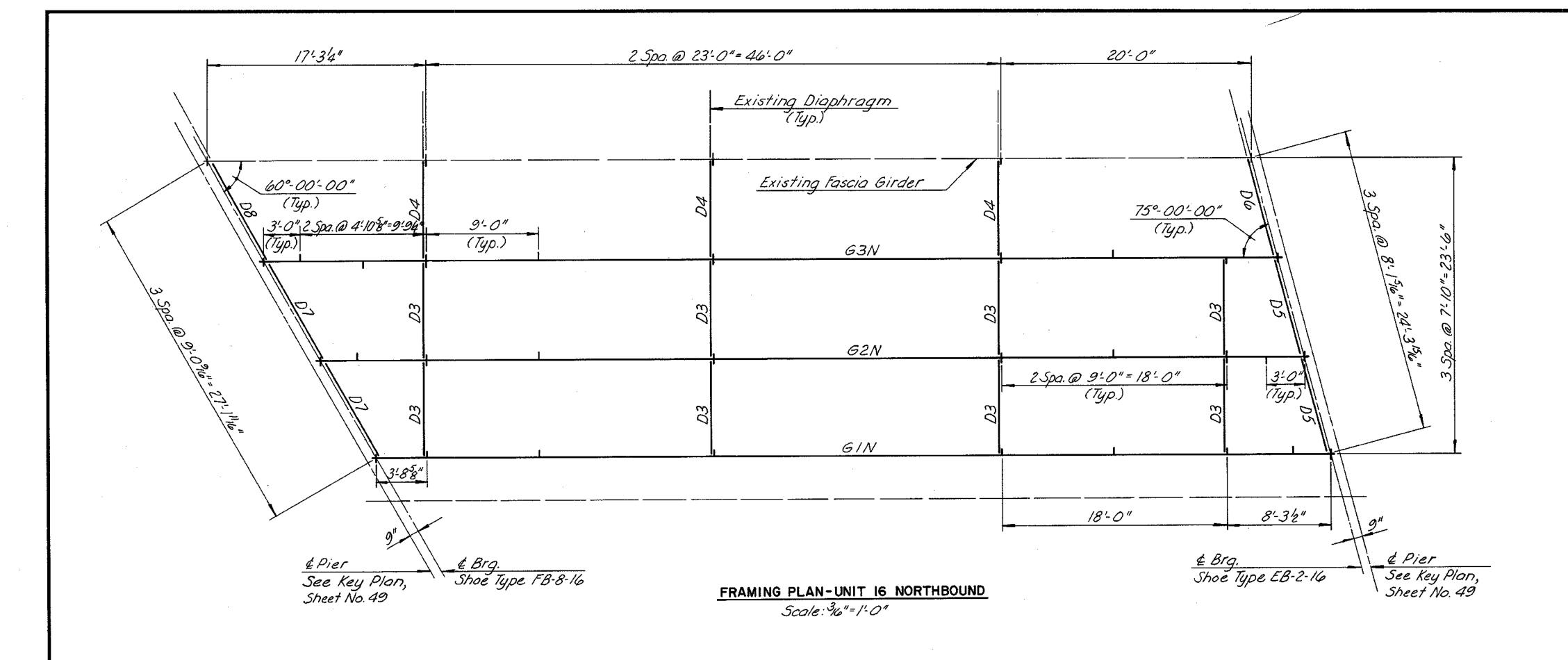


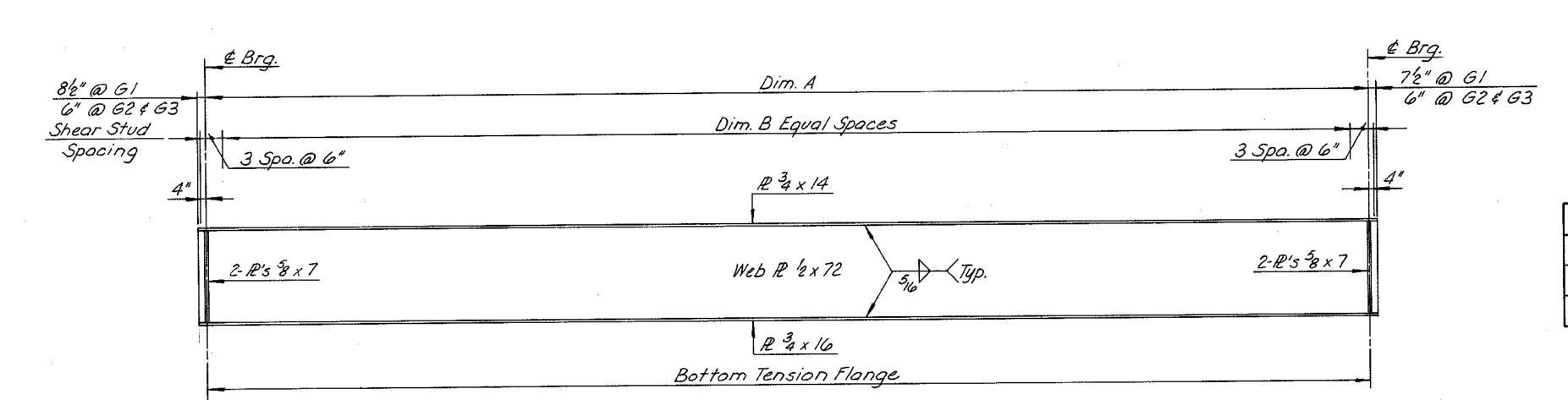




NOTE:

For Steel Details, see Sheet No.45.





GIRDER ELEVATION-UNIT 16 NORTHBOUND

Scale: 316"= 1-0"

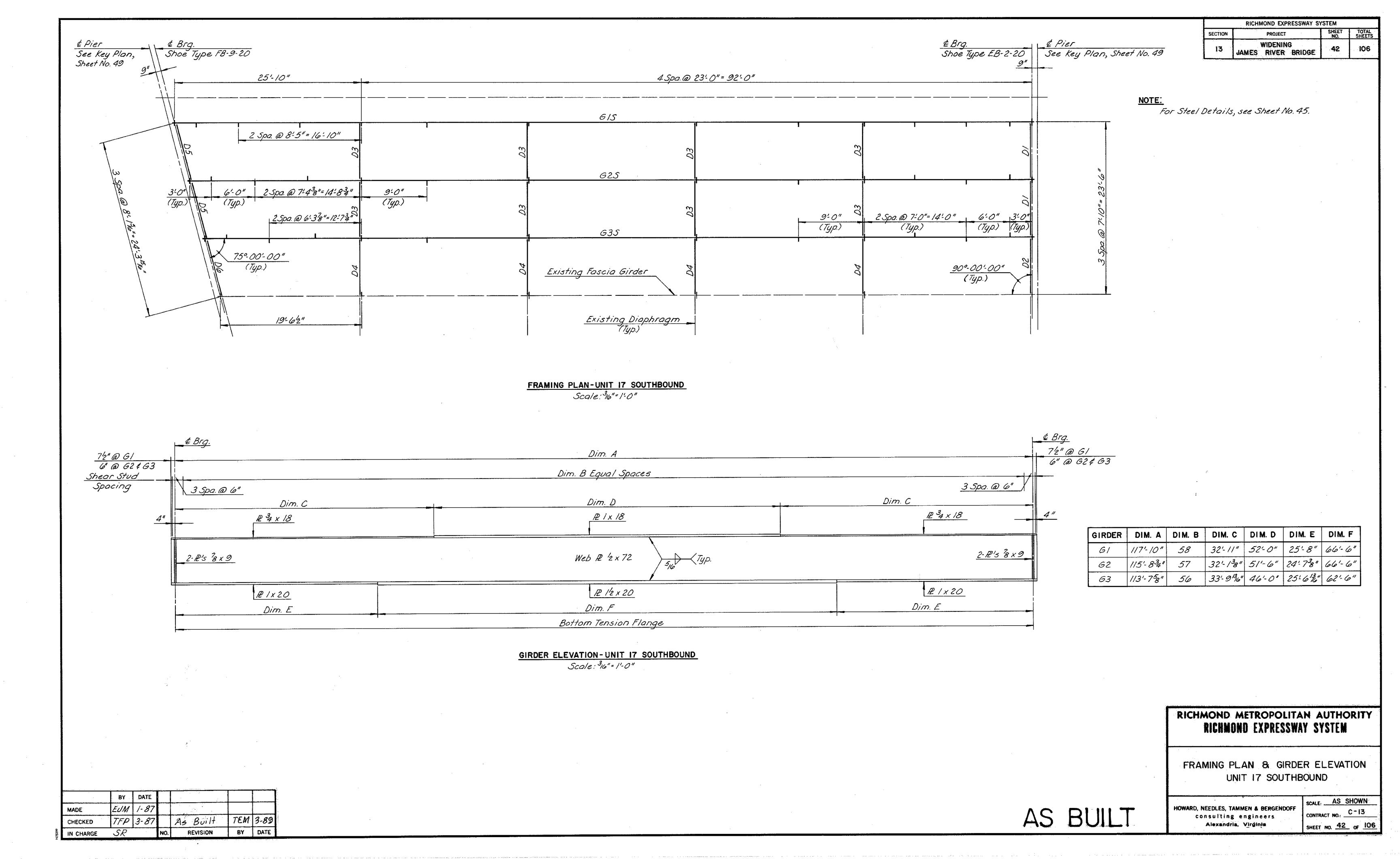
GIRDER	DIM. A	DIM. B
G1	76'-0"	43
G2	78'-5'8"	44
<i>G3</i>	80'-10'4"	46

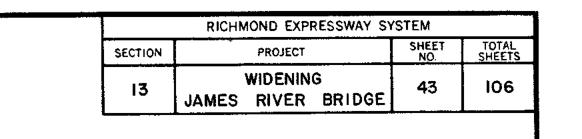
RICHMOND METROPOLITAN AUTHORITY
RICHMOND EXPRESSWAY SYSTEM

FRAMING PLAN & GIRDER ELEVATION
UNIT 16 NORTHBOUND

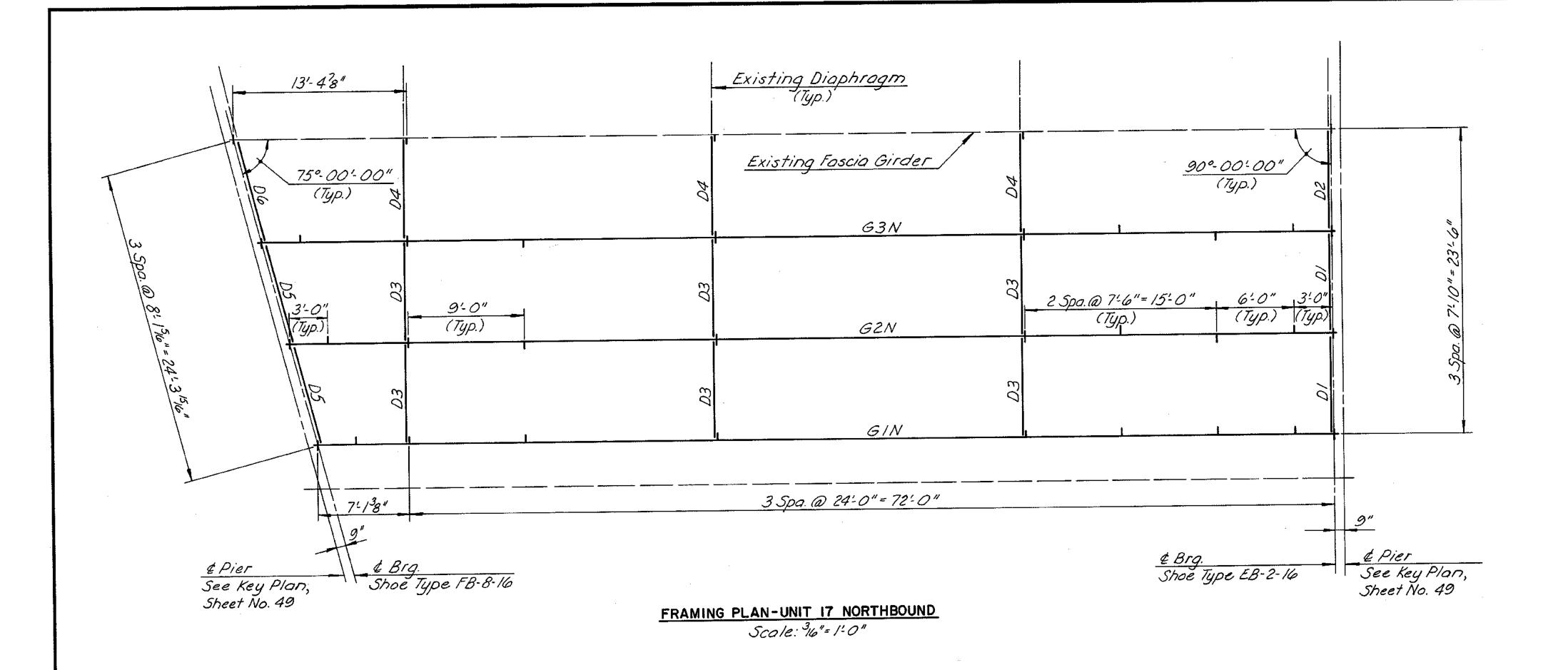
HOWARD, NEEDLES, TAMMEN & BERGENDOFF
consulting engineers
Alexandria, Virginia

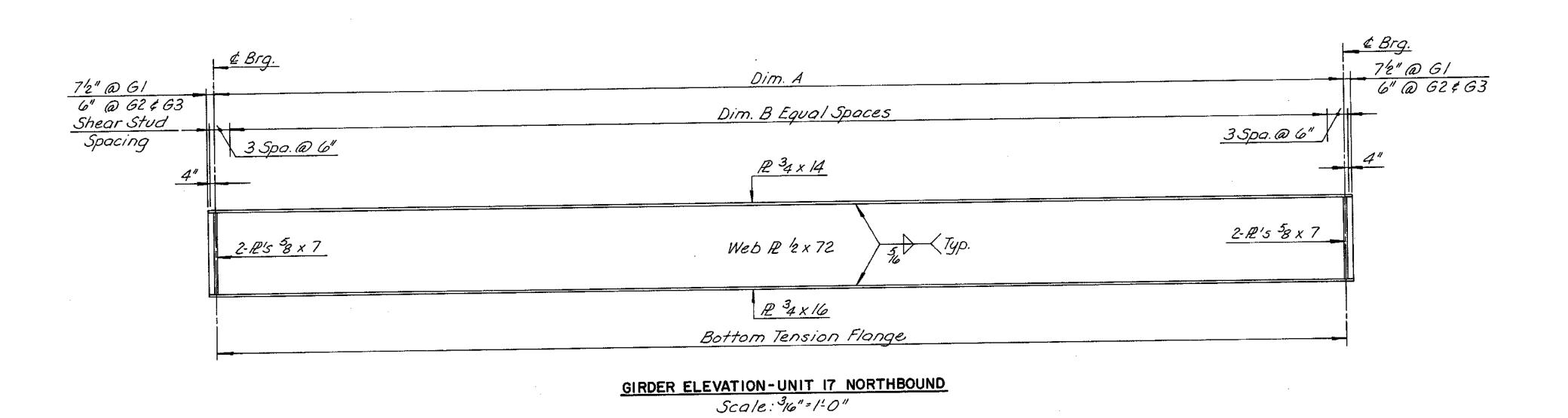
SCALE: AS SHOWN
CONTRACT NO: C-13
SHEET NO. 41 OF 106





NOTE: For Steel Details, see Sheet No. 45.





GIRDER	DIM. A	DIM. B
G/	79-138"	43
<i>G</i> 2	81'-2'2"	44
63	83'-334"	45

ICHMOND ME	<b>IROPOLITAN</b>	<b>AUTHORITY</b>	
RICHMOND	<b>EXPRESSWAY</b>	SYSTEM	

SCALE: AS SHOWN

CONTRACT NO.: C-13

SHEET NO. 43 OF 106

FRAMING PLAN & GIRDER ELEVATION UNIT 17 NORTHBOUND

HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers Alexandria, Virginia

AS	B	

	BY	DATE				
MADE	EUM	1-87				
CHECKED	TFP	3-87		As Built	TEM	3-89
IN CHARGE	SR		NO.	REVISION	BY	DATE

E Brg. North Abutment Front Face of Backwall
Sta. 149 + 44.18

Sta. 149 + 45.68 Shoe Type FB-8-16 £ Pier 17 Sta. 148+64.18 Shoe Type EB-2-16 1-6" G15 625 3 Spa. @ 5'-8" = 17'-0" 3 Spa. @ 5'.5"= 16'-3" (Typ.) (Typ.) 635 90°-00'-00" Existing Fascia Girder (Typ.) Existing Diaphragm

> FRAMING PLAN - UNIT 18 SOUTHBOUND & NORTHBOUND (SOUTHBOUND SHOWN) Scale: 316"= 1'-0"

3 Spa. @ 20'-0" = 60'-0"

& Brg. & Brg. 79'-3" 72" @ G1 6" @ G2 \$ G3 47 Equal Spaces Shear Stud Spacing 3 Spa. @ 6" 3 Spa. @ 6" R3 x 14 2-R's 58x7 2-R'5 58 x 7 Web R 2 x 72 R 34 x 16 Bottom Tension Flange

GIRDER ELEVATION-UNIT 18 SOUTHBOUND & NORTHBOUND

Scale: 3/6" = 1'-0"

BY DATE EUM 1-87 MADE TFP 3-87 AS Built TEM 3-89

D. REVISION BY DATE SR IN CHARGE

19:3"

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

RICHMOND EXPRESSWAY SYSTEM

106

WIDENING

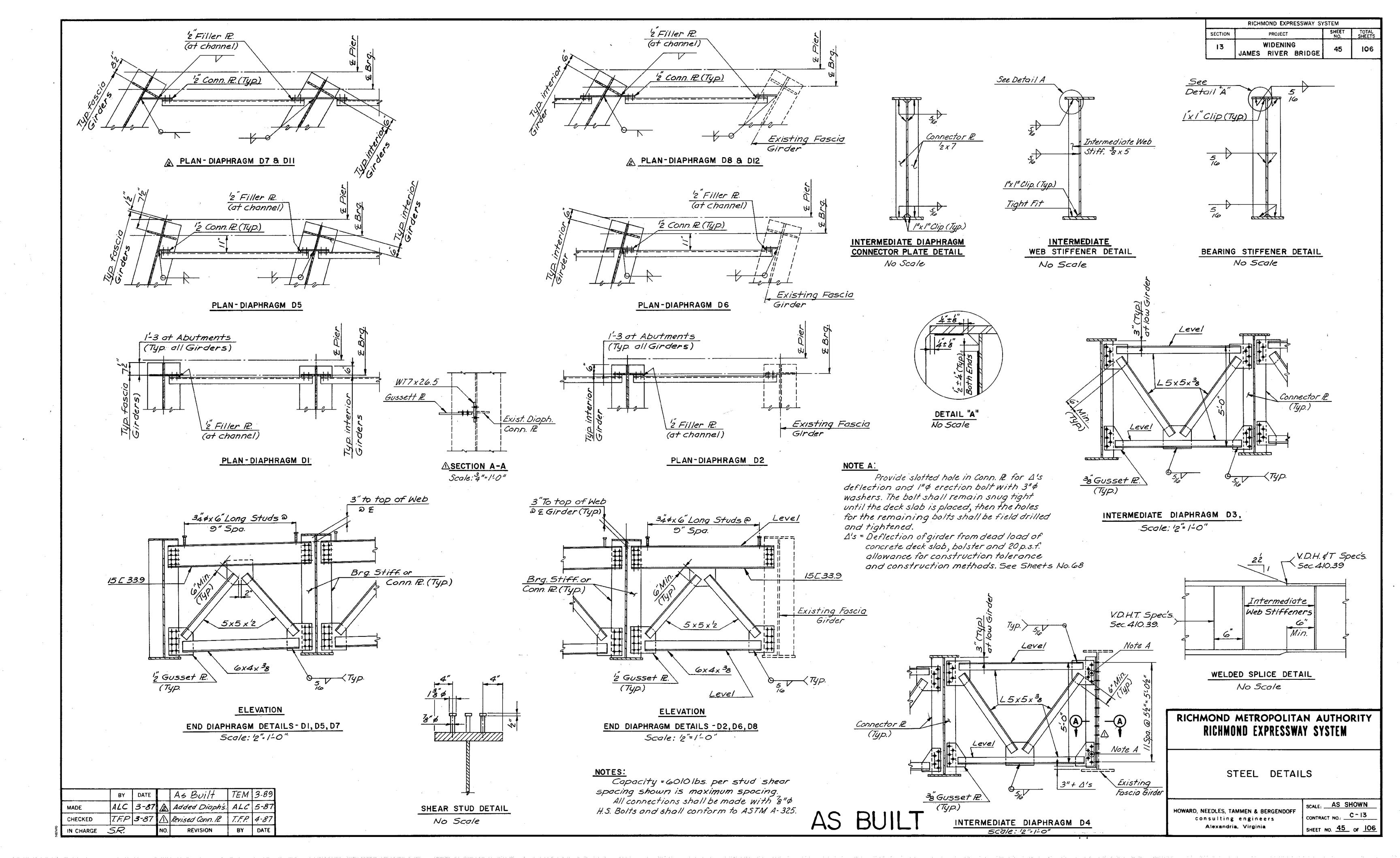
JAMES RIVER BRIDGE

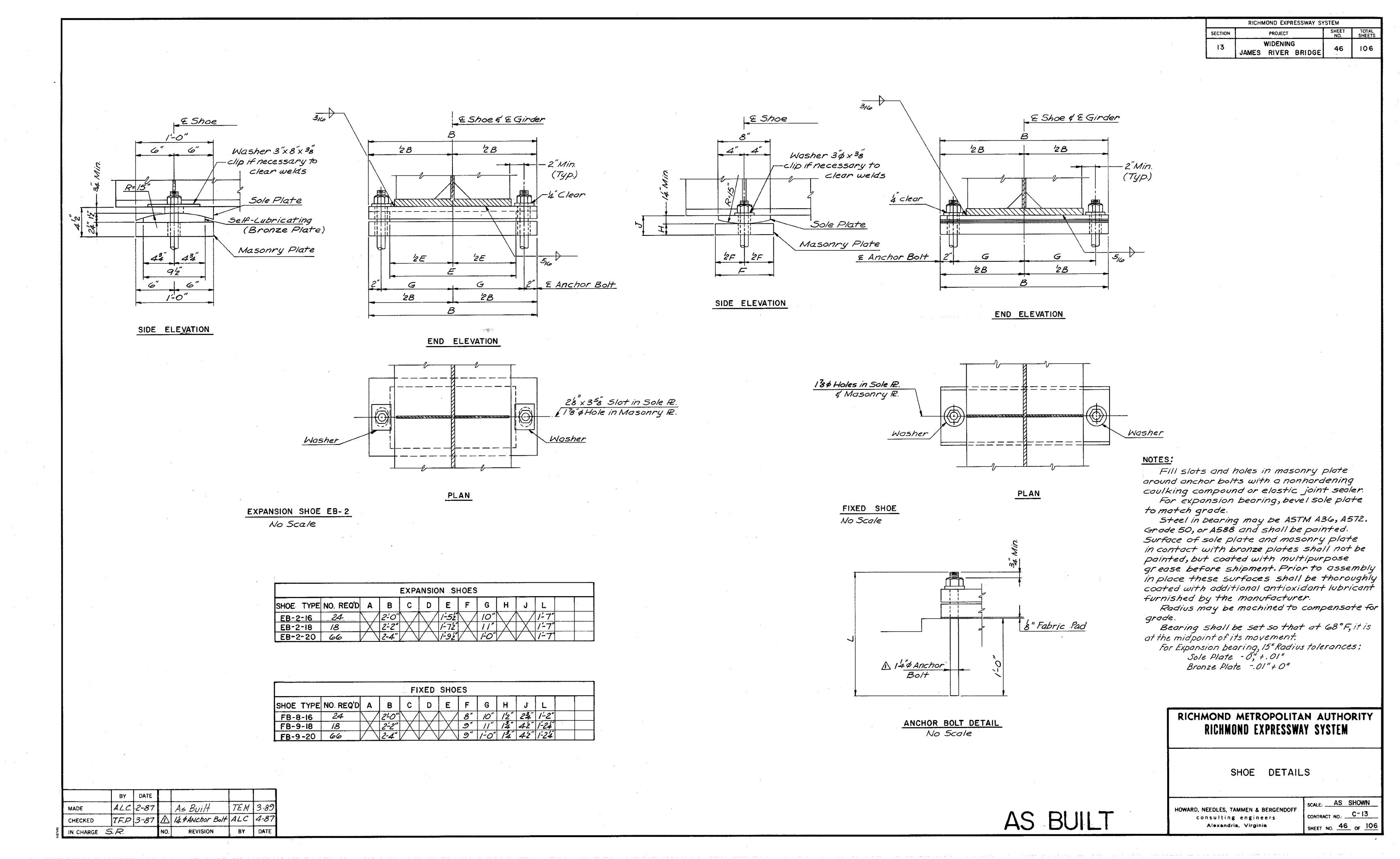
SECTION

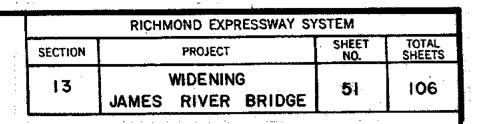
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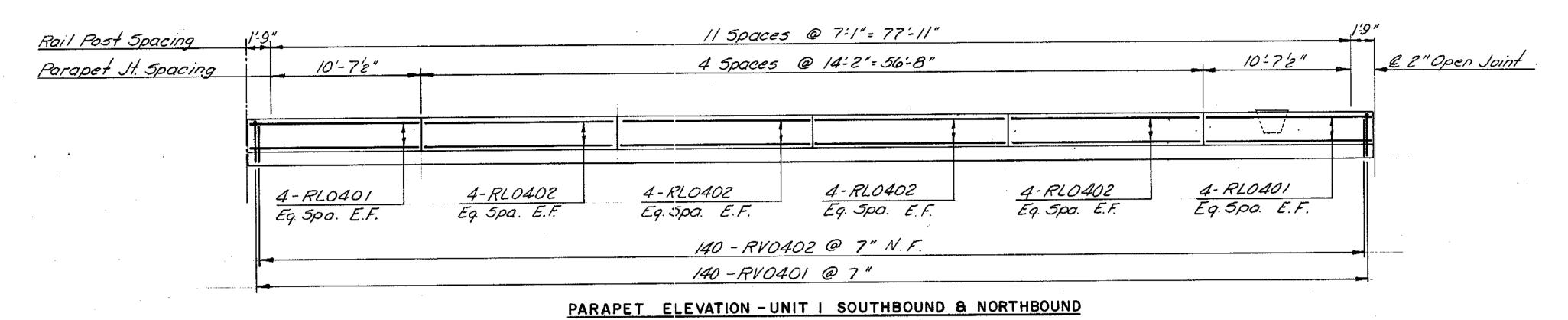
FRAMING PLAN & GIRDER ELEVATION UNIT 18 SOUTHBOUND & NORTHBOUND

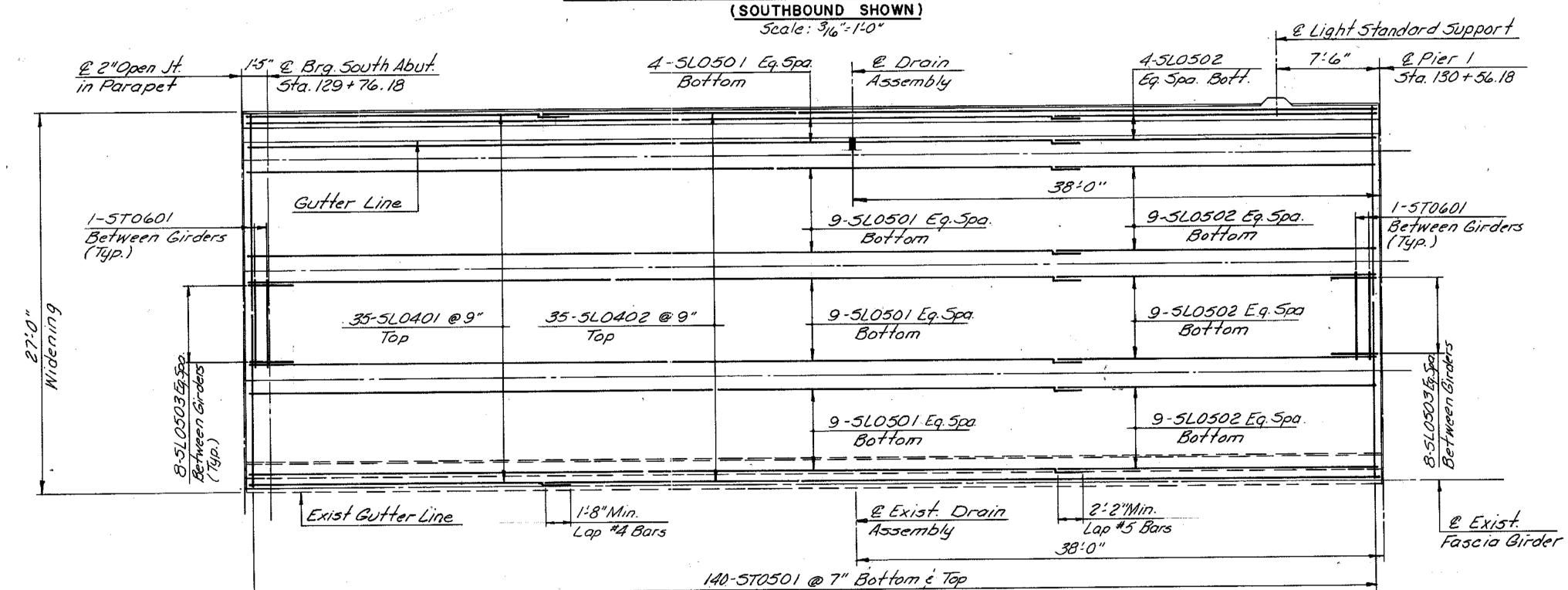
SCALE: AS SHOWN HOWARD, NEEDLES, TAMMEN & BERGENDOFF
consulting engineers
Alexandria. Virginia CONTRACT NO.: C-13 SHEET NO. 44 OF 106





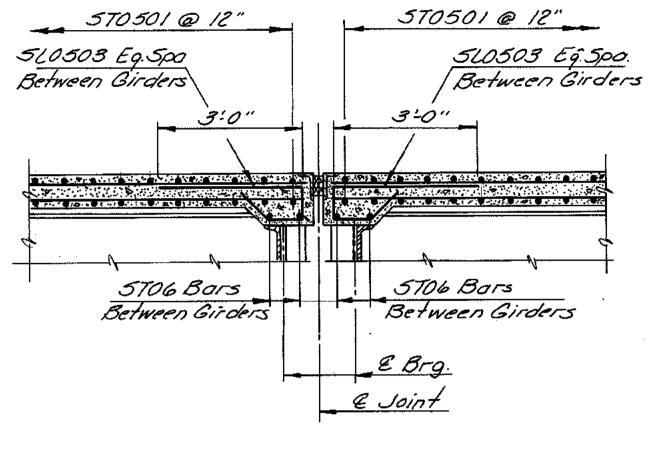






DECK PLAN - UNIT | SOUTHBOUND & NORTHBOUND (SOUTHBOUND SHOWN)

5cale: 3/6"=1'-0"



TYPICAL SECTION AT JOINT Scale: 12"=1'-0"

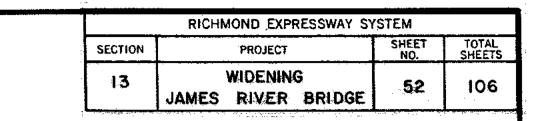
> RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

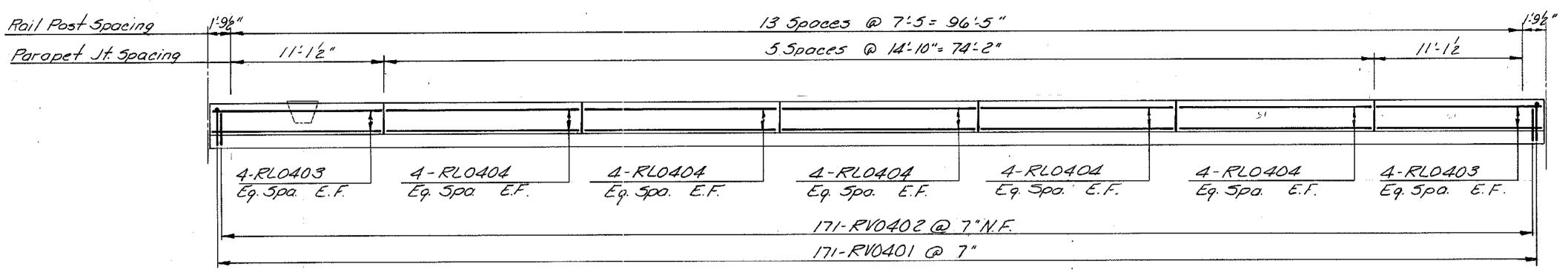
DECK PLAN UNIT I SOUTHBOUND & NORTHBOUND

consulting engineers

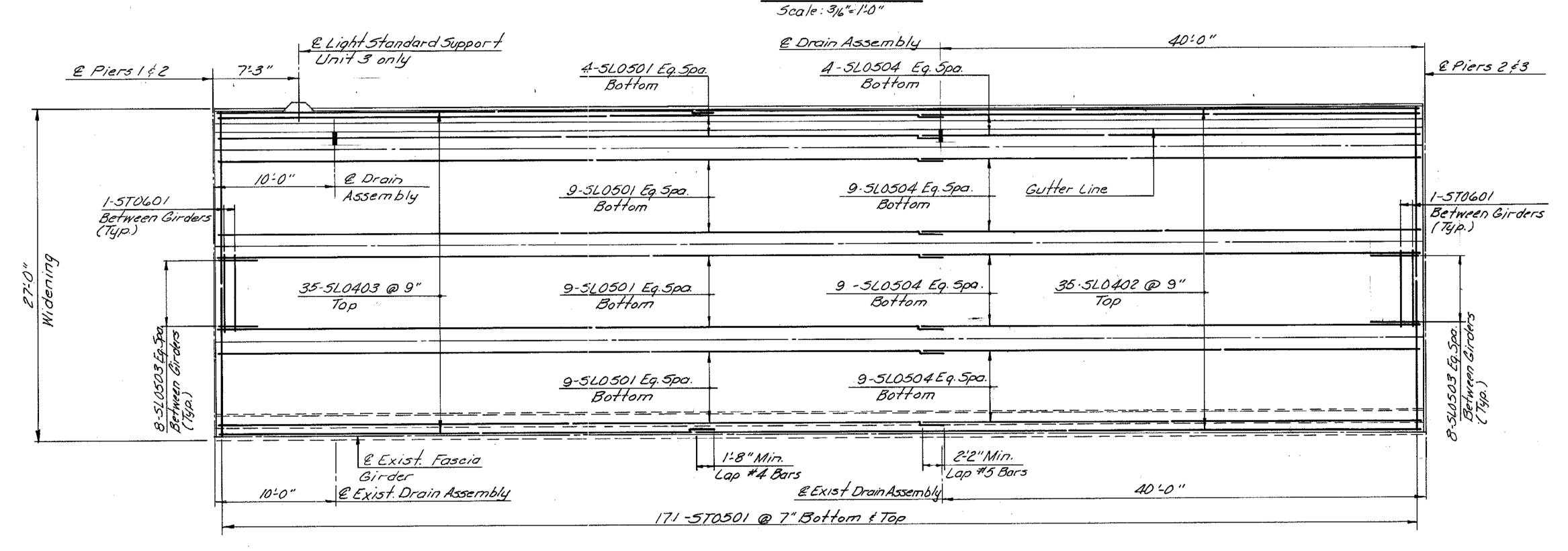
SCALE: AS SHOWN HOWARD, NEEDLES, TAMMEN & BERGENDOFF CONTRACT NO.: C-13 SHEET NO. 51 OF 106 Alexandria, Virginia

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	MADE	TAL	3-87					,
	CHECKED	T.F.R.	3-87	:	As Built	TEM	3-89	
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#### PARAPET ELEVATION - UNITS 2 & 3 SOUTHBOUND & NORTHBOUND (SOUTHBOUND SHOWN)



DECK PLAN - UNITS 2 & 3 SOUTHBOUND & NORTHBOUND (SOUTHBOUND SHOWN)

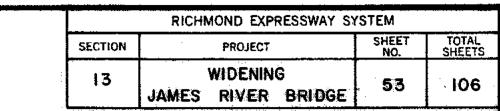
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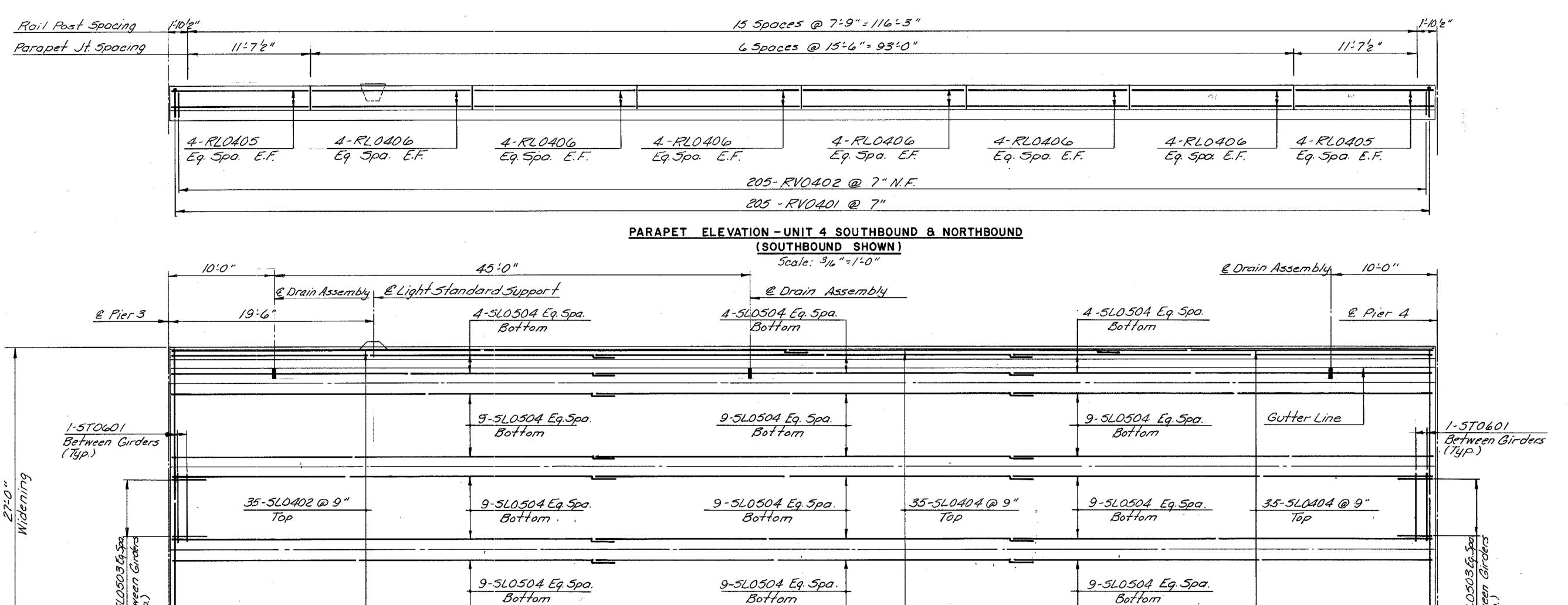
> RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

DECK PLAN UNITS 2 & 3 SOUTHBOUND & NORTHBOUND

WARN NECHIES TAMMEN & DEDGENDOES !	SCALE: AS SHOWN		
	CONTRACT NO.: C-13		
Alexandria, Virginia	SHEET NO. 52 OF 106		

	BY	DATE				
MADE	TAL	3.87				
CHECKED	7.F.P.	3-87		As Built	TEM	3-89
IN CHARGE	S.R.		NO.	REVISION	BY	DATE





DECK PLAN - UNIT 4 SOUTHBOUND & NORTHBOUND

(SOUTHBOUND SHOWN)

5cale: 3/6" = 1-0"

205-570501@7" Bottom & Top

1-8" Min.

2-2"Min. Lap #5 Bars

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

DECK PLAN
UNIT 4 SOUTHBOUND & NORTHBOUND

HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers
Alexandria, Virginia

SCALE: AS SHOWN

CONTRACT NO.: C-13

SHEET NO. 53 OF 106

	ВУ	DATE				
MADE	TAL	3-87				
CHECKED	TEP	3-87		As Built	TEM	3-89
IN CHARGE	5.R.	*	NO.	REVISION	BY	DATE

EExist. Drain Assembly

10-0"

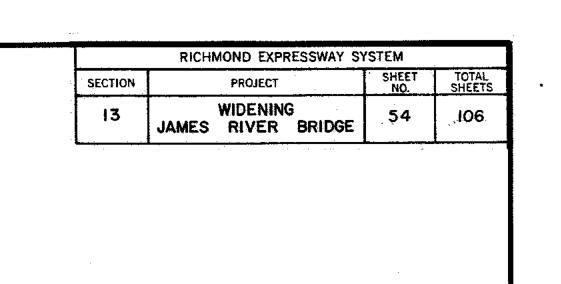
& Exist Fascia Girder

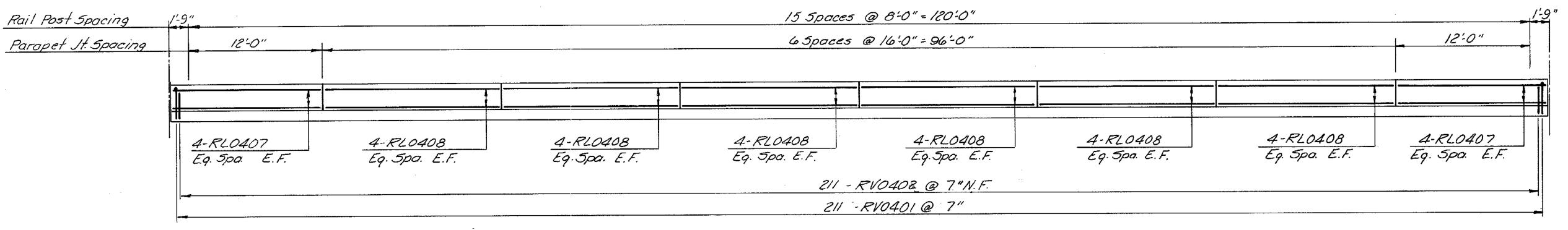
45-0"

10'-0"

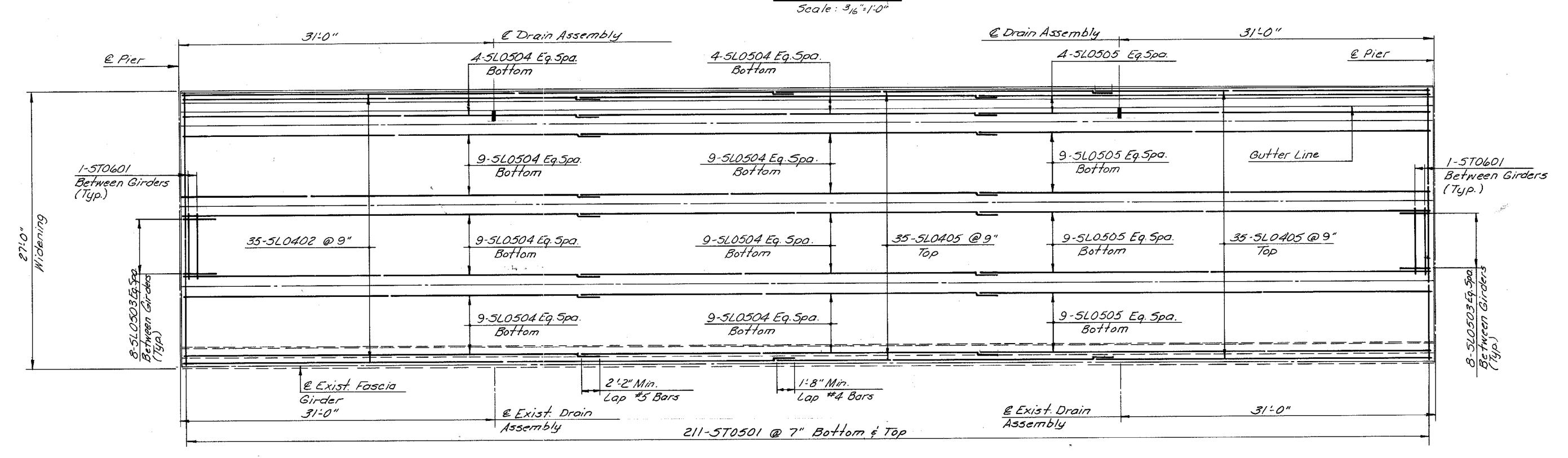
& Exist. Drain

Assembly

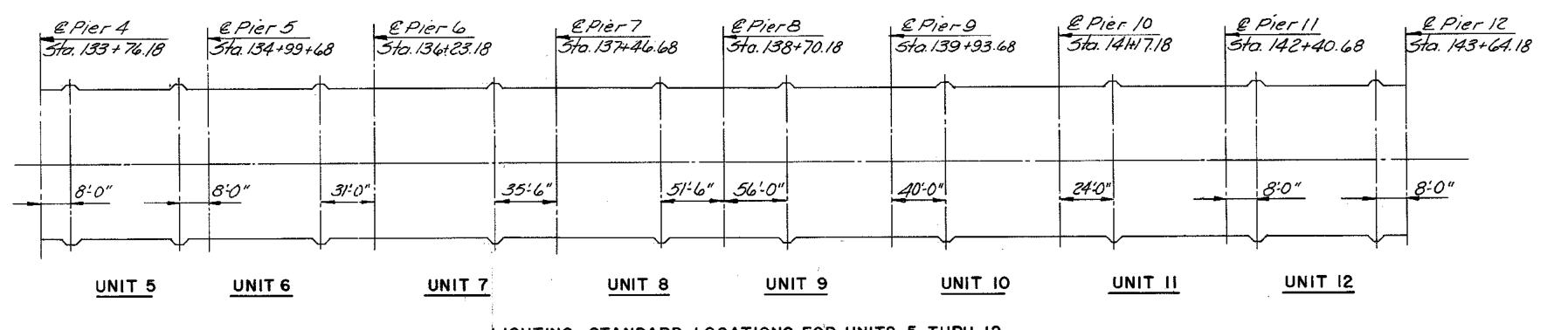




### PARAPET ELEVATION - UNITS 5 THRU 12 SOUTHBOUND & NORTHBOUND (SOUTHBOUND SHOWN)



# DECK PLAN-UNITS 5 THRU 12 SOUTBOUND & NORTHBOUND (SOUTHBOUND SHOWN) 5cale: 3/6"=/-0"



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LIGHTING STANDARD LOCATIONS FOR UNITS 5 THRU 12
No Scale

AS BUILT

## RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

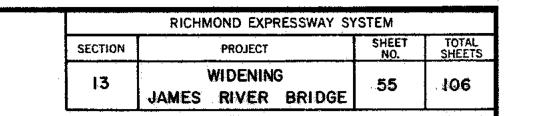
DECK PLAN
UNITS 5 THRU I2 SOUTHBOUND & NORTHBOUND

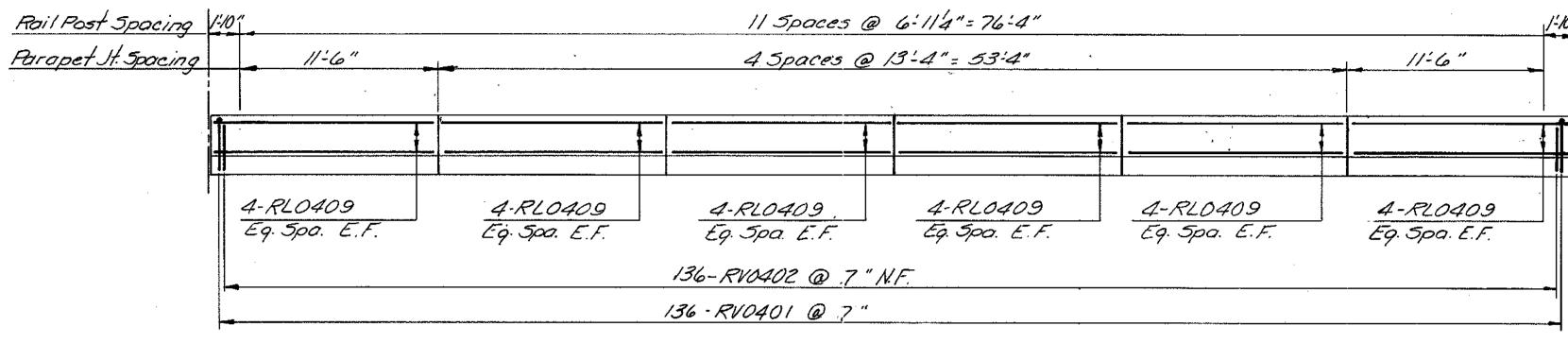
HOWARD, NEEDLES, TAMMEN & BERGENDOF	F
consulting engineers	
Alexandria, V <u>i</u> rginia	,

CONTRACT NO.: C-13

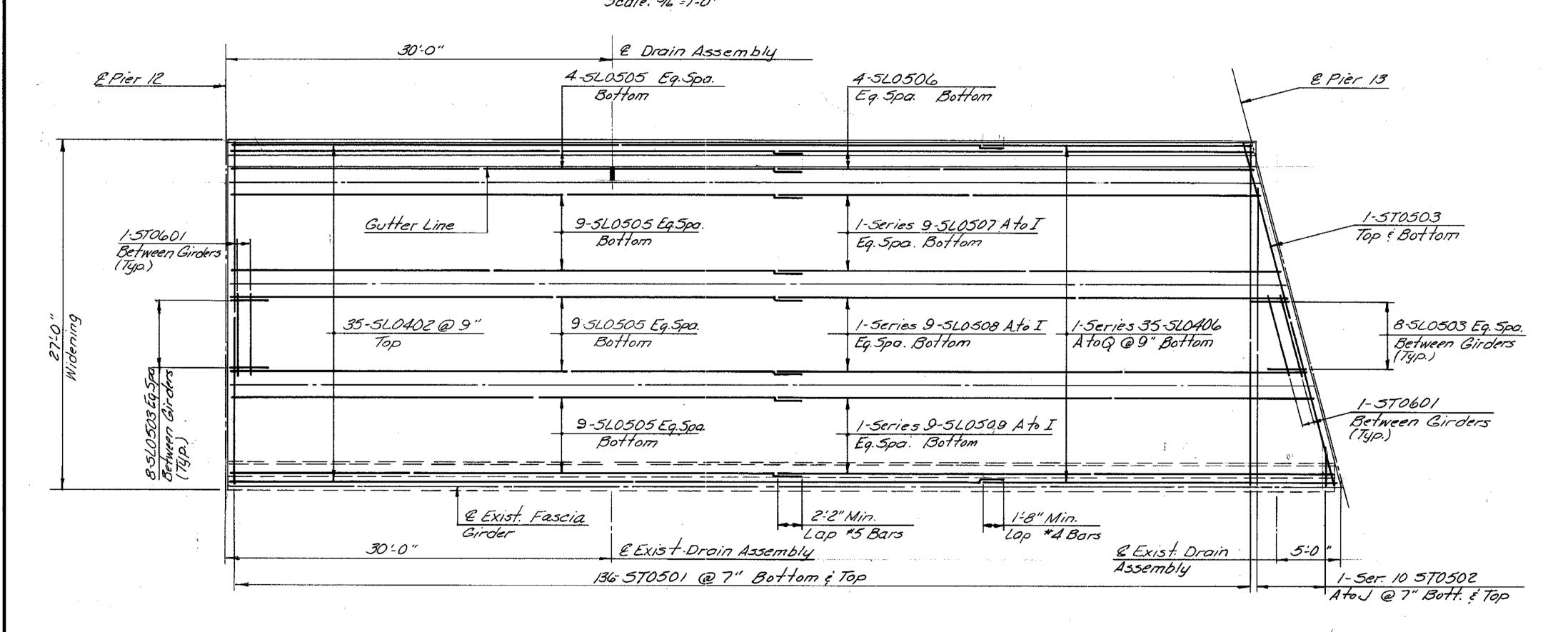
SHEET NO. 54 OF 106

SCALE: AS SHOWN





### PARAPET ELEVATION - UNIT 13 SOUTHBOUND 5cale: 36"=1-0"



DECK PLAN - UNIT 13 SOUTHBOUND

5cole: 316"= 1-0"

RICHMOND	METI	ROPOLITAN	<b>AUTHORITY</b>
RICHMO	ND E	XPRESSWAY	SYSTEM

DECK PLAN - UNIT 13 SOUTHBOUND

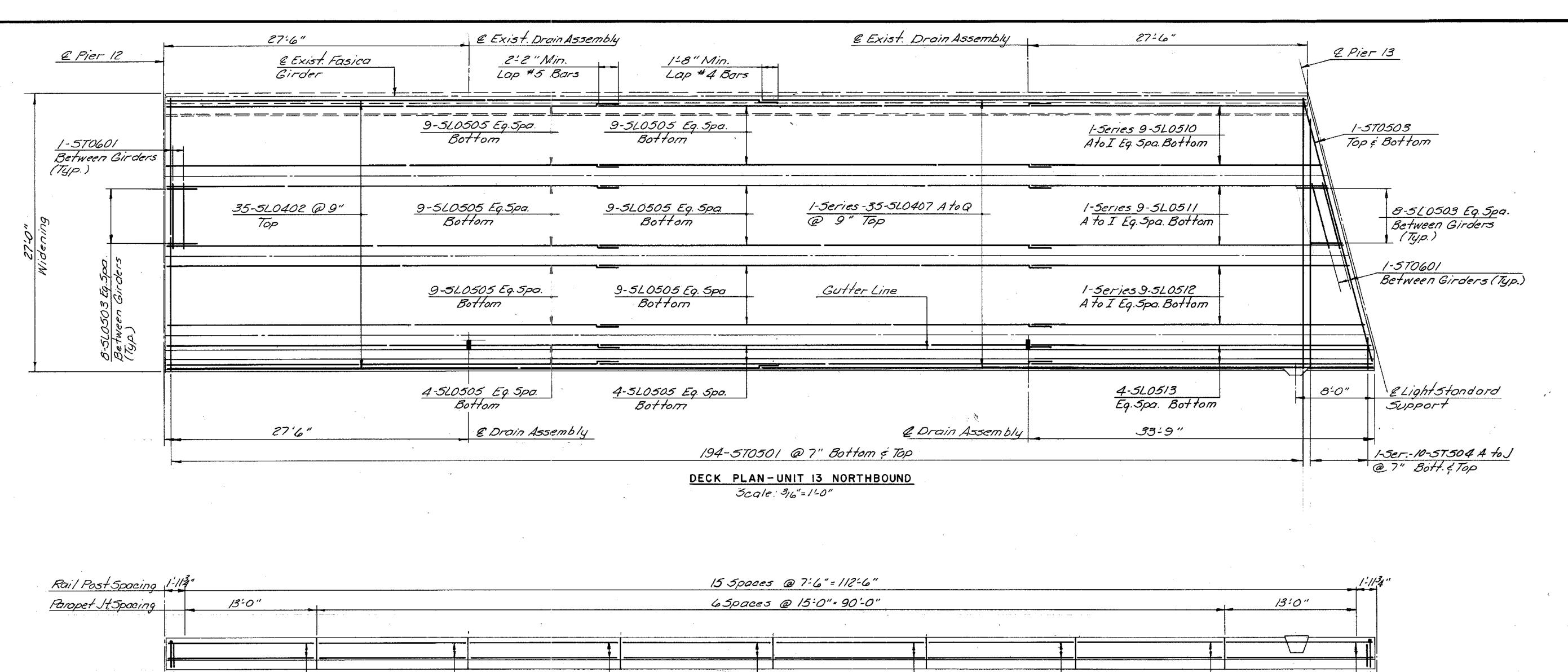
HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers
Alexandria, Virginia

SCALE: AS SHOWN

CONTRACT NO.: C = 13

SHEET NO. 55 OF 106

					<u></u>	
	BY	DATE				
MADE	TAL	3-87				
CHECKED	TFP	3-87		As Built	TEM	3-89
IN CHARGE	SR		NO.	REVISION	BY	DATE



4-RL0410 Eq. 5pa. E.F. 4-RL0410 Eq. 5po. E.F. <u>4-RLO410</u> Eq. 5po. E.F. 4-RL0410 4-RL0410 Eq. Spa. E.F. Eq. Spa. E.F. 198 - RV0402 @ 7" F.F. 198-RV0401 @ 7 "

PARAPET ELEVATION - UNIT 13 NORTHBOUND Scale: 3/6"=1-0"

> RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

> > SCALE: AS SHOWN

CONTRACT NO.: C-13

SHEET NO. 56 OF 106

RICHMOND EXPRESSWAY SYSTEM

56

10.6

PROJECT

WIDENING

JAMES RIVER BRIDGE

SECTION

DECK PLAN - UNIT 13 NORTHBOUND

HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers

Alexandria, V<u>i</u>rginia

AS BUILT

	·	BA	DATE				
	MADE	TAL	3-87				
	CHECKED	TFP	3.87		As Built	TEM	3.89
3	IN CHARGE	D.R.		NO.	REVISION	BY	DATE

4-RL0410

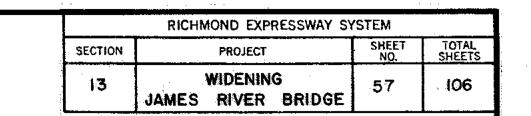
Eq. Spa. E.F.

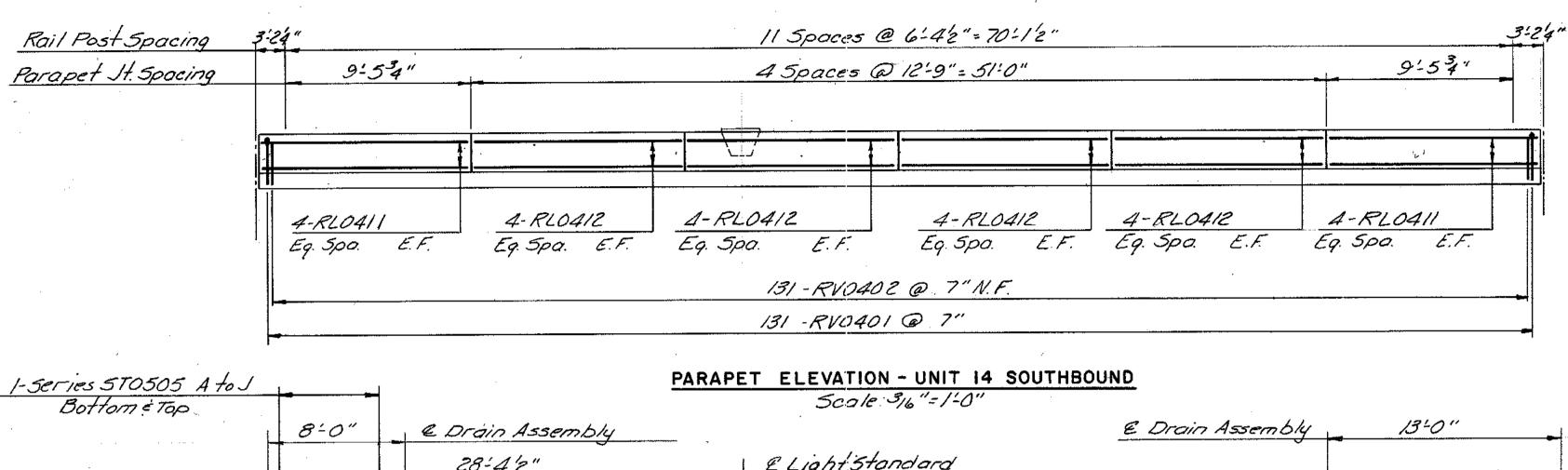
4-RL0410

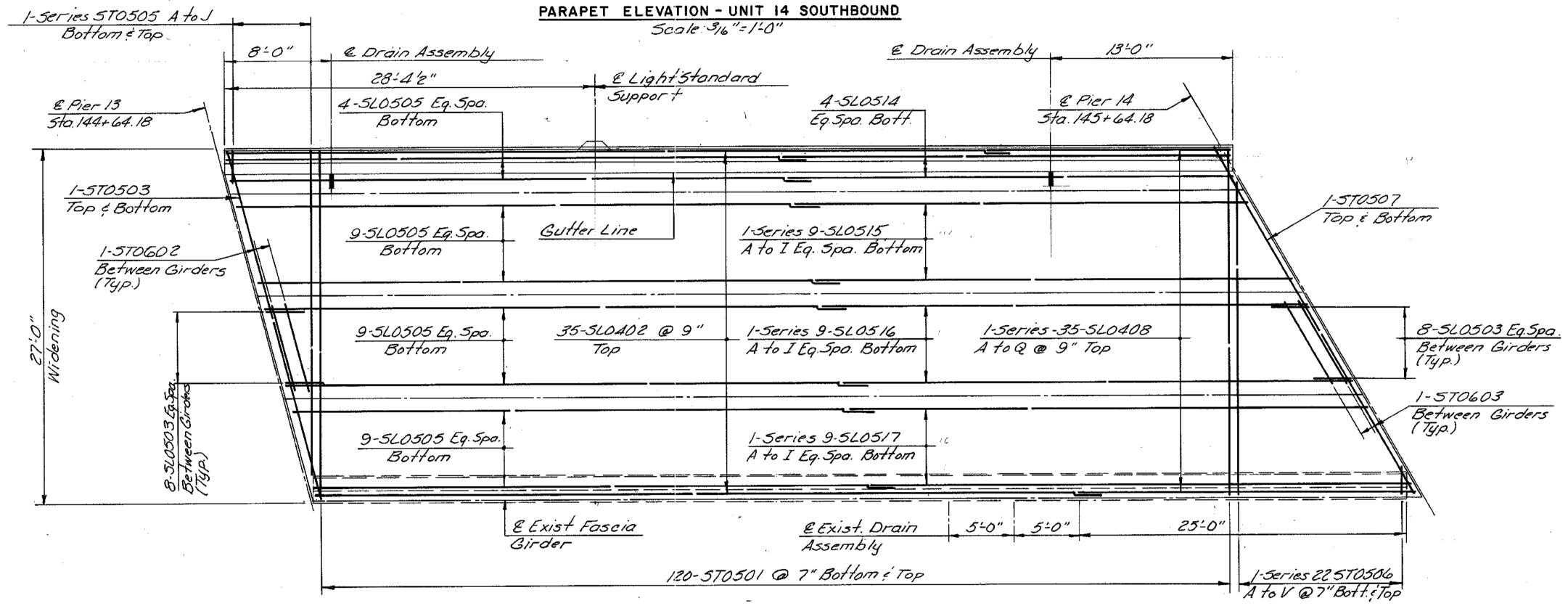
Eq. Spo. E.F.

4-RL0410

Eq. Spa. E.F.







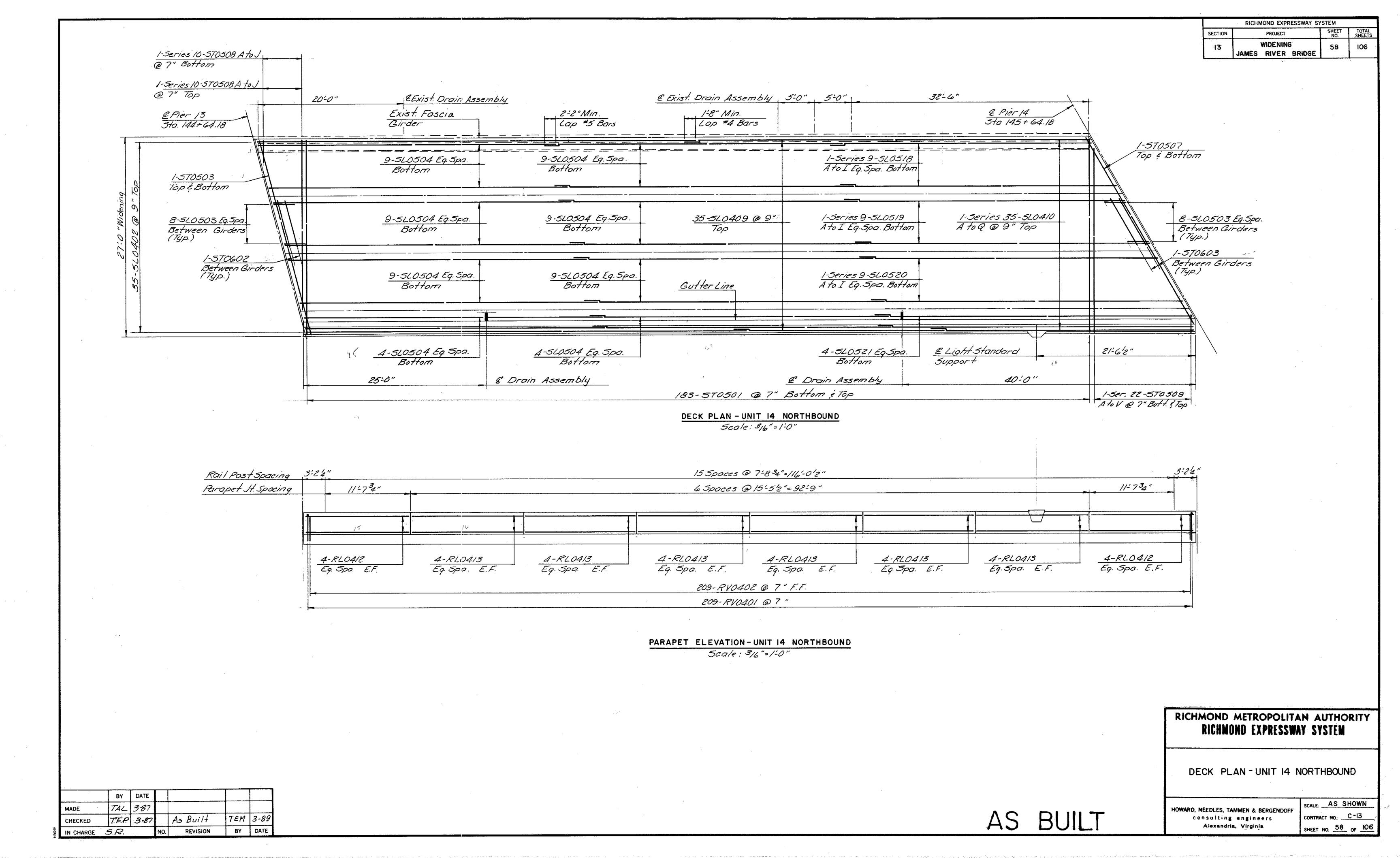
DECK PLAN - UNIT 14 SOUTHBOUND Scale: 3/6"=1'-0"

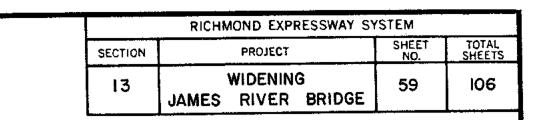
RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

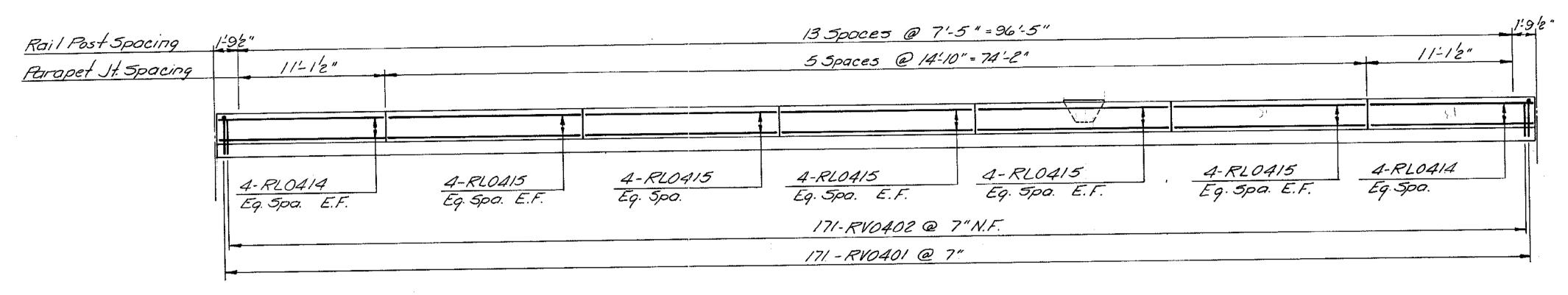
57 of 106

DECK PLAN - UNIT 14 SOUTHBOUND

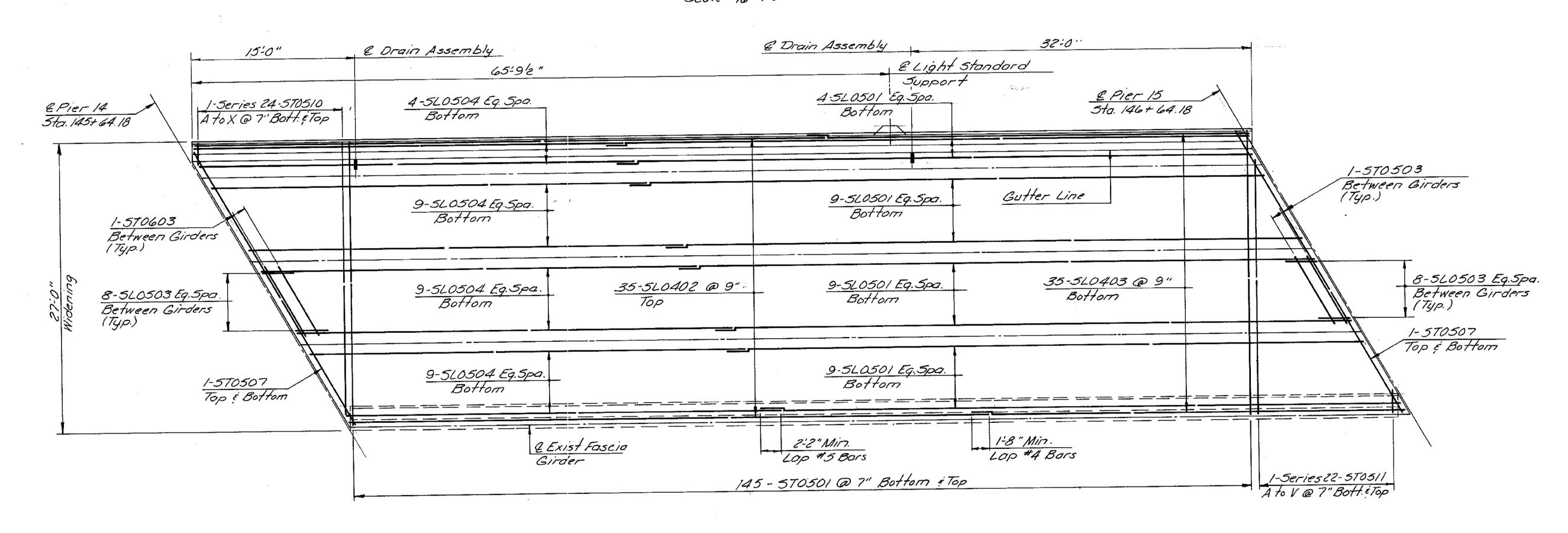
	SCALE: AS SHOWN
MINNAUL NEEDLES TAMMEN / DEDCENDOES .	CONTRACT NO. C-13
Alexandria, Virginia	SHEET NO. 57 OF 10







### PARAPET ELEVATION - UNIT 15 SOUTHBOUND Scale: 316"= 1-0"



DECK PLAN - UNIT 15 SOUTHBOUND

Scale: 3/6"=1-0"

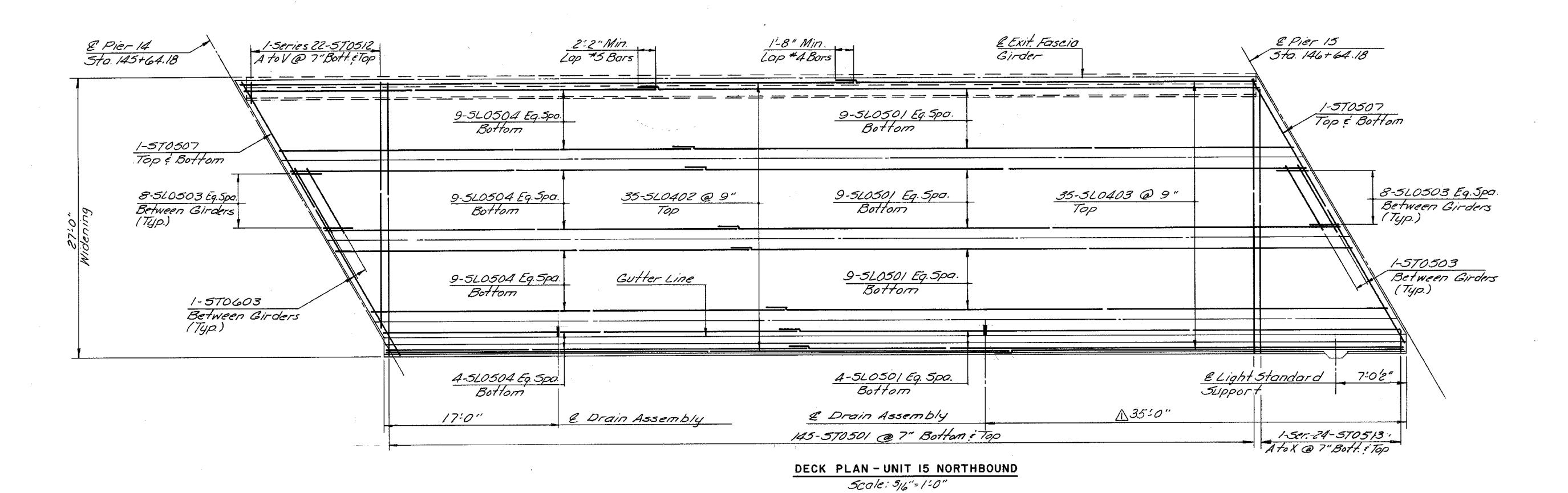
RICHMOND MET	<b>TROPOLITAN</b>	<b>AUTHORITY</b>
RICHMOND	<b>EXPRESSWAY</b>	SYSTEM

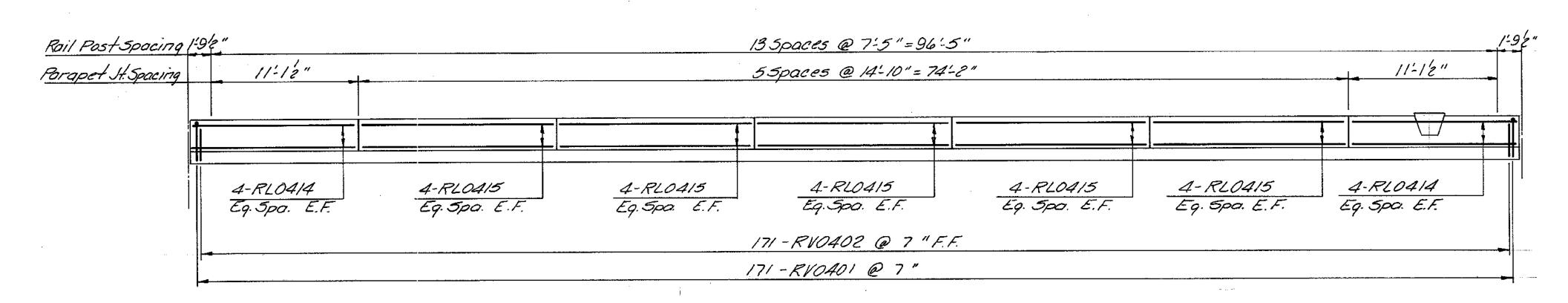
DECK PLAN - UNIT 15 SOUTHBOUND

	SCALE: AS SHOWN
CONTRACTOR SECTION FOR TAXABLE MEDICAL DEPOSITIONES.	CONTRACT NO.: C-13
	SHEET NO. 59 OF 106

- 1						·	
1		ВУ	DATE				
	MADE	TAL	3-87				
	CHECKED	T.F.P.	3-87		As Built	TEM	3-89
35	IN CHARGE	. R2.		NO.	REVISION	BY	DATE

RICHMOND EXPRESSWAY SYSTEM SHEET TOTAL NO. SHEETS SECTION PROJECT WIDENING 60 JAMES RIVER BRIDGE





PARAPET ELEVATION - UNIT 15 NORTHBOUND Scole: 316"=1'-0"

BY DATE As Built TEM 3-89 7.F.P 3-87 A Rel. Drain Assbly. EUM 7-87
5.R. NO. REVISION BY DATE IN CHARGE S.R.

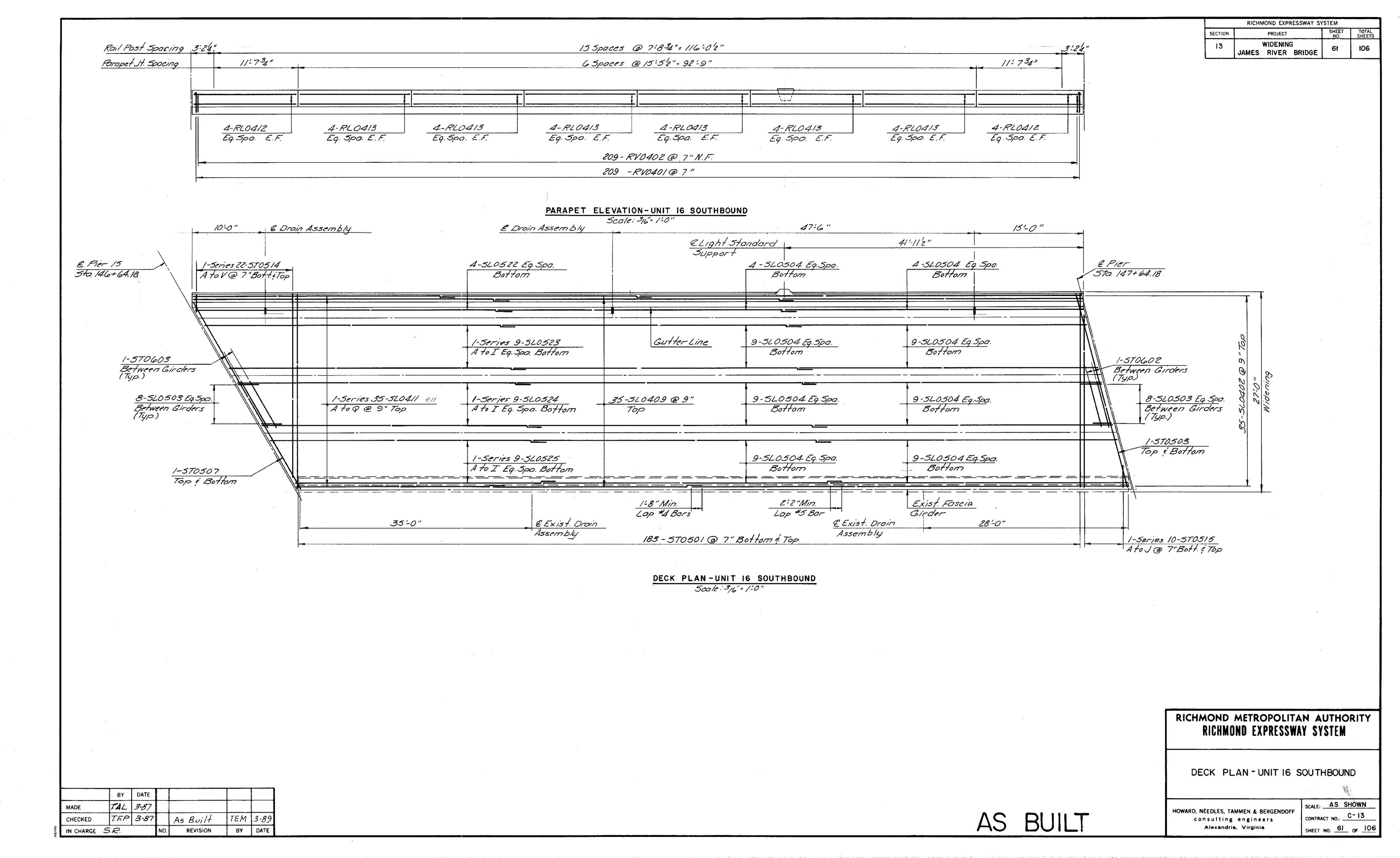
RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

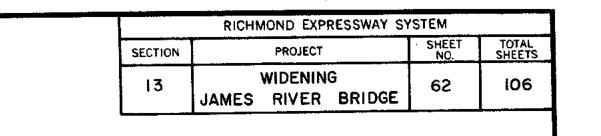
DECK PLAN - UNIT 15 NORTHBOUND

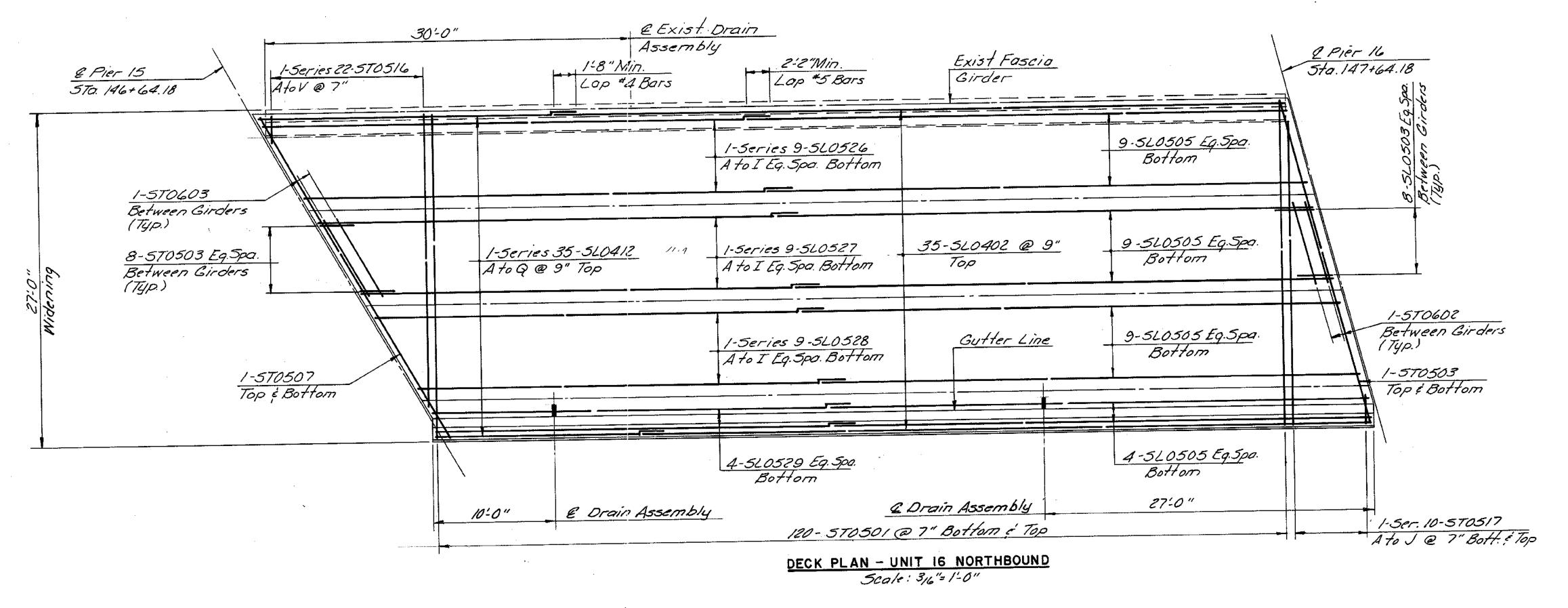
HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers Alexandria, Virginia

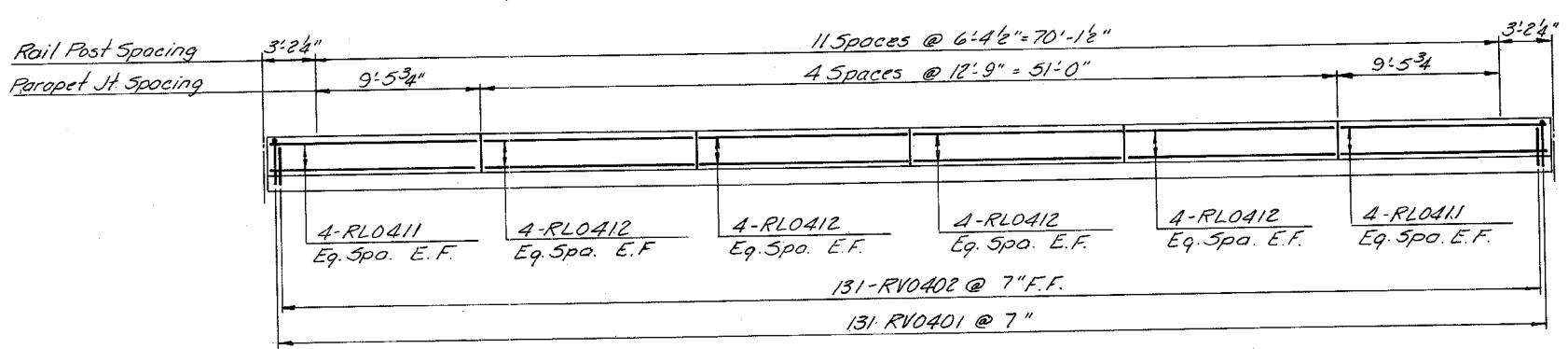
AS BUILT

SCALE: AS SHOWN CONTRACT NO.: C - 13 SHEET NO. 60 OF 106









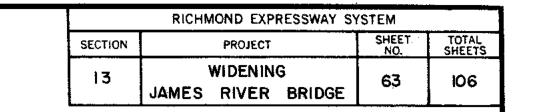
PARAPET ELEVATION - UNIT 16 NORTHBOUND Scale: 3/6" = 1-0"

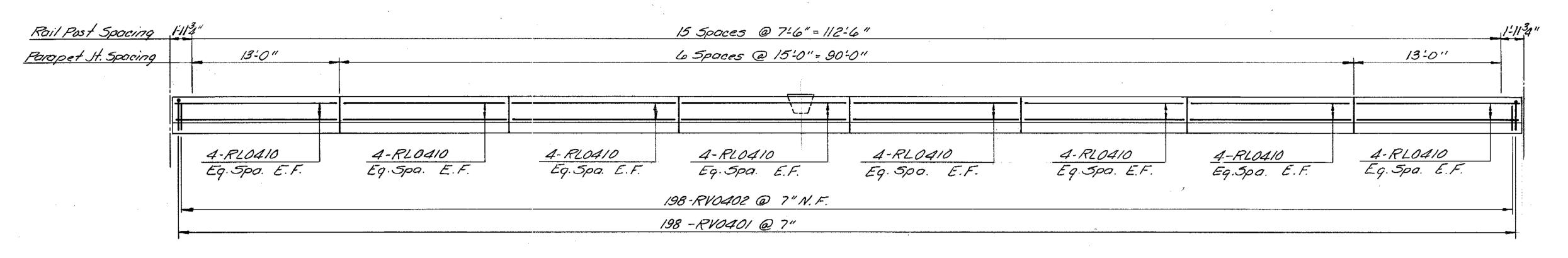
RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

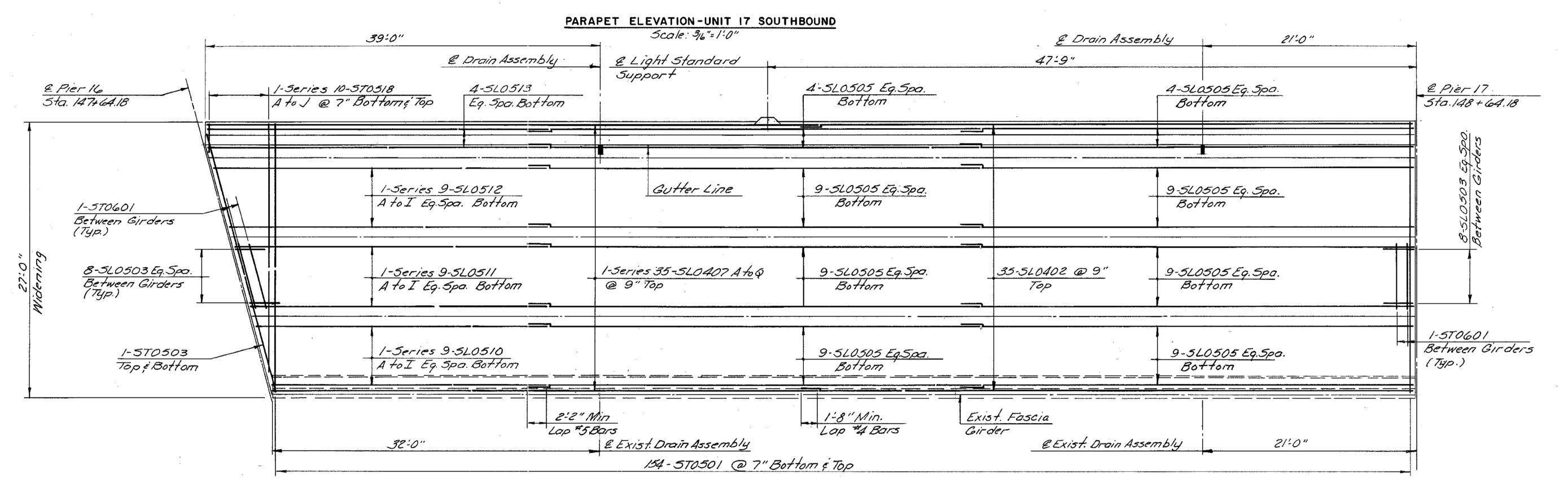
DECK PLAN - UNIT 16 NORTHBOUND

AS SHOWN

	SCALE: AS SHOWN
<u> </u>	CONTRACT NO.: C-13 SHEET NO. 62 OF 106







DECK PLAN - UNIT 17 SOUTHBOUND Scale: 3/6"=1:0"

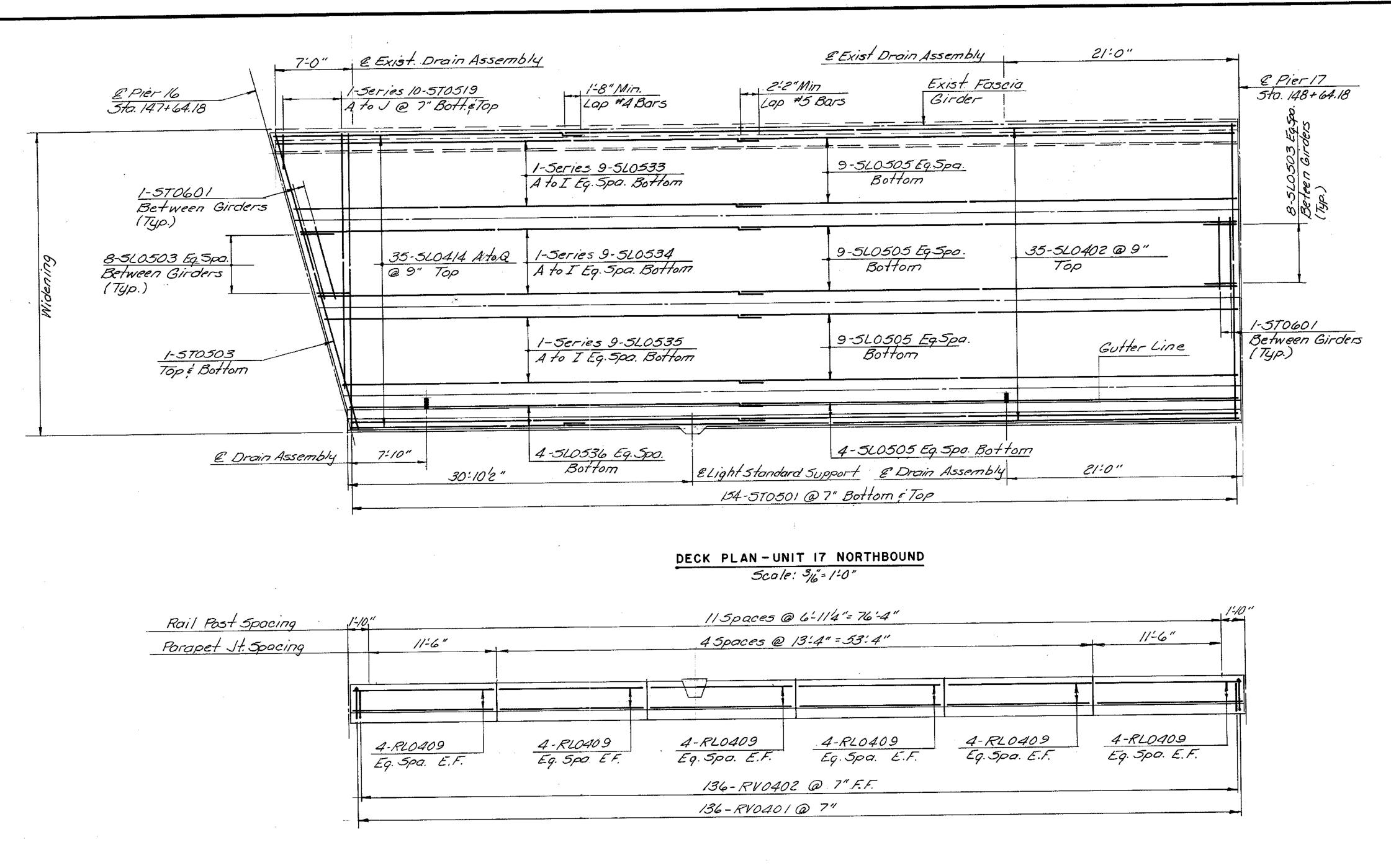
RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

DECK PLAN - UNIT 17 SOUTHBOUND

HOWARD, A

NEEDLES, TAMMEN & BERGENDOFF onsulting engineers	SCALE: AS SHOWN
	CONTRACT NO.: C-13
Alexandria, Virginia	SHEET NO. 63 OF 106

		ВҮ	DATE					
M.	ADE	TAL	3-87			<del> </del>		:
CI	HECKED	T.F.P	3-87		As	Built	TEM	3-89
IN	CHARGE 5	R.		NO.		REVISION	BY	DATE



PARAPET ELEVATION - UNIT 17 NORTHBOUND Scale: 3/6"=1-0"

BY DATE TAL 3-87 MADE 
 TEM
 3-89

 BY
 DATE
 As Built TFP 3-87 CHECKED REVISION IN CHARGE S.R.

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

RICHMOND EXPRESSWAY SYSTEM

64

PROJECT

WIDENING

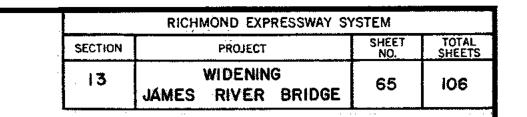
JAMES RIVER BRIDGE

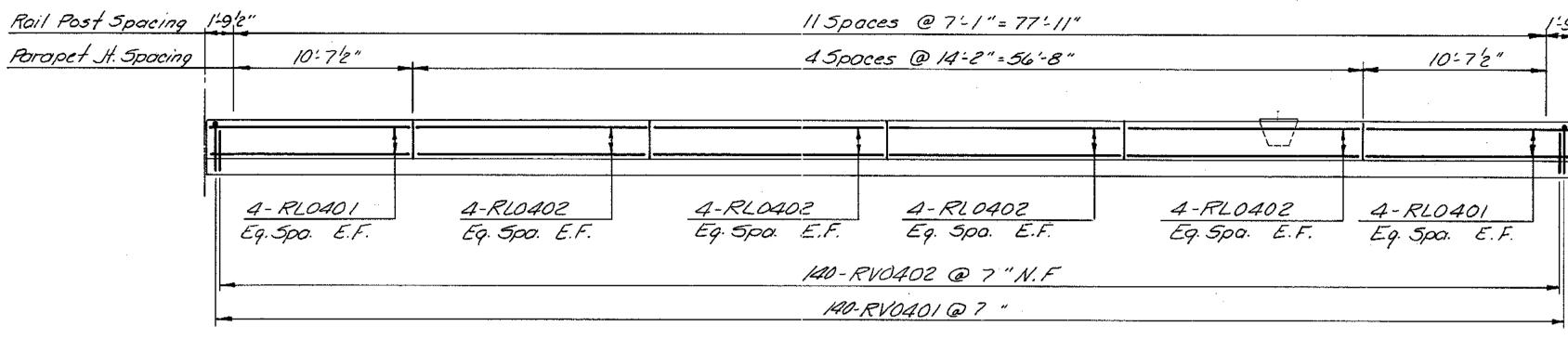
SECTION

DECK PLAN - UNIT 17 NORTHBOUND

SCALE: AS SHOWN HOWARD, NEEDLES, TAMMEN & BERGENDOFF CONTRACT NO.: C-13 consulting engineers SHEET NO. 64 OF 106 Alexandria, Virginia

AS BUILT





PARAPET ELEVATION - UNIT IS SOUTHBOUND & NORTHBOUND (SOUTHBOUND SHOWN) Scale: 316"=1'-0" & Drain Assembly & Light 17-8" 38-0" Standard Supp. & Brg. North Abut. 5ta 149+44.18 & Pier 17 4-510502 Eq. Spa. Bottom & 2"Open St. 4-5L0501 Eq.5pa. Sta. 148+64.18 Bottom in Parapet 9-5L0502 Eq. Spo. Bottom 9-5L0501 Eq. 5pa. 1-570601 1-570601 Botton Between Girders (Typ.) Between Girders (Typ.) 9-5L0501 Eq. Spa. Bottorn 9-5L0502 Eg. Spa. 35-5L0401 @9" 35-5L0402@9" Bottom TOP 9-5L0502 Eq.Spa. Bottom 9-5L050/ Eq.Spa. Bottom Exist. Fascia Girder 2-2" Min. Lap #5 Bars 1-8"Min.
Lap #4 Bars 38-0" E Exist. Drain Assembly

DECK PLAN - UNIT 18 SOUTHBOUND & NORTHBOUND

(SOUTHBOUND SHOWN)

5cale: 3/6"=1-0"

140-570501@ 7" Bottom & Top

 MADE
 TAL
 3-87
 Security
 TEM
 3-89

 CHECKED
 TER
 3-87
 As Built
 TEM
 3-89

 IN CHARGE
 S.R.
 NO.
 REVISION
 BY
 DATE

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

DECK PLAN
UNIT 18 SOUTHBOUND & NORTHBOUND

SCALE: AS SHOWN

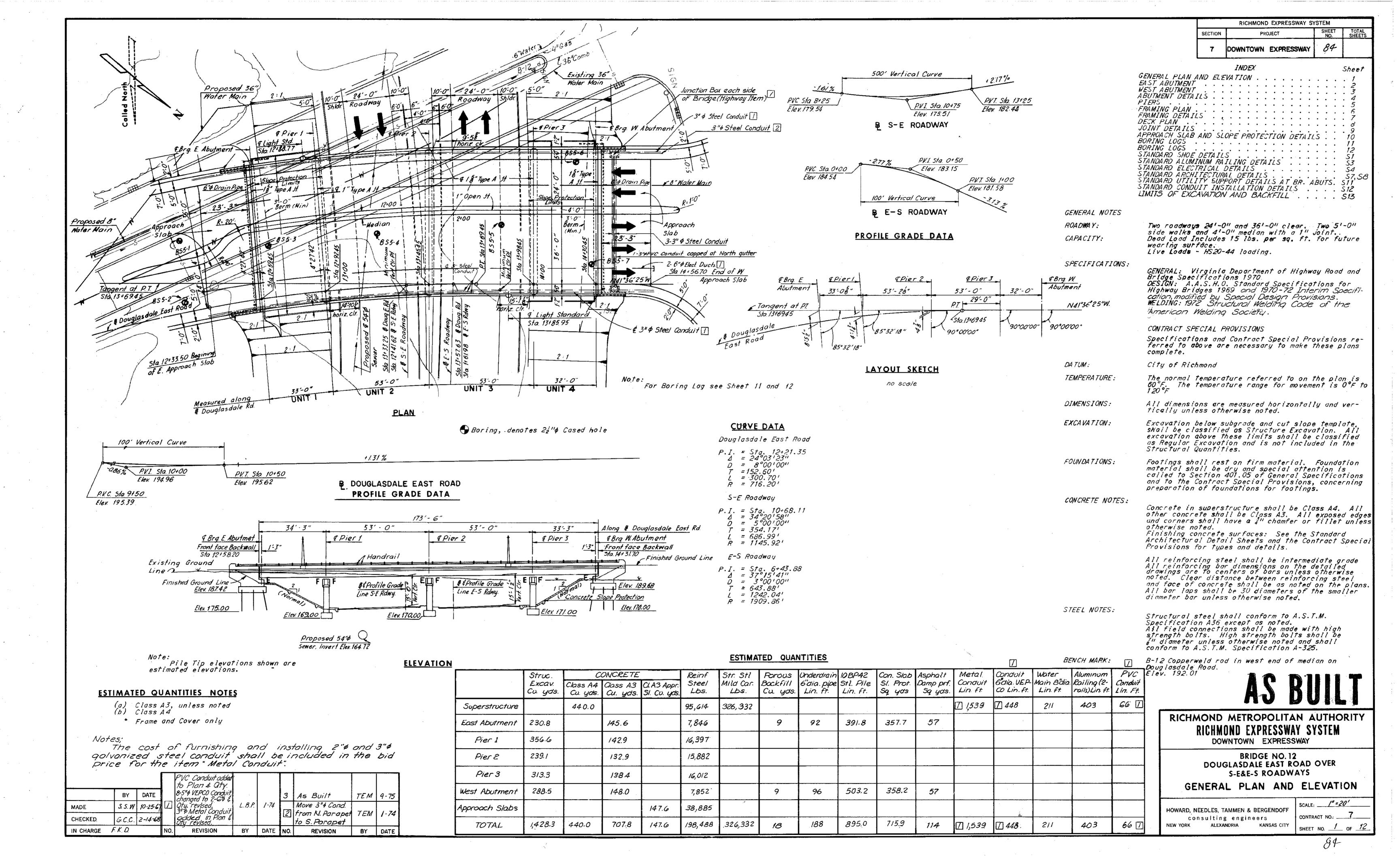
CONTRACT NO.: C-13

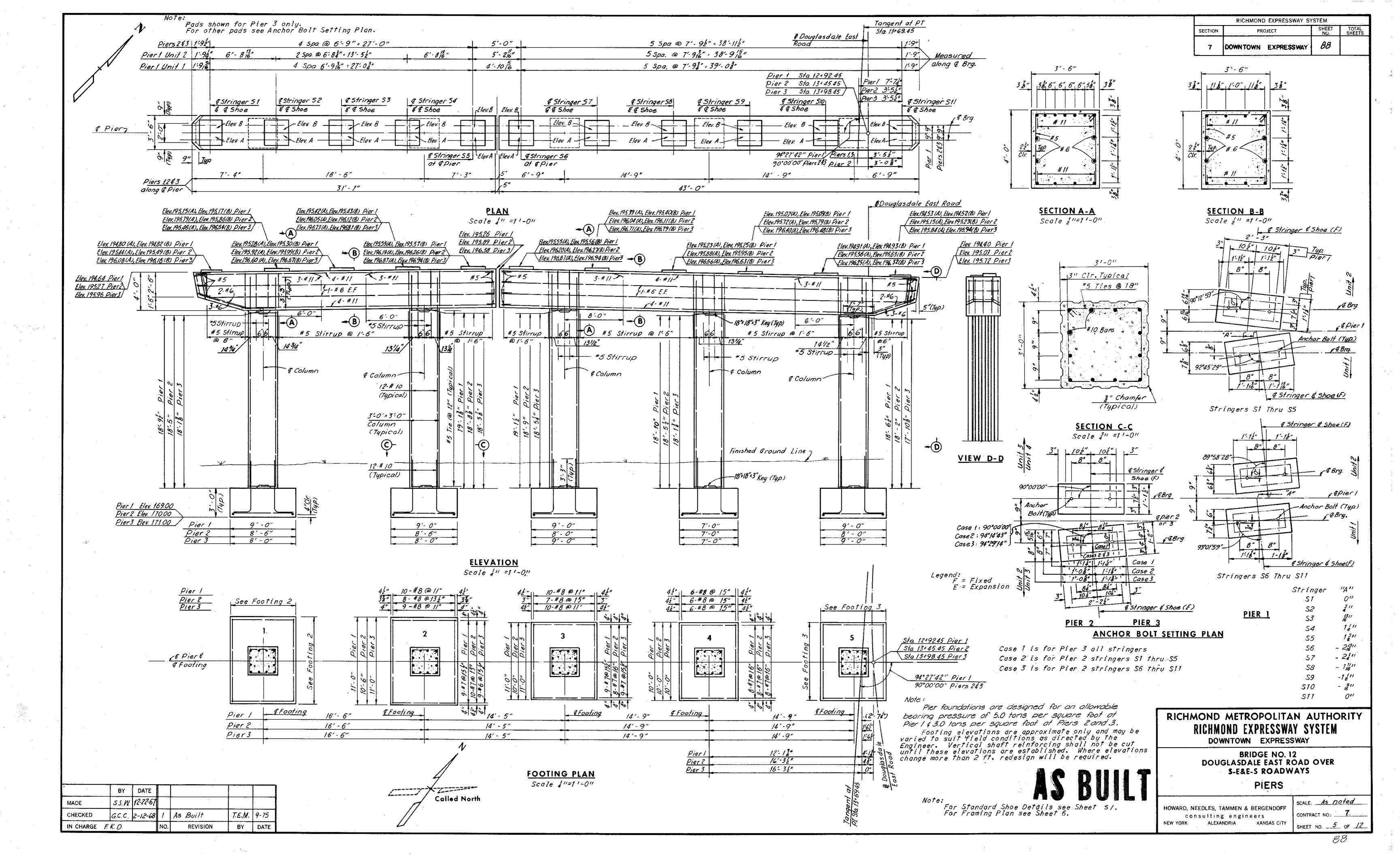
SHEET NO. 65 OF 106

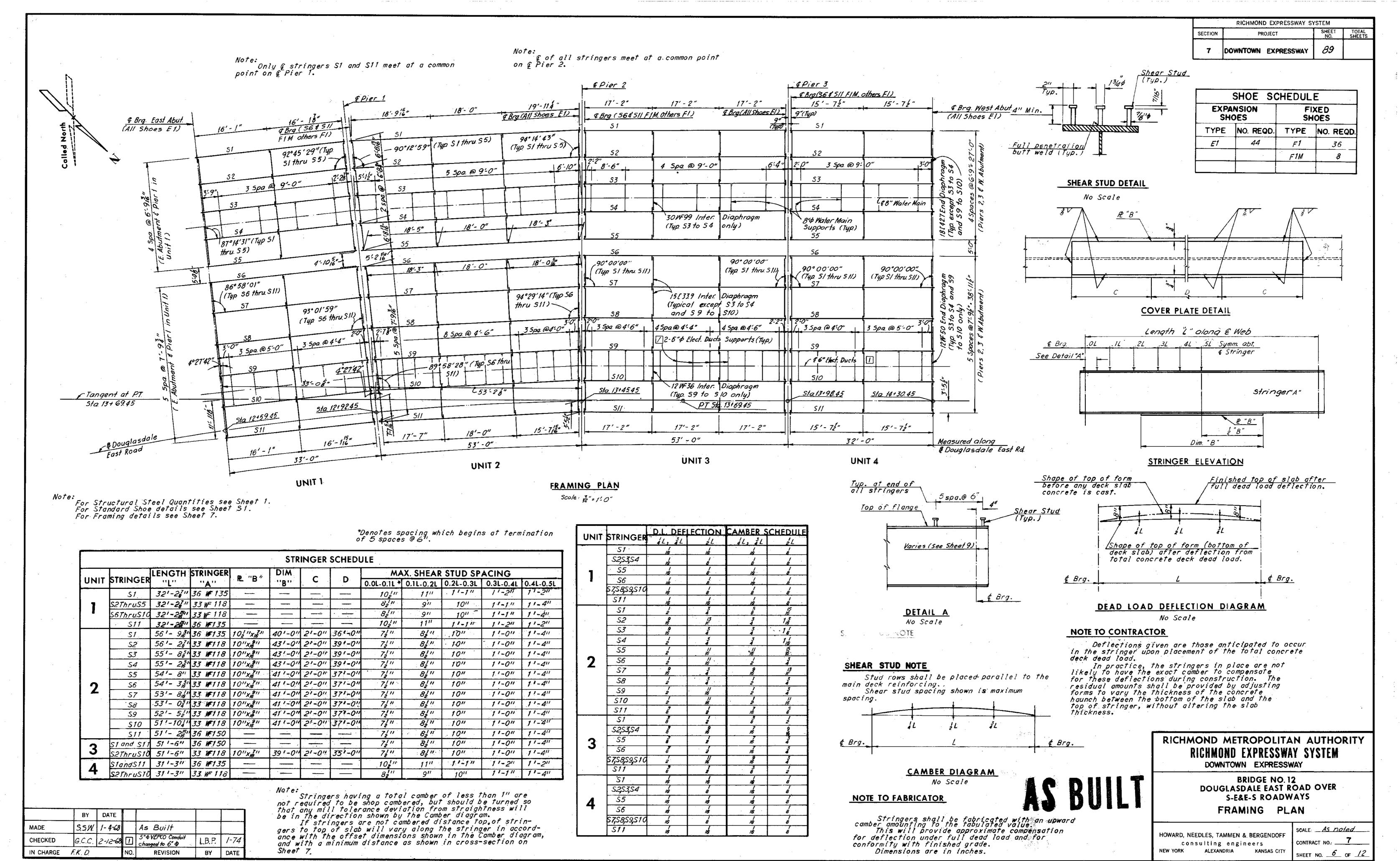
HOWARD, NEEDLES, TAMMEN & BERGENDOFF
Consulting engineers
Alexandria, Virginia

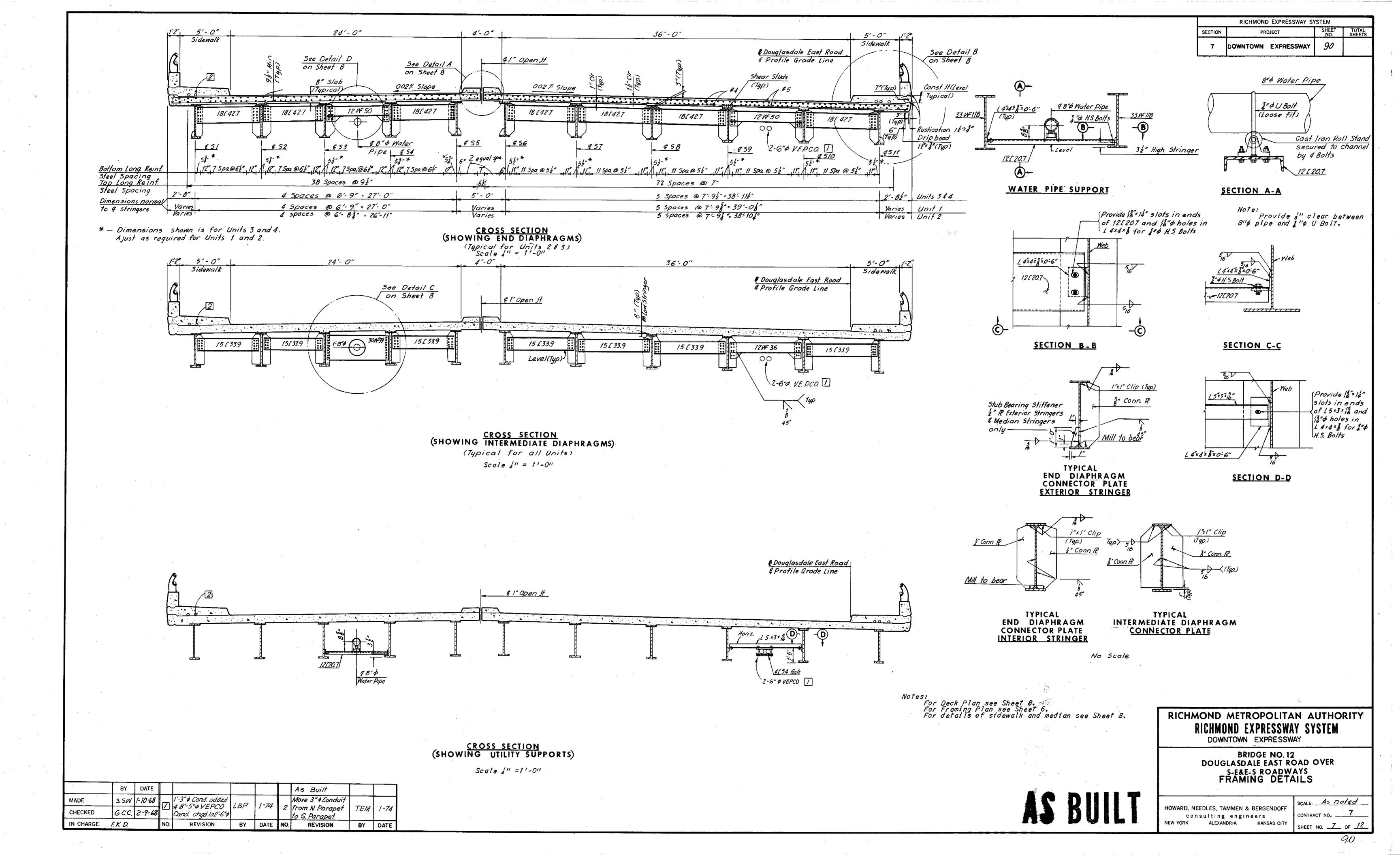
AS BUILT

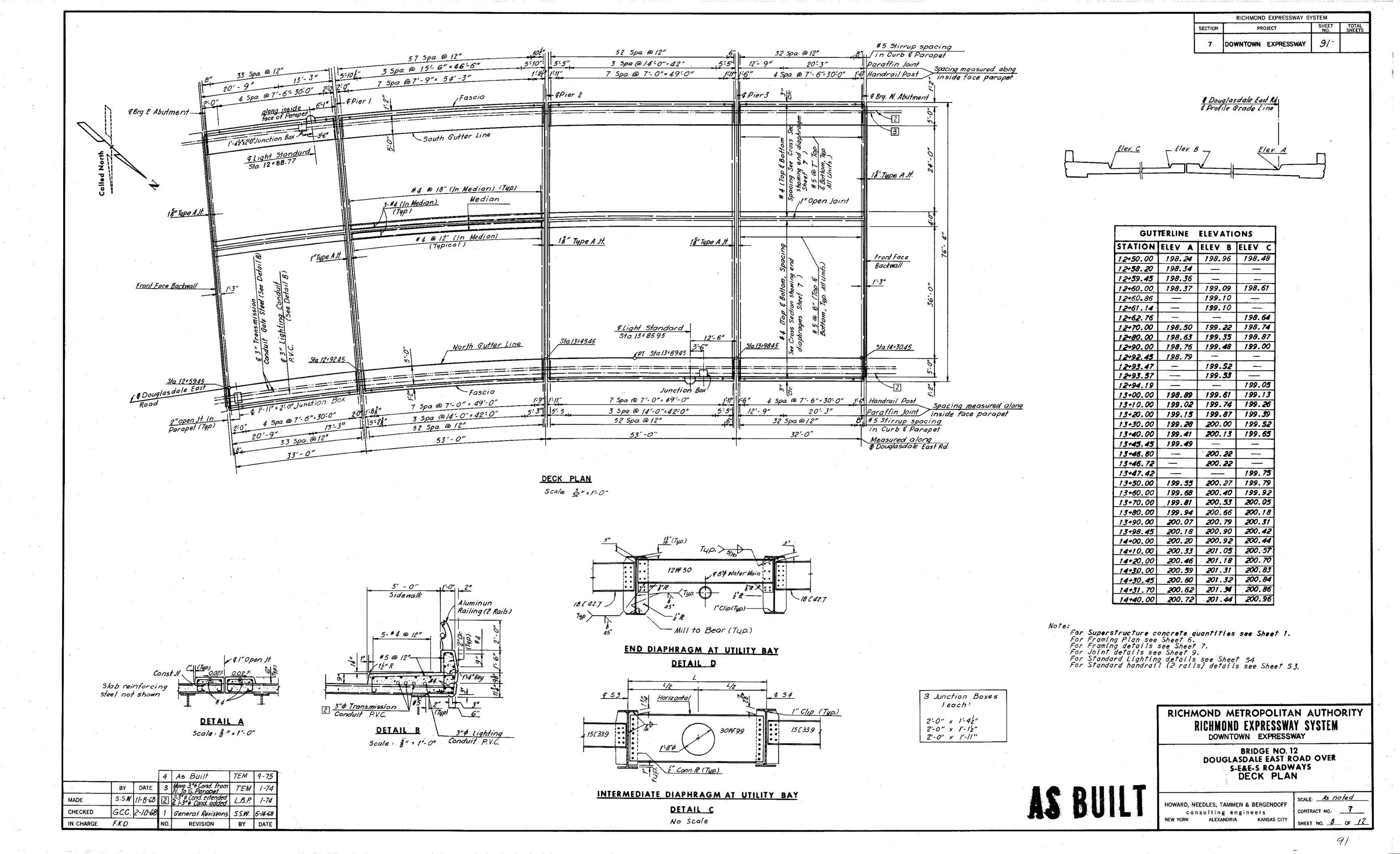
Douglasdale Road Over Downtown Expressway Connector (VA-146)



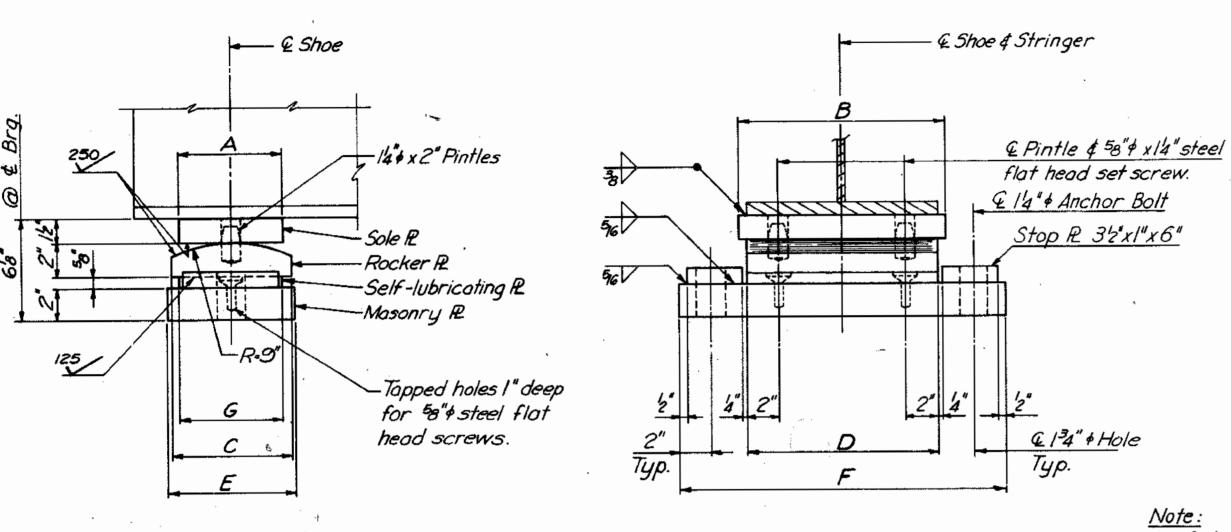




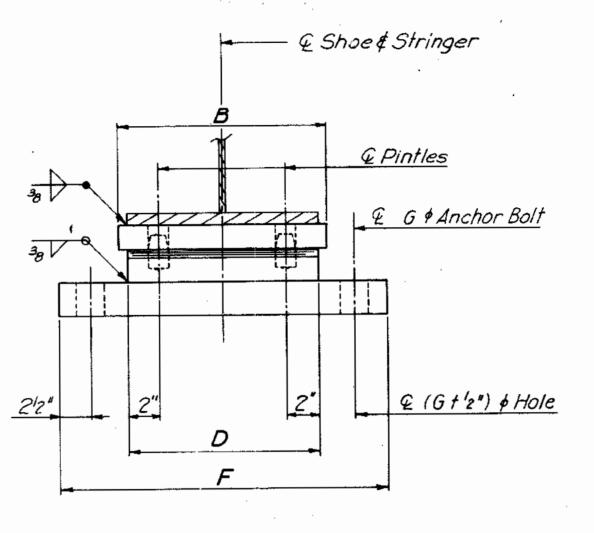


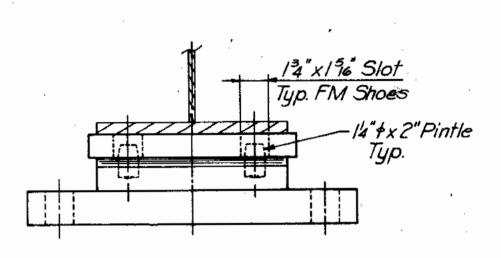


RICHMOND EXPRESSWAY SYSTEM PROJECT DOWNTOWN EXPRESSWAY



- & Shoe -14" \$x2" Pintles Rocker R Mosonry R





Fixed Modified Shoes some as Fixed Shoes except as shown.

SIDE ELEVATION

MADE

CHECKED

END ELEVATION

Sole plate is to be beveled to compensate for grade.

SIDE ELEVATION

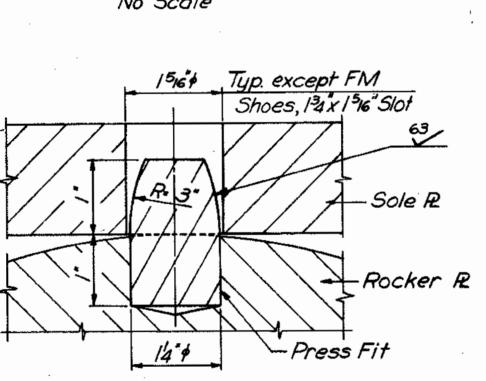
END ELEVATION

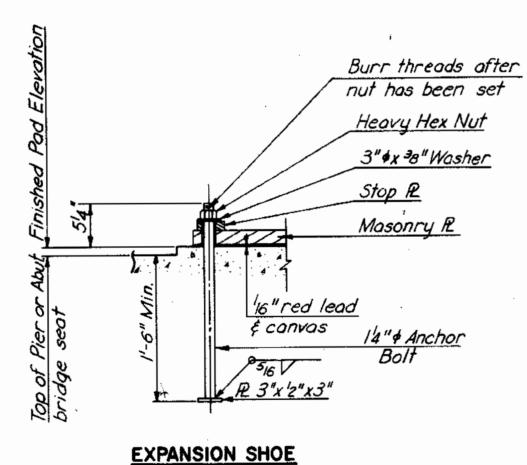
END ELEVATION

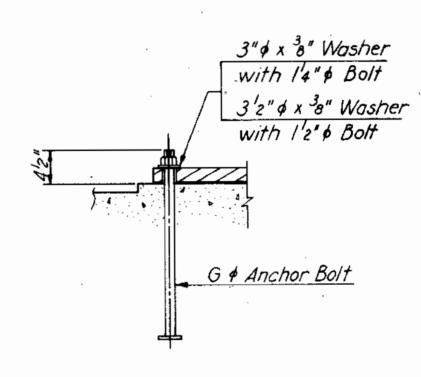
#### **EXPANSION SHOE** No Scale

PINTLE DETAIL

Scale: 34"= |"







Anchor Bolt for Fixed Shoes

same as Anchor Bolt for Exp. Shoes

### Shoe Notes.

FIXED MODIFIED SHOE No Scale

FIXED SHOE

No Scale

Moterials for shoes (exclusive of self-lubricating plates) shall be high strength low alloy structural steel conforming to A.S.T.M. Specifications A588.
Top of masonry plates, bottom of rocker plates and top and bottom of sole plates shall be planed, straightened or otherwise treated to secure true plane surfaces. Contact surfaces noted on the plans with finish symbols shall be finished in occordance with the American Standards Association surface roughness requirement as defined in ASA B45.1, Surface Roughness, Waviness and Lay, Part I.

The plates comprising the expansion shoes shall be set so as to be truly centered

under full dead load at a temperture of 68°F.

Concrete pads shall be formed integral with abutment or pier and not less than 's" or more than 'a" above finished elevation. Dress down pads by rubbing, grinding or as otherwise approved by the Engineer, to true level surfaces at the finished elevation. Anchor bolt assemblies shall conform to AS.T.M. A-307-66 and shall be hot-dip galvanized conforming to A.ST.M. A-153-66.

Templates shall be used to accurately set the anchor bolts.

Materials for self-lubricating plotes shall be lead Tin Bronze conforming to A.S.T.M Specifications B22, alloy D modified to the extent that ercent lead is allowable.

hall be included with structural steel item for payment.

The state of the s

### FIXED SHOE

except as shown.

#### ANCHOR BOLT DETAIL No Scale

					(	S H C	ΟE	DII	MENS	IONS							
	· ·	EXP/	ANSIO	N SH	OES	,		10 0 00 00	FIXED SHOES								
TYPE	NO. REQD.	A	В	С	D	Ę	F	G	TYPE	NO. REQD.	A	В	С	D	Ε	F	G
E/	63	6"	/-/"	72"	1-0"	8"	1482"	6"	F/ /	<i>5</i> 3	6"	/-/"	6"	1-0"	72"	1-9"	14"0
£З	12	6"	1-1"	9"	1-1"	92"	1-92"	7"	FIM	16	6"	/-/"	6"	/-'0"	72"	1-9"	/4"ø
<i>E</i> 6	7	6″	1-7"	8"	1-6"	82	2:22	6"	F4	10	6"	1:7"	6"	16	7"	24"	120
E7	7	6"	147"	95"	1.6"	102	2-22"	72	F4M	4	6"	1-7"	6"	16	7"	24"	129
E2	6	6"	1-1"	81/2	1-0"	9"	1-8/2	6%	F12	8	6"	1:1"	6"	<i> - "</i>	72"	1-10"	144
									FI2M	4	6"	<i> -</i> ' "	6"	1-1"	7/2"	120	140
											ļ				. `	-	
									±. *								
,														<u> </u>			<u> </u>
																	<u> -</u>

E	Shoe		1.5. to	2.5' pero Shoes sho
@ & Brg. 6'2" / 2" 2" / KB	2 V 2 V	A	- 14"\$ x 2" - Sold	· ·
	-	E	-	

## SIDE ELEVATION FIXED SHOE TYPE FI3

### RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM DOWNTOWN EXPRESSWAY

### STANDARD SHOE DETAILS **Bridges 12, 36, 37**

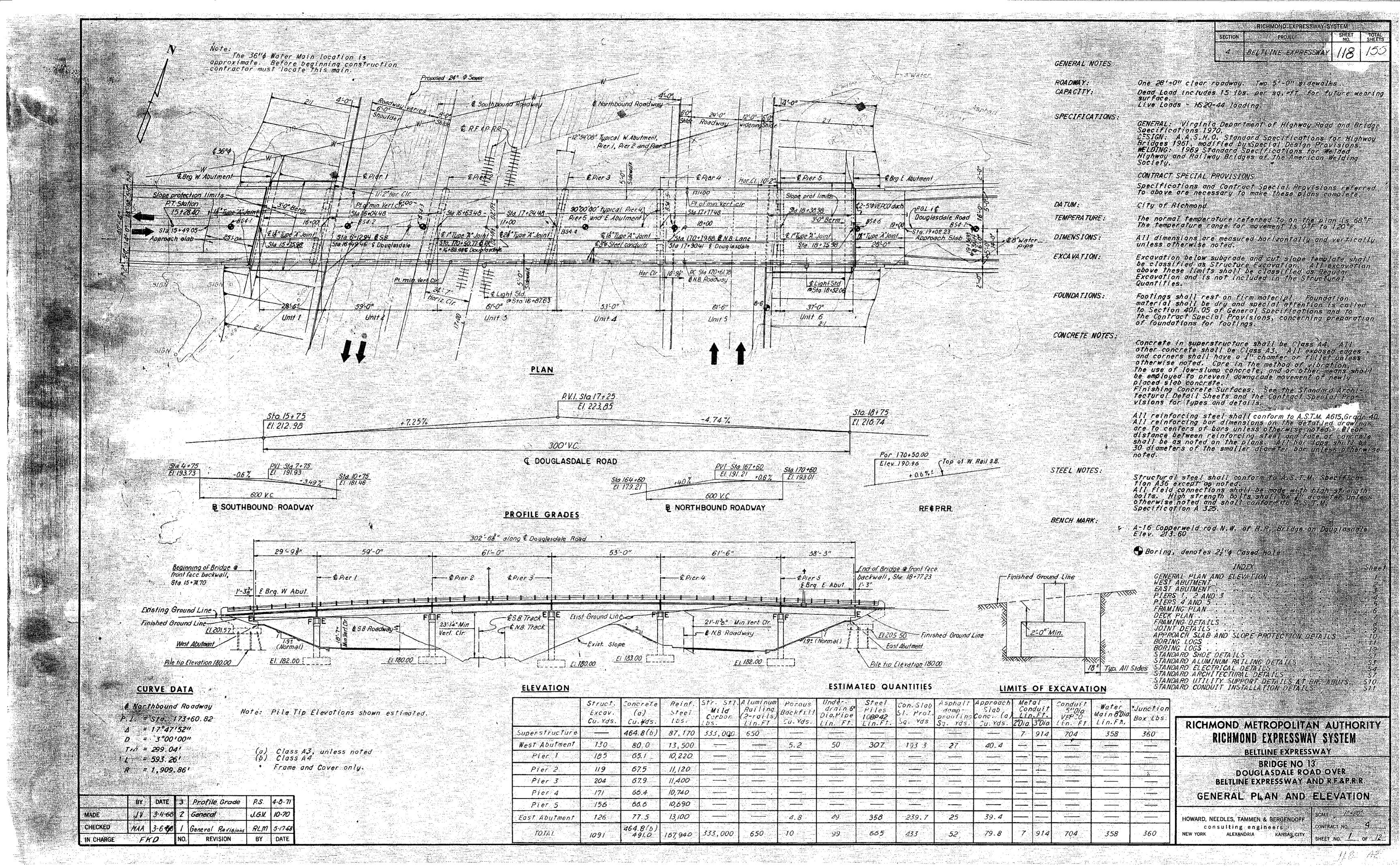
SCALE AS SHOWN consulting engineers ORK ALEXANDRIA KANSA

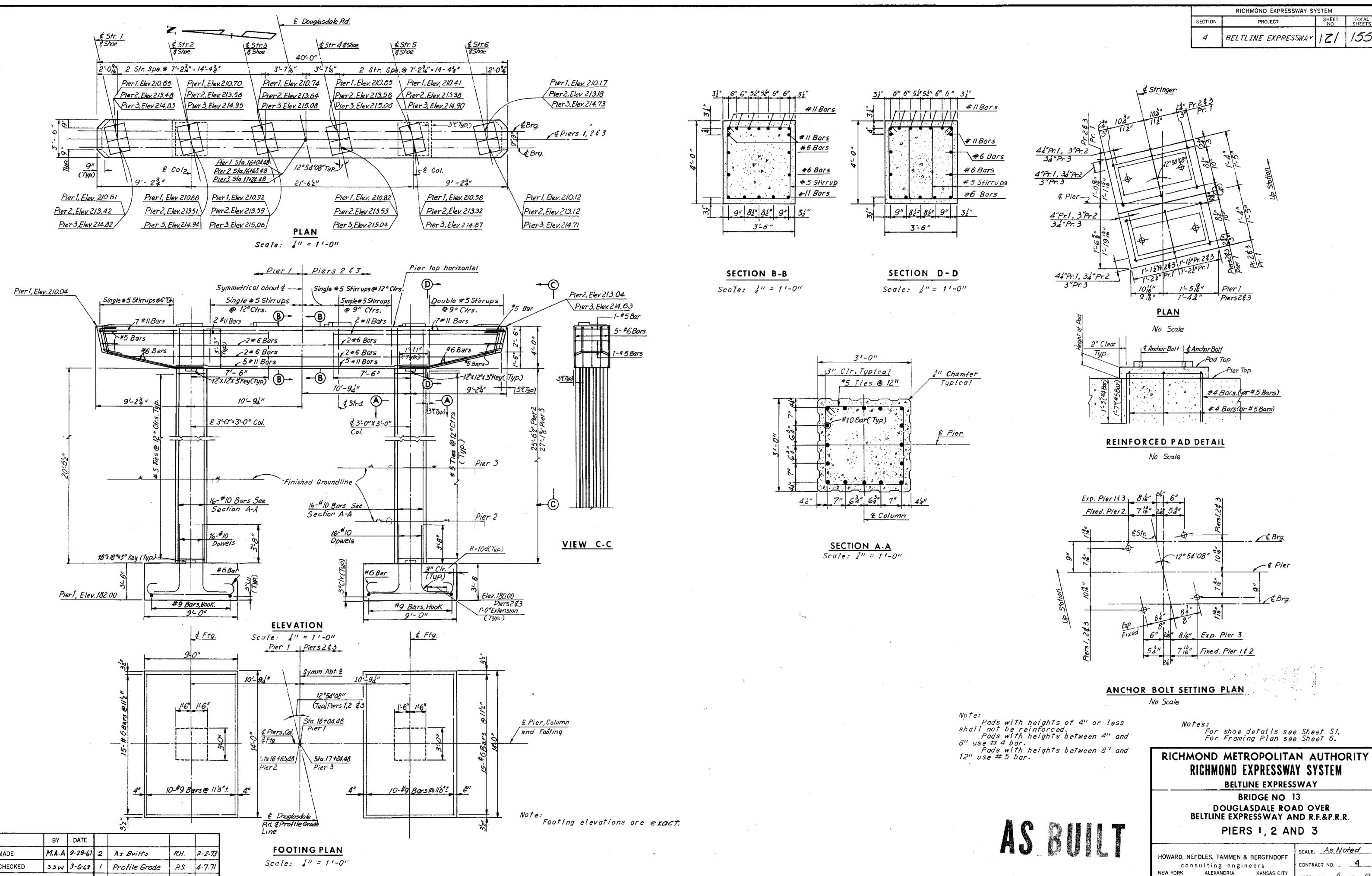
BY DATE No Scale: S.B.P. 3-68 Note: For details and dimensions not shown A.J.S. 3-68 As Built see Fixed Shoe details above. IN CHARGE PR.Y. REVISION

SHEET NO. S.

Douglasdale Road Over

Northbound and Southbound Powhite Parkway (VA-76) connector to I-195 and CSX Railroad





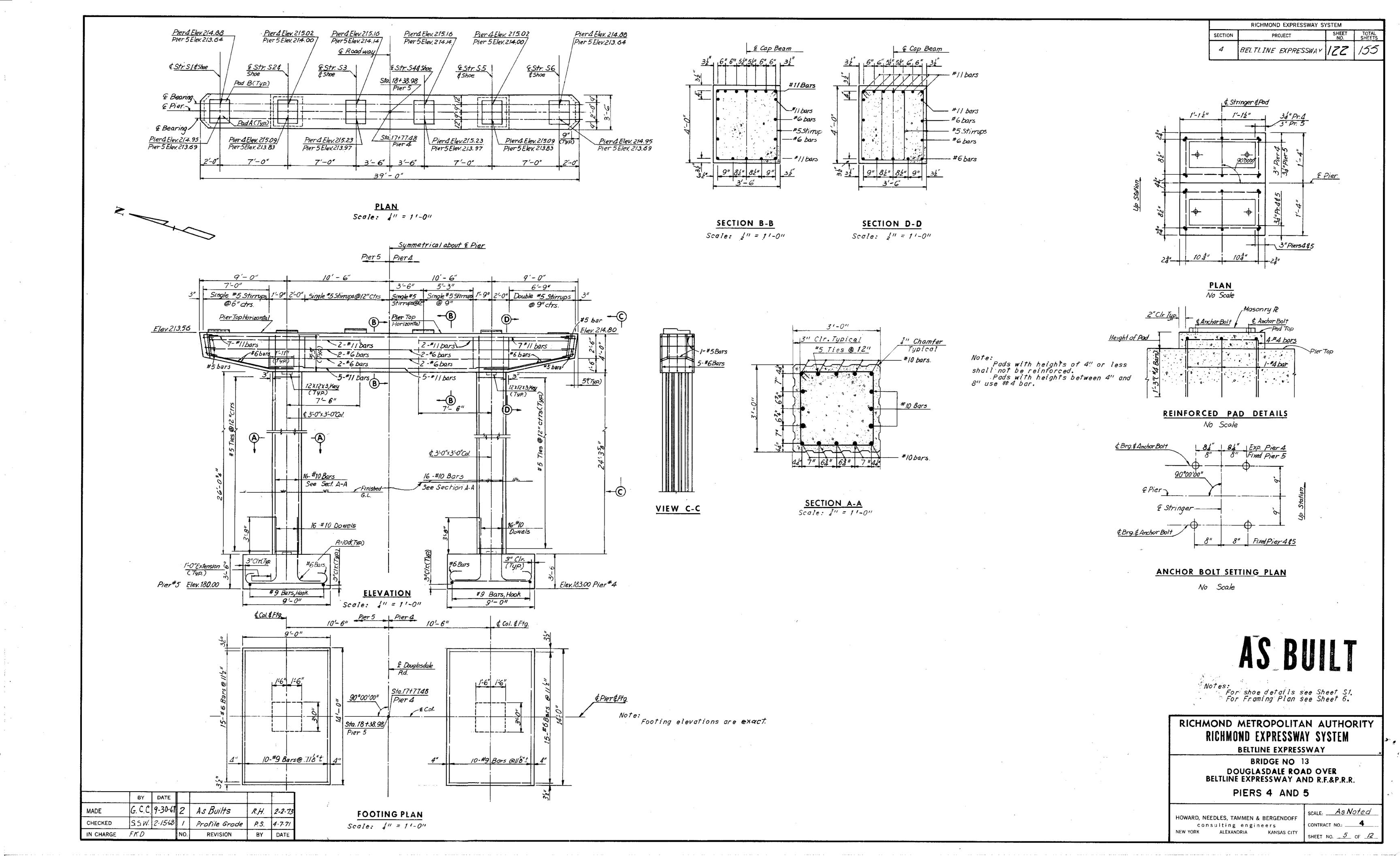
BY DATE

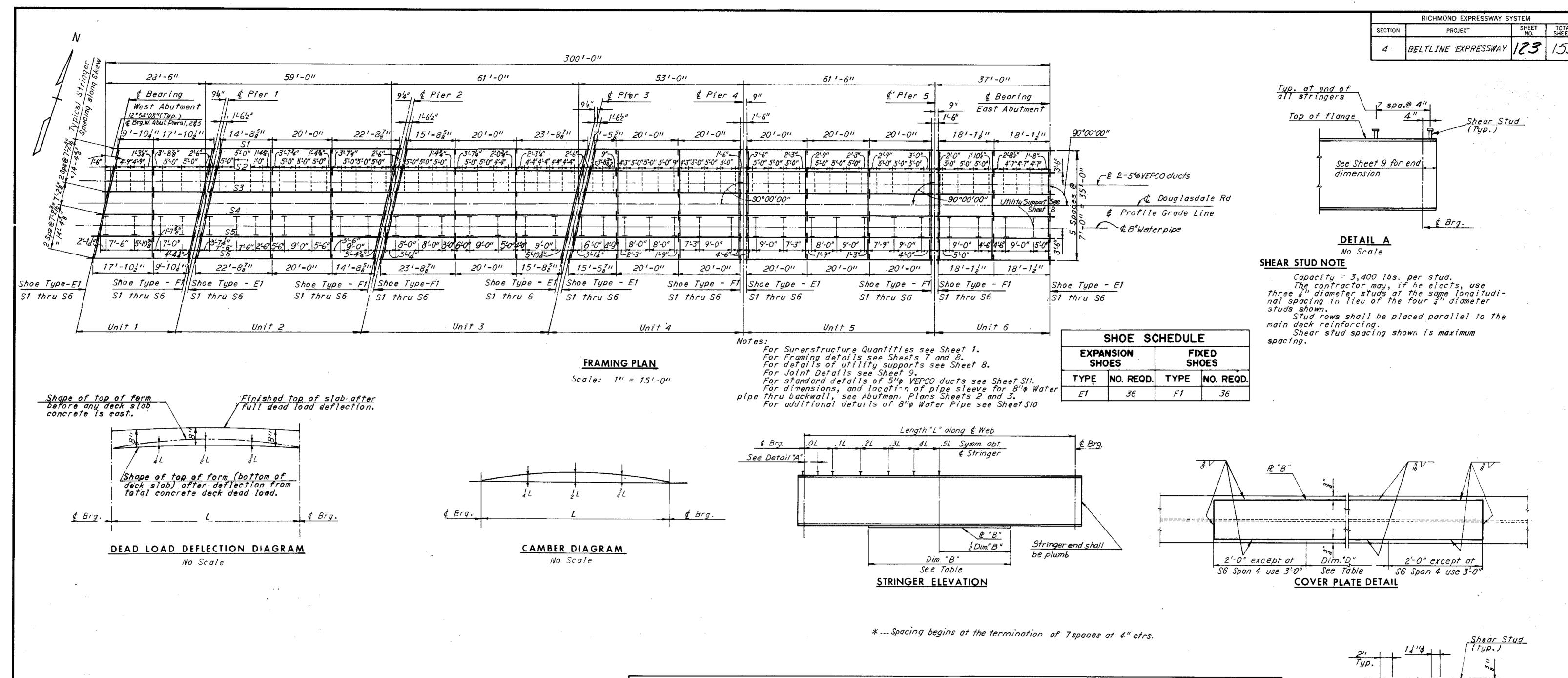
REVISION

FKD

IN CHARGE

SCALE: As Noted CONTRACT NO.: \_\_ 4\_\_



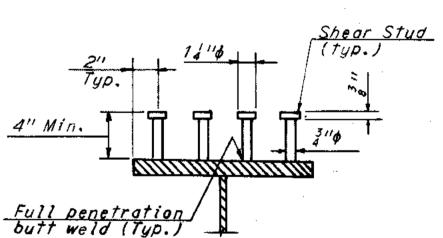


	·		DEA	DLOAL	) DEFLE	CTION SC	"HEDOF!					
UNIT 1			UNI	UNIT 2 UNIT		Т 3	3 UNIT 4		UNI	T 5	UNIT 6	
STRINGER	¼L ¾L	½L	¼L ¾L	1⁄2L	¼L ¾L	<b>½</b> L	¼L ¾L	½L	1/4L 3/4L	½L	¼L ¾L	<b>⅓</b> L
S1	16 11	8'''	511 8	15 //	16	/"	4"	8"	1111	1,6"	8'11	311 16
S2	16 11	8'''	911	13"	1/11	1511 16	511 16	2"	1111	1"	8"	<i>إ</i> !!
S3	11 16 11 B	8'11	911	130	1111	1511 16	311	9"	1111	/"	811	8'11
<i>S4</i>	111	8"	911 16	1311 16	1111	1511 16	ž11.	9"	//11 16	1"	8"1	8'"
S <b>5</b>	/6 //	8'''	911	1311	1111	15 II 16	711	11 11	liii K	1"	<i>!</i> "	<i>!</i> "
S6	16"	8'''	511 8	151	//// /6	/"	111	3"	1111	1/6"	8"	31. 16

ļ							
				4	As Builts	RH.	2-2-75
		BY	DATE	-3	Profile Grade	P.S.	4-7-71
	MADE	A.H.H	11-20-67	2	General		10-70
	CHECKED	5,5,W.	2-8-68	7	General Revisions	МНН	3-12-68
	IN CHARGE	FKD	- · · · · · · · · · · · · · · · · · · ·	NO.	REVISION	BY	DATE

N	ofe:
	Stringers having a total camber of less than 1" are
n	of required to be shop cambered, but should be turned so hat any mill tolerance deviation from straightness will
7	hat any mill Tolerance deviation from Straightness Will
D	e in the direction shown by the Camber diagram.  If stringers are not cambered distance top of strin-
0	ers to top of slab will vary along the stringer in accord
a	ers to top of slab will vary along the stringer in accordince with the offset dimensions shown in the Camber diagram
a	and with a minimum distance as shown in cross-section on
S	heet 7.

						STRINGER	SCHEDULE							
IINIT	STRINGER	STRINGER LENGTH	NGTH STRINGER	₽ "B"	DIM. "B"	DIM. " D "		SHEA	R STUD SP	ACING		CAMB	ER SCH	EDULE
	JIKHYGEK	"Lu	SIZE	ų, D 	DIM. D	DIMI.	.OL1L *	.1L2L	.2L3L	.3L4L	.4L5L	1/4L	1/2L	3/4L
.=	S1 & S6	271-8311	36WF 135				92111	11"	132"	162111	22"	711	511 8	711
1	S2 & S5	271-8311	33 WF 118				92"	11"	13211	162"	22"	711	511 8	16.1
	S3 & S4	271-8311	33 WF 1181				9/14	11"	13211	16211	22''	711 16	511 8	711 16
	S1 & S6	571-5/11	36 WF 135	10/x4	43'-2"	39 1-211	8"	92"	112"	14"	182''	2311	316"	2311
2	S2 & S5	571-5/11	36 WF 135	10/x3	43'-2"	39' <b>-</b> 2"	8"	9/11	112"	14"	18211	28"	27"	28"
	S3 <b>&amp;</b> S4	57' <b>-</b> 5 <sub>2</sub> '"	36 W 135	102 x 3	43'-2"	39'-2"	8"	9/11	11211	14"	18/11	2'8"	28711	28"
	S1 & S6	59'-52'"	36 WF 135	102 x 3	44'-8"	40'-8"	8"	92111	1,12"	14"	18211	2511	34"	215 11
3	S2 <b>&amp; 5</b> 5	59' <b>-</b> 5½'''	36 F 135	102 X4	44'-8"	40'-8"	8"	92111	11211	1'4"	18211	216	33,11	2511
	53 & 54	59' <b>-</b> 5 <sub>2</sub> '''	36 WF 135	102 x 4	44'-8"	40'-8"	8"	9/11	112"	14"	18211	2511	33/1	251
	S1	47'-58"	36 WF 135	102x2	351-811	31'-8"	8"	9"	11".	14"	18211	14"	1711	14"
	<i>S2</i>	49'-08"	36 F 135	10 x x	361-1011	32'-10"	8"	9"	11"	14"	18/11	1711	2"	13"
4	<i>S3</i>	50'-88"	36 W 135	102 x8	38'-1"	34'-1"	8"	911	1111	14"	18/11	1911	216"	1 <u>9</u> 11
	S4	521-38"	36 W 135	102 X 4	391-3"	35'-3"	8"	9"	11"	14"	18211	15"	25"	1511
	<i>S5</i>	53'-10g5"		102 X8	40'-5"	36'-5"	8"	9"	11''	14"	1.82 "	13,11	2'2"	13/1
	<i>S6</i>	55'-5 <sub>8</sub> "	36 W 135	10/x1	41'-8"	35'-8"	8"	9"	1111	14"	18211	1/5"	2"6"	1/5"
	S1 \$ S6	60'-0"	36 WF 135	$10^{1}_{2}x_{4}^{3}$	45'-0"	41'-0"	8"	92111	11211	14"	182111	23"	35/1	2811
5	S2 & S5	60'-0"	36 W 135	10/x4	45'-0"	41'-0"	8"	92111	112"	14"	18111	23"	34"	28"
	S3 & S4	60'-0"	36 W 135	102 x 4	45'-0"	41'-0"	8"	91"	112"	14"	18211	238"	34"	238"
	S1 & \$6	36'-3"	36 WF 135				9211	11"	132111	162"	22''	4"	76"	4"
6	S2 & S5	36'-3"	33 WF 118				9/11	11"	13/11	16211	22"	3 "	2"	37/ 8
	S3 & S4	36'-3"	33 WF 118				91"	11"	13!"	16/11	22"	3"	2"	3 /



SHEAR STUD DETAIL

No Scale

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

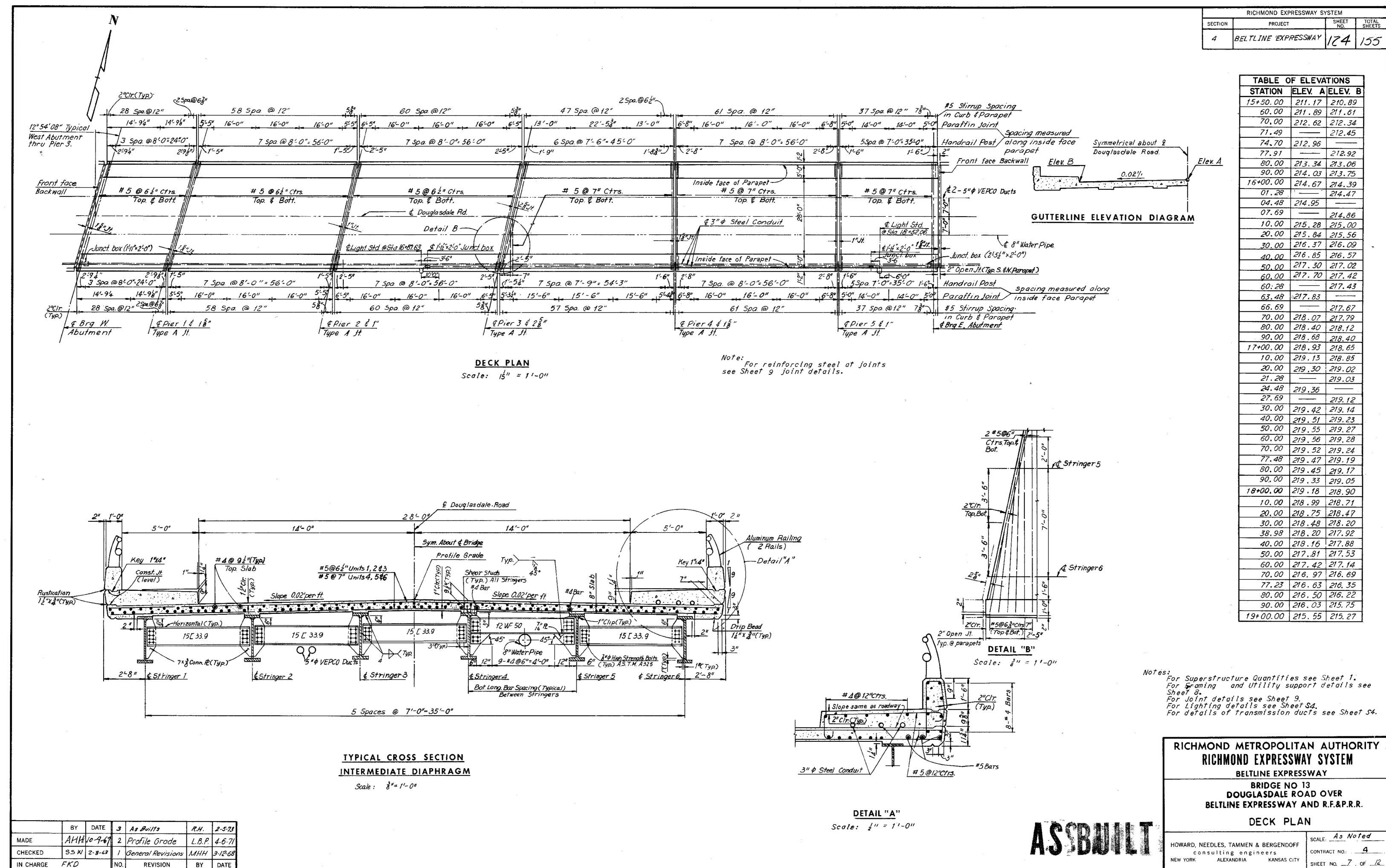
BELTLINE EXPRESSWAY

BRIDGE NO 13 DOUGLASDALE ROAD OVER BELTLINE EXPRESSWAY AND R.F.&P.R.R.

FRAMING PLAN

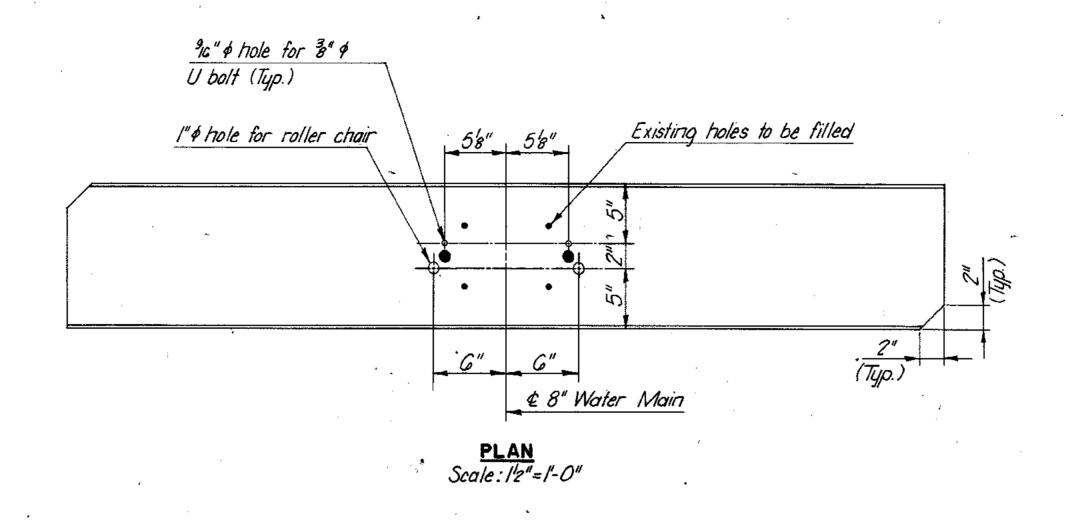
SCALE: As Noted HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers NEW YORK ALEXANDRIA KANSAS CITY

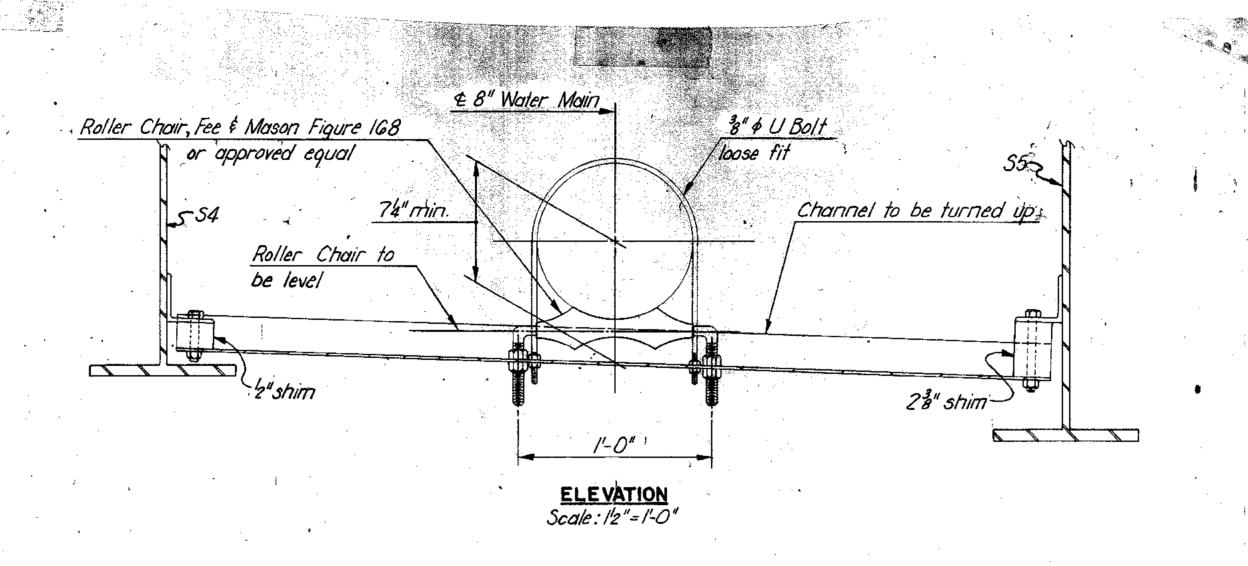
CONTRACT NO.: \_\_ SHEET NO. 6 OF 1

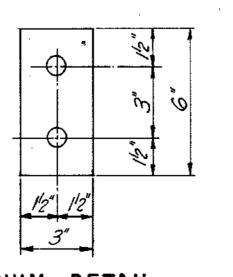


SHEET NO. -7 . OF  $-\frac{12}{}$ 

RICHMOND EXPRESSWAY SYSTEM								
SECTION	PROJECT	SHEET NO.	TOTAL SHEETS					
4	BELTLINE EXPRESSIVAY	11.57	/					







	BY	DATE				
MADE						
CHECKED			/	AS Builts	RH	2-5-73
IN CHARGE			NO.	REVISION	BY	DATE

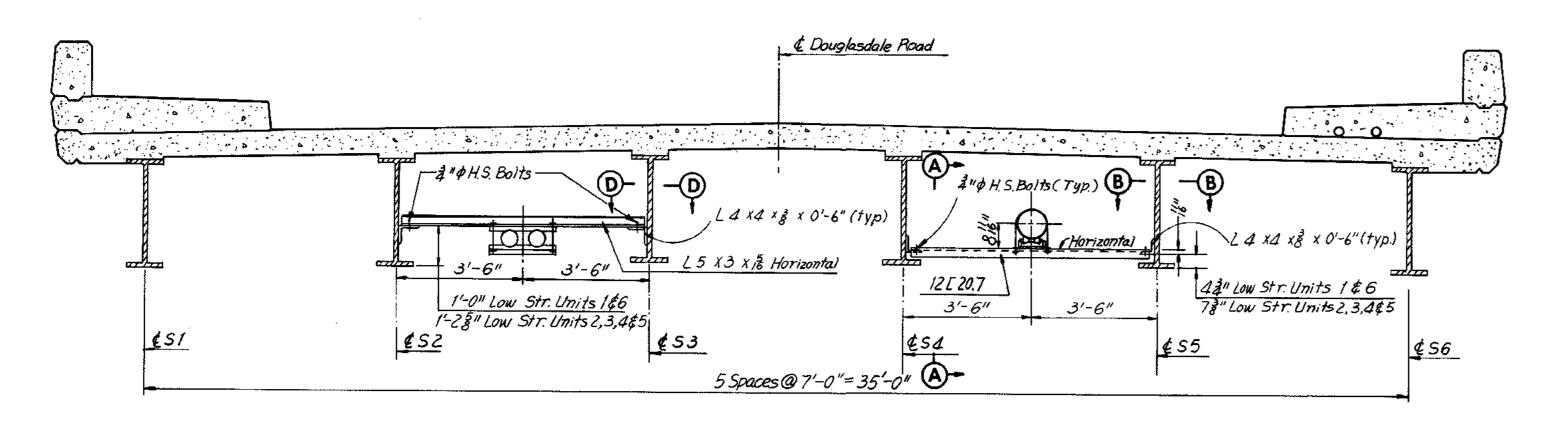
••	
SHIM	DETAIL
Scale:	3"=1"-0"
JUCITE.	J 1 U

RICHMOND METROPOLITAN AUTHORITY

RICHMOND EXPRESSWAY SYSTEM

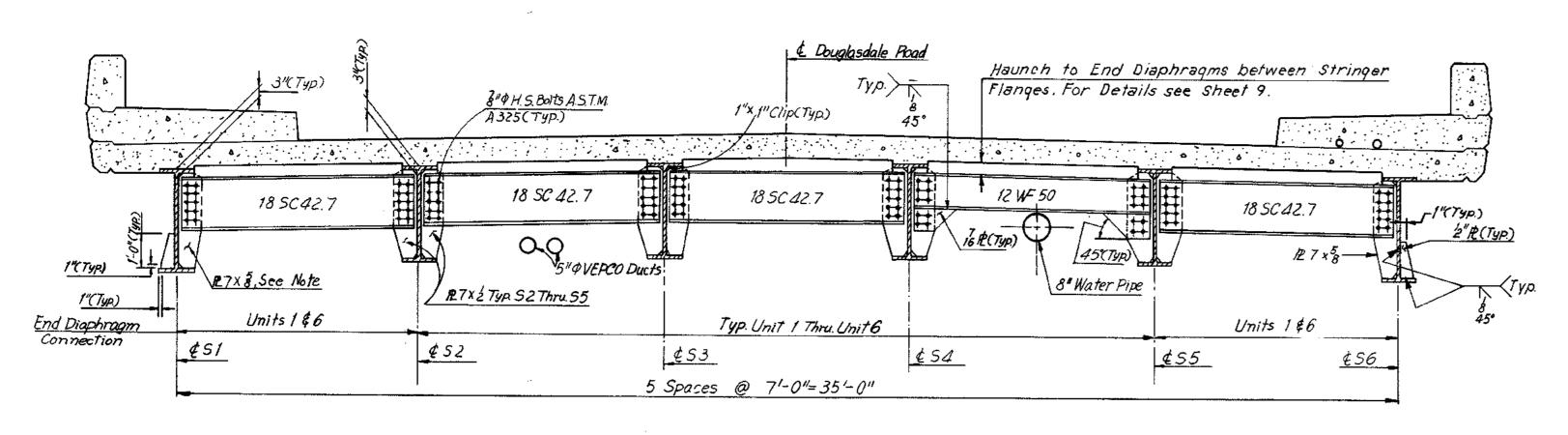
BRIDGE NO. 13
DOUGLASDALE ROAD OVER
BELTLINE EXPRESSWAY AND R.F. & P. R.R.
WATER MAIN SUPPORT DIAPHRAGMS
UNITS 1 & 6

	SCALE: AS SHOWN
	CONTRACT NO.: 4
ARD, NEEDLES, TAMMEN & BERGENDOFF General Consultants	SHEET NO. 8A OF 12



### UTILITY SUPPORTS TYPICAL SECTION BETWEEN DIAPHRAGMS

Scale : 3" = 1'-0"



## CROSS SECTION

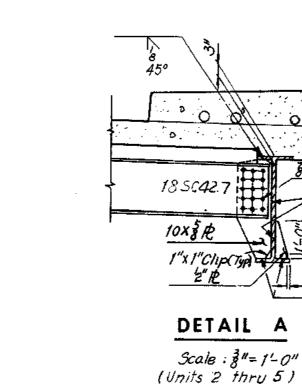
#### END DIAPHRAGMS

Scale: \$"=1'-0" Note:

Note:

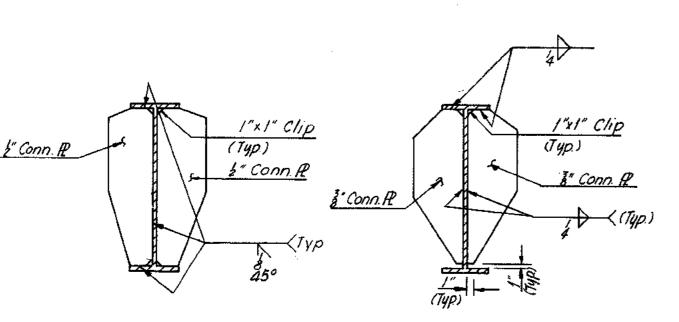
End diaphragms and connections shown obove are typical for all units with the following exceptions:

7"x8" Bearing and connection plates on exterior stringers with 2 rows of 5 bolts as shown for Units 1 and 6, to be replaced by 10"x8" plate with 3 rows of 5 bolts for Units 2 through 5, see Detail A below.



В"ФН.3.Bolts A.S.T.M.

For Intermediate diaphragms see Sheet 7.



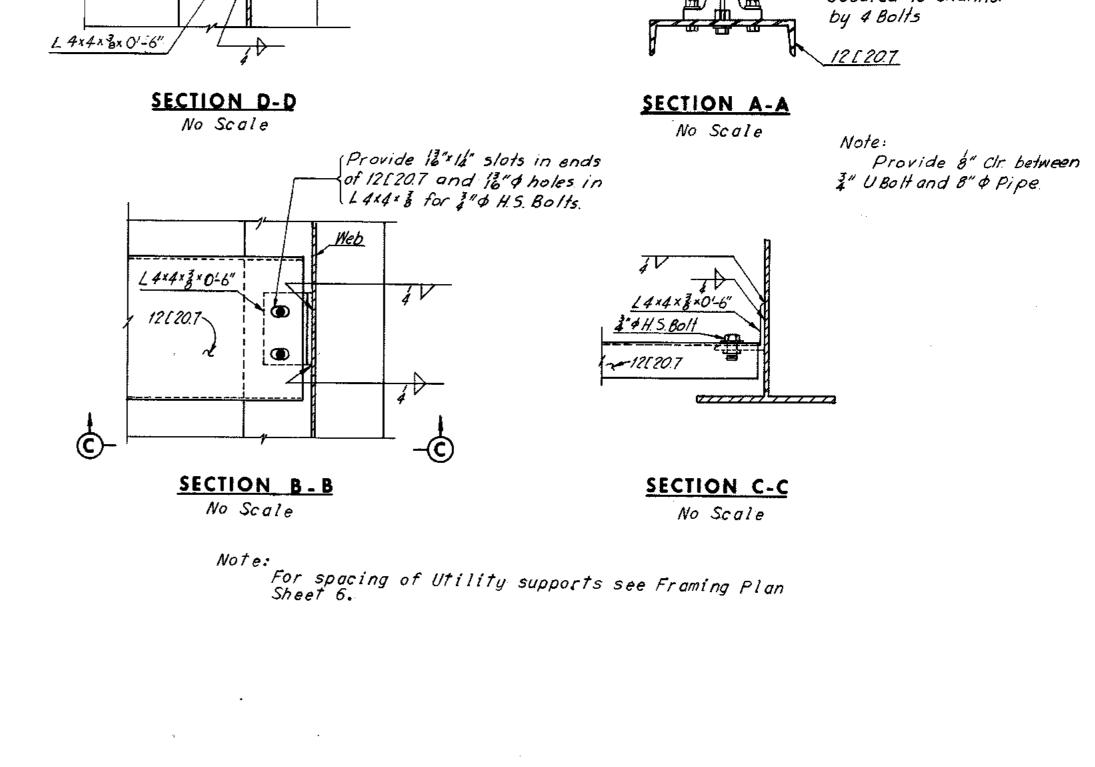
TYPICAL END DIAPHRAGM CONNECTOR PLATE INTERIOR STRINGER

No Scale

TYPICAL INTERMEDIATE DIAPHRAGM CONNECTOR PLATE

No Scale

	ВҮ	DATE				
MADE	AMH	11-27-61	2	As Builts	RH	2.5.73
CHECKED	S.5.W.	2-14-68	1	General Revisions	MHH	3-12-68
IN CHARGE	FKD		NO.	REVISION	BY	DATE

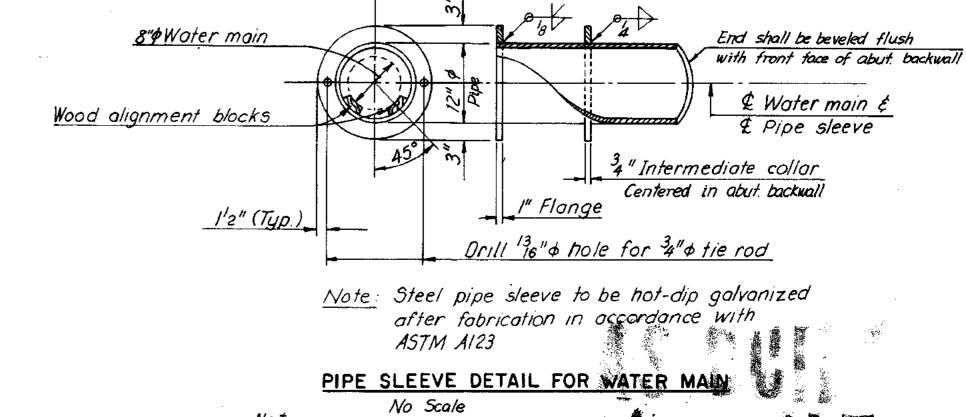


(Provide 18" 14"

slots in ends of 15 x 3 x /6 and 18" \$

holes in L4x4x3

for 1 4 H.S. Bolts.



8"4 Water Pipe

Cast Iron Roll Stand
secured to channel

(Loose fit)

For location and addition 8"ø Water pipe see Abutmen

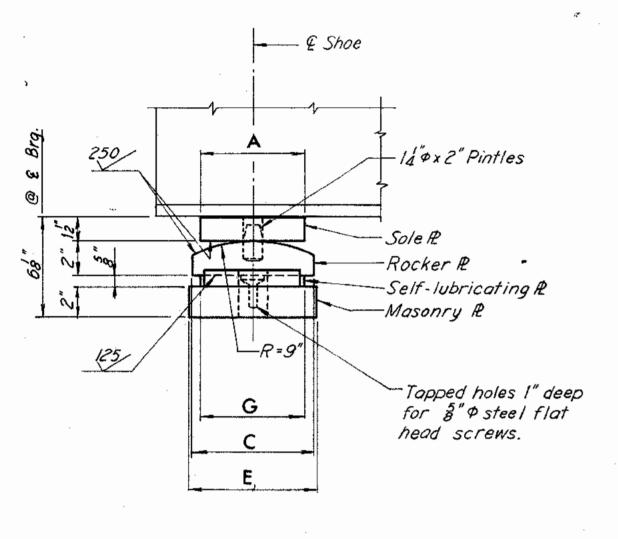
RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

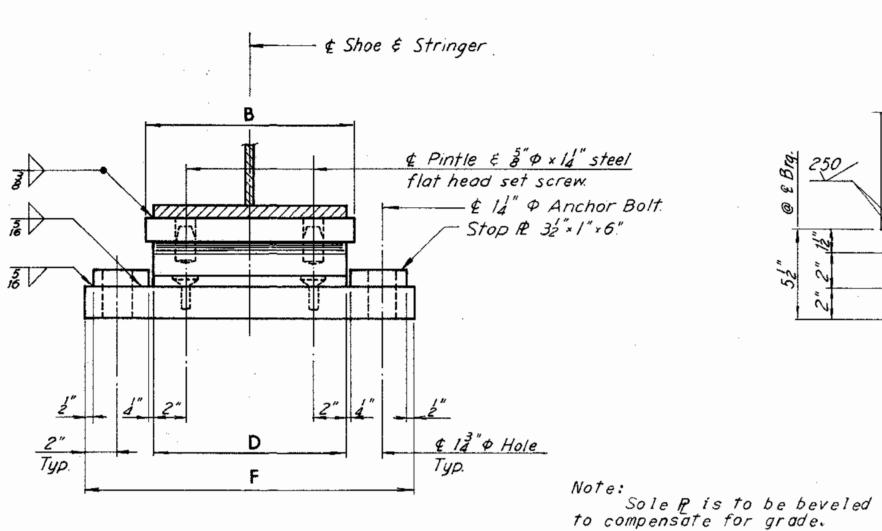
**BELTLINE EXPRESSWAY** 

BRIDGE NO 13 DOUGLASDALE ROAD OVER
BELTLINE EXPRESSWAY AND R.F.&P.R.R.

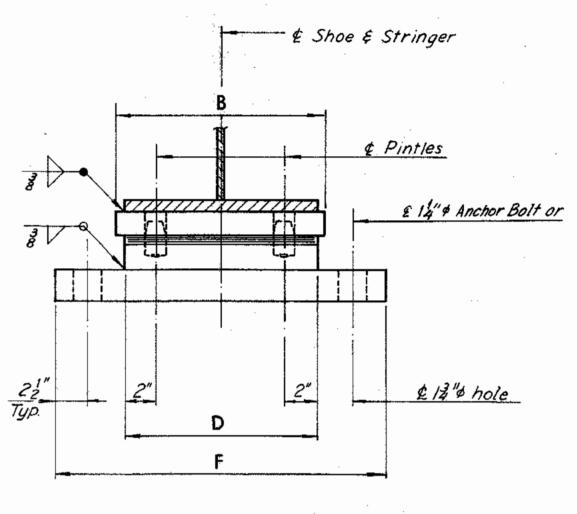
FRAMING DETAILS

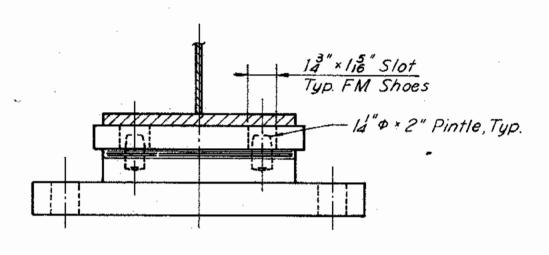
SCALE: As Noted HOWARD, NEEDLES, TAMMEN & BERGENDOFF CONTRACT NO.: \_\_\_ consulting engineers KANSAS CITY SHEET NO. <u>8</u> OF <u>12</u>





— & Shoe -Rocker A -Masonry R





Note: Fixed Modified Shoes same as Fixed Shoes except as shown.

SIDE ELEVATION

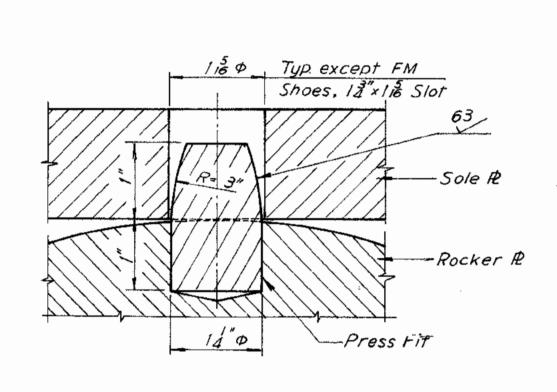
END ELEVATION

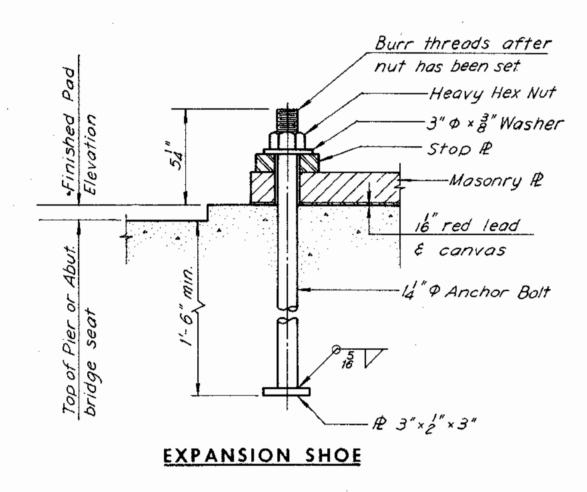
SIDE ELEVATION

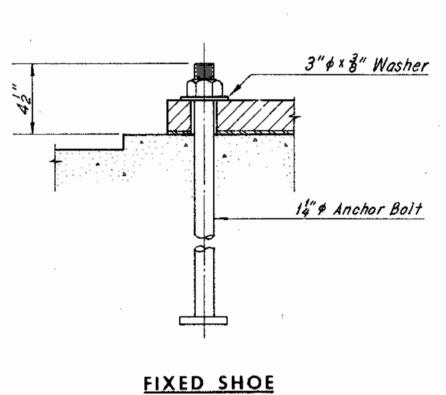
END ELEVATION

END ELEVATION

**EXPANSION SHOE** 







### PINTLE DETAIL

ANCHOR BOLT DETAIL

Anchor Bolt for Fixed Shoes same as Anchor Bolt for Exp. Shoes except as shown.

**FIXED SHOE** 

FIXED MODIFIED SHOE

Shoe Notes:

Top of masonry plates, bottom of rocker plates and top and bottom of sole plates shall be planed, straightened or otherwise treated to secure true plane surfaces. Contact surfaces noted on the plans with finish symbols shall be finished in accordance with the American Standards Association surface roughness requirement as defined in ASA 845.1, Surface Roughness, Waviness and Lay, Part I.

The plates comprising the expansion shoes shall be set so as to be truly centered under full dead load at a temperature of 68°F.

Concrete pads shall be formed integral with abutment or pier and not less than "" or more than 4" above finished elevation. Dress down pads by rubbing, grinding or as otherwise approved by the Engineer, to true level surfaces at the finished elevation. Anchor bolt assemblies shall conform to A.S.T.M. A-307 and shall be hot dipped galvanized, conforming to A.S.T.M. A-153.

Templates shall be used to accurately set the anchor bolts.

Material for shoes lexibilities of self-lubrication plates) shall be high strength law allow structural steel conforming to

Material for shoes (exclusive of self-lubricating plates) shall be high strength low alloy structural steel conforming to

Material for self-lubricating plates shall be Leaded Tin Bronze conforming to A.S.T.M. specification B22, alloy D modified to the extent that 1.5 to 2.5 percent lead is allowable.

Shoes shall be included with structural steel item for payment.

SHOE DIMENSIONS																
EXPANSION SHOES	Α	В	С	D	E	F	G	NUMBER REQUIRED	FIXED SHOES	Α	В	С	D	E	F	NUMBER
E1	6"	1'- 1"	7/11	1'- 0"	8"	1'-8!"	6"	61	F1	6"	1'-1"	6"	1'-0"	72!11	11-9"	50
E2	6"	1'- 1"	8/11	1'- 0"	9"	1'-82"	62 "	4	F1M	6"	1'-1"	6"	1'-0"	72"	11-9"	15
				İ												

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM BELTLINE EXPRESSWAY

> STANDARD SHOE DETAILS Bridges 13, 17

HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers NEW YORK ALEXANDRIA KANSAS CITY

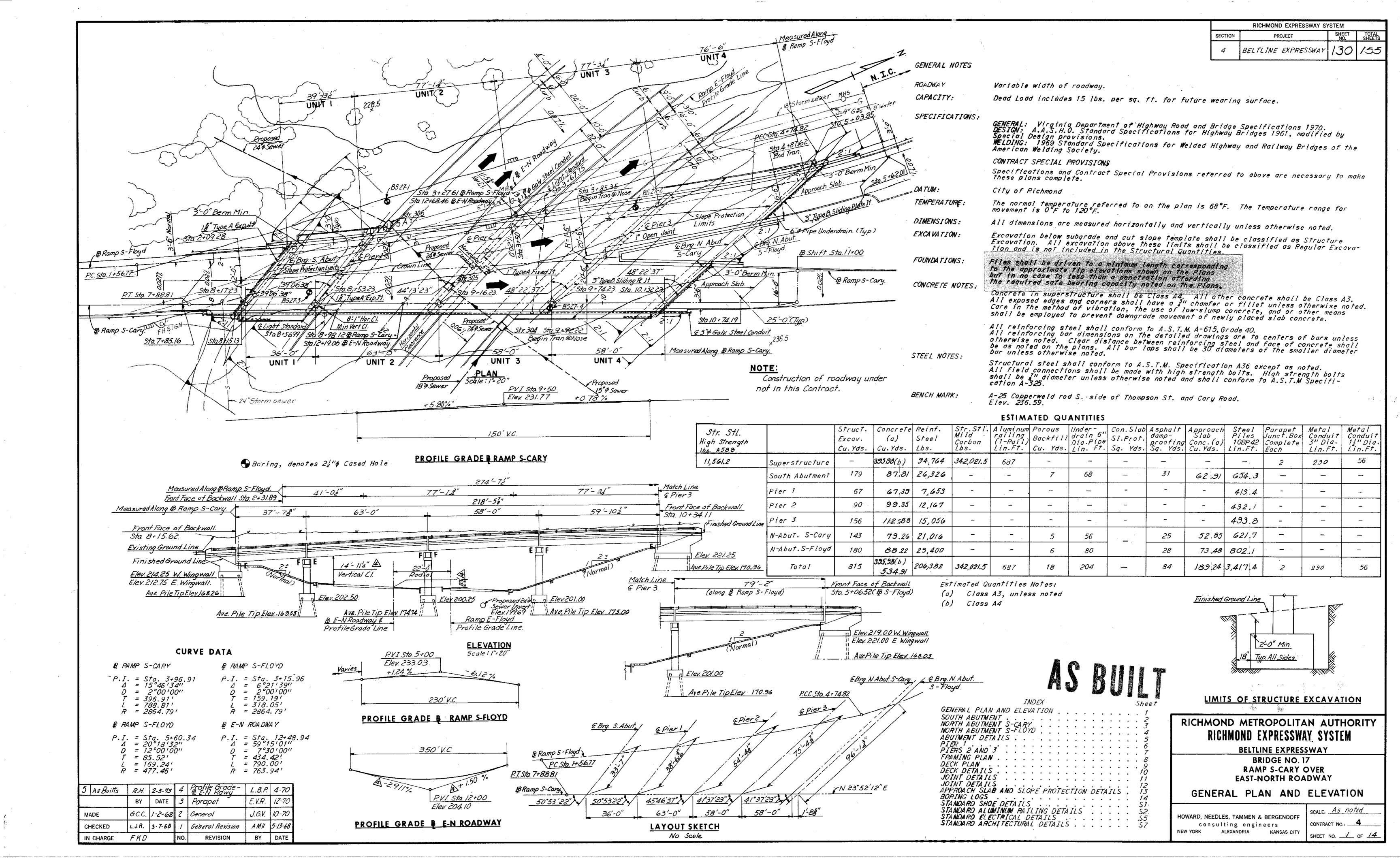
MHH 2-1-68 J.V. 2-6-68 J.G.V. 10-70 General BY DATE IN CHARGE FKD

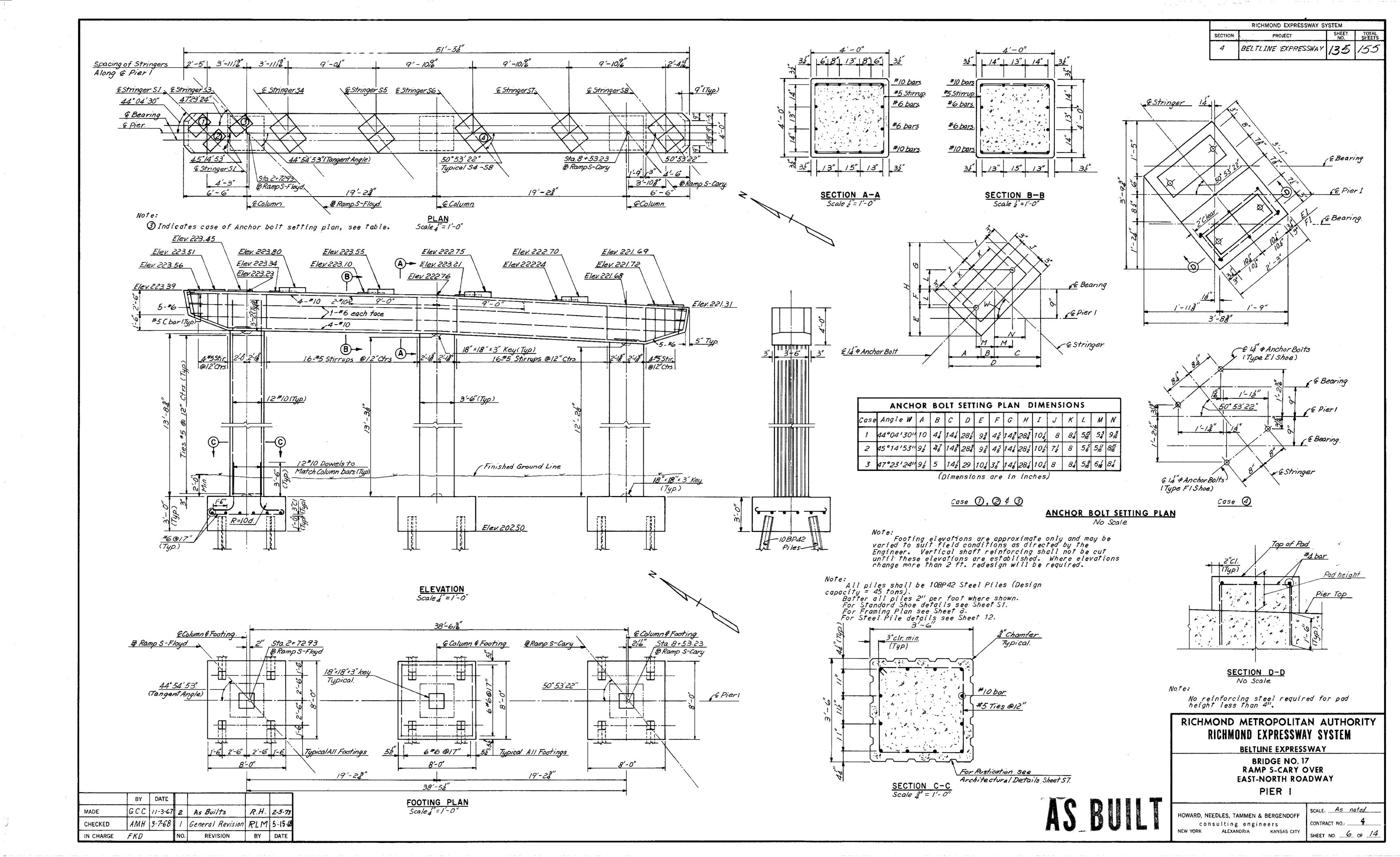
CONTRACT NO.: SHEET NO. 51

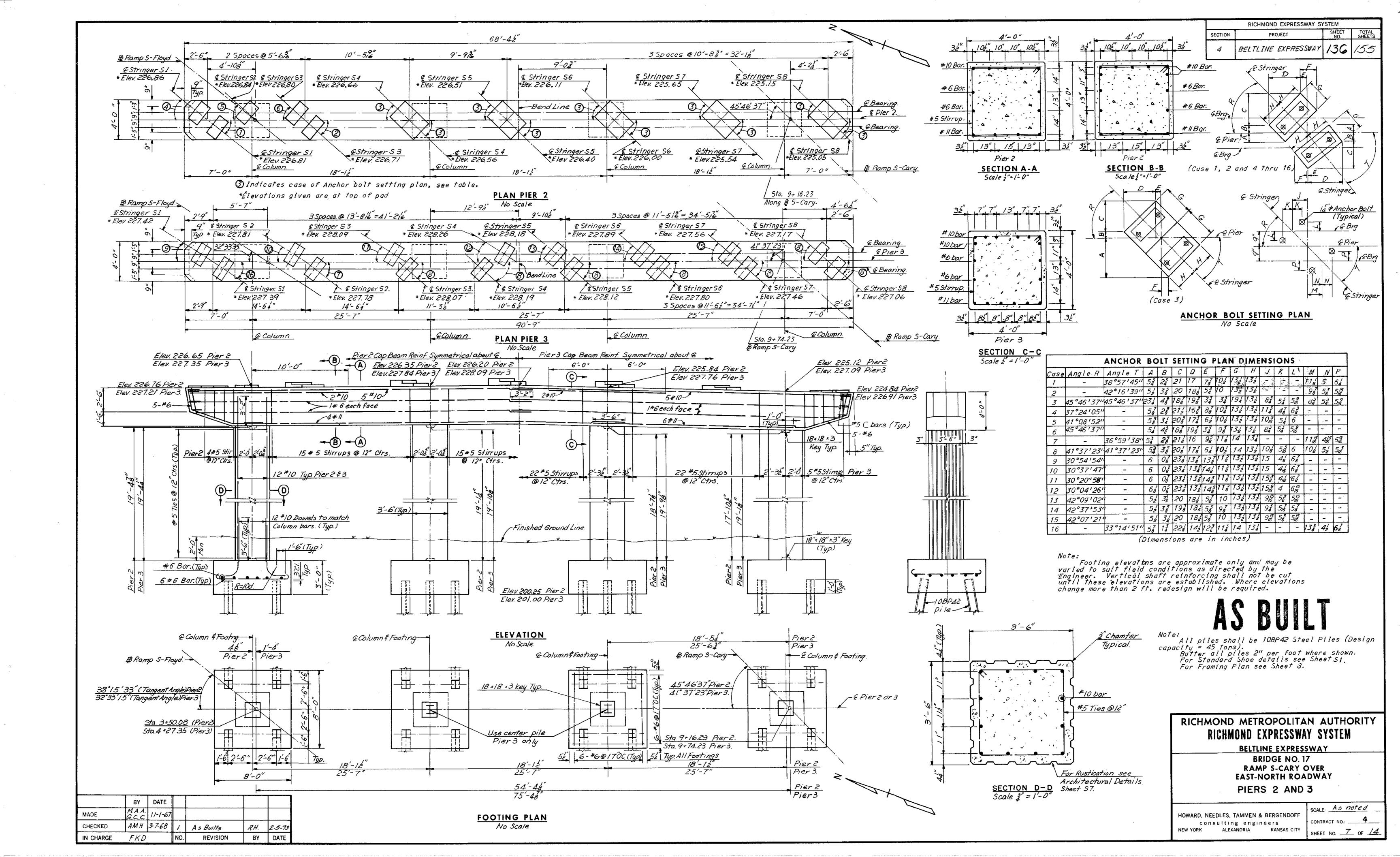
SCALE: No Scale

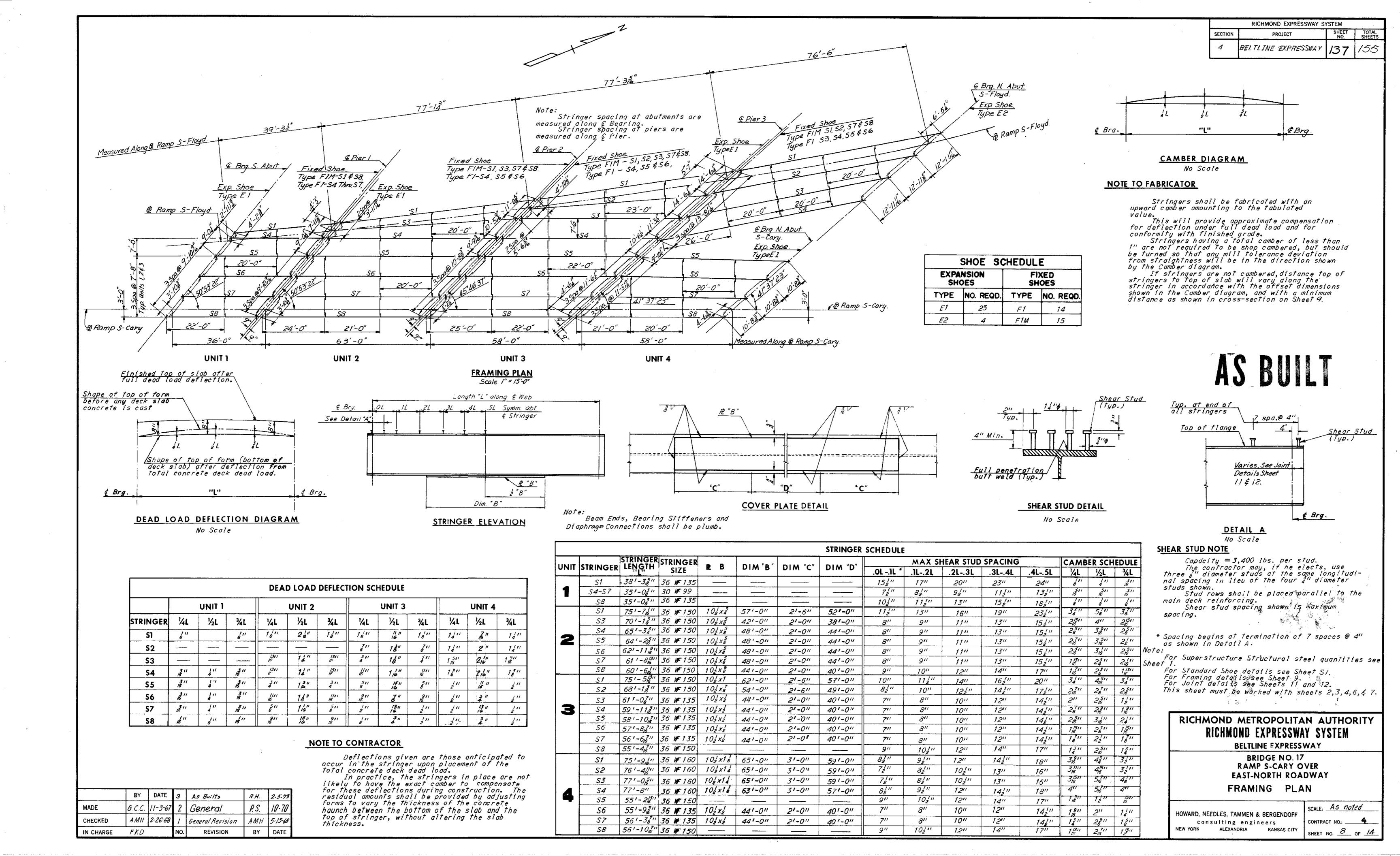
Northbound Powhite Parkway (VA-76) Ramp to Floyd Ave. and Cary Street Over

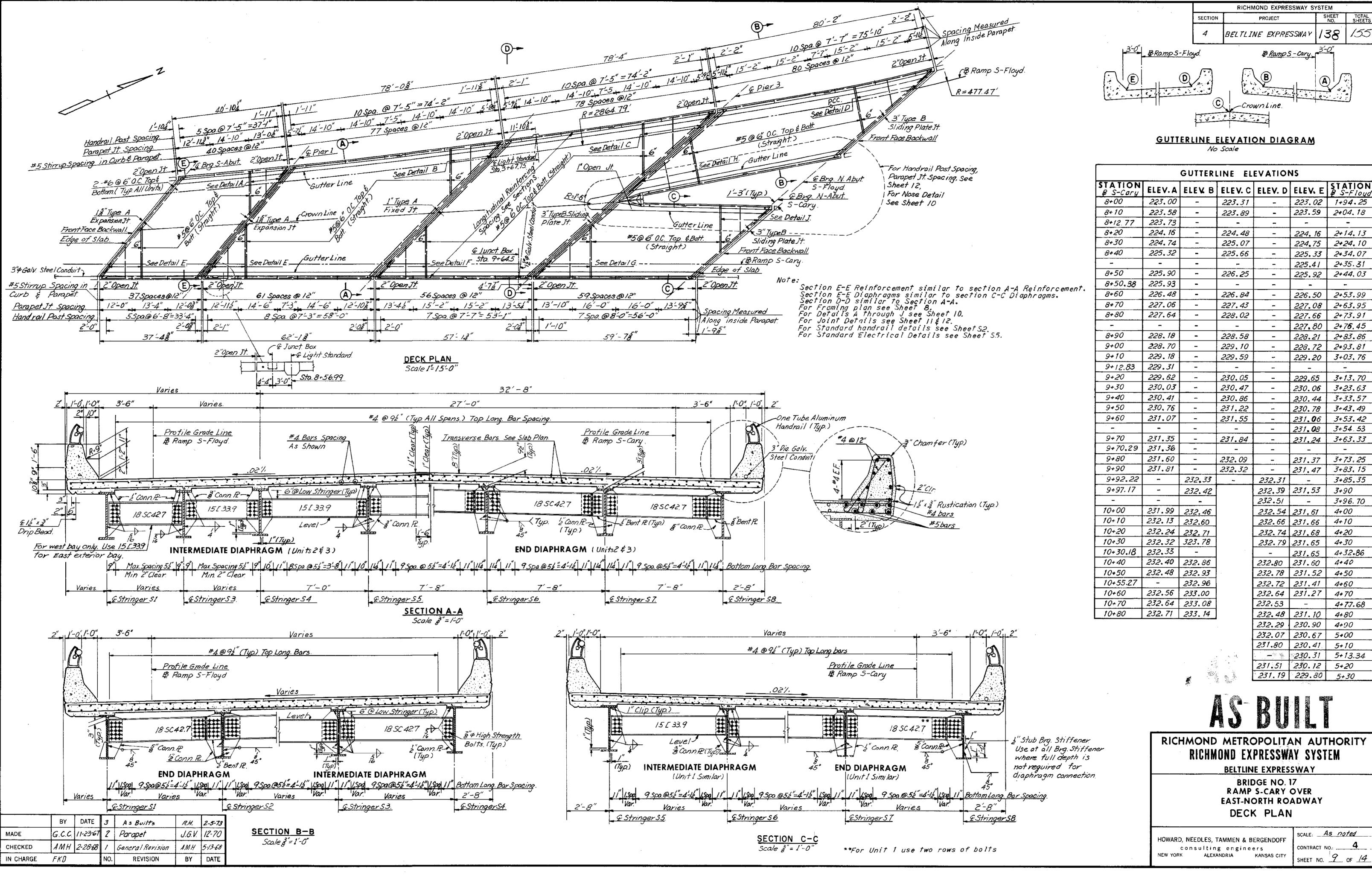
Northbound I-195 and the Ramp from Northbound I-195 to Floyd Avenue

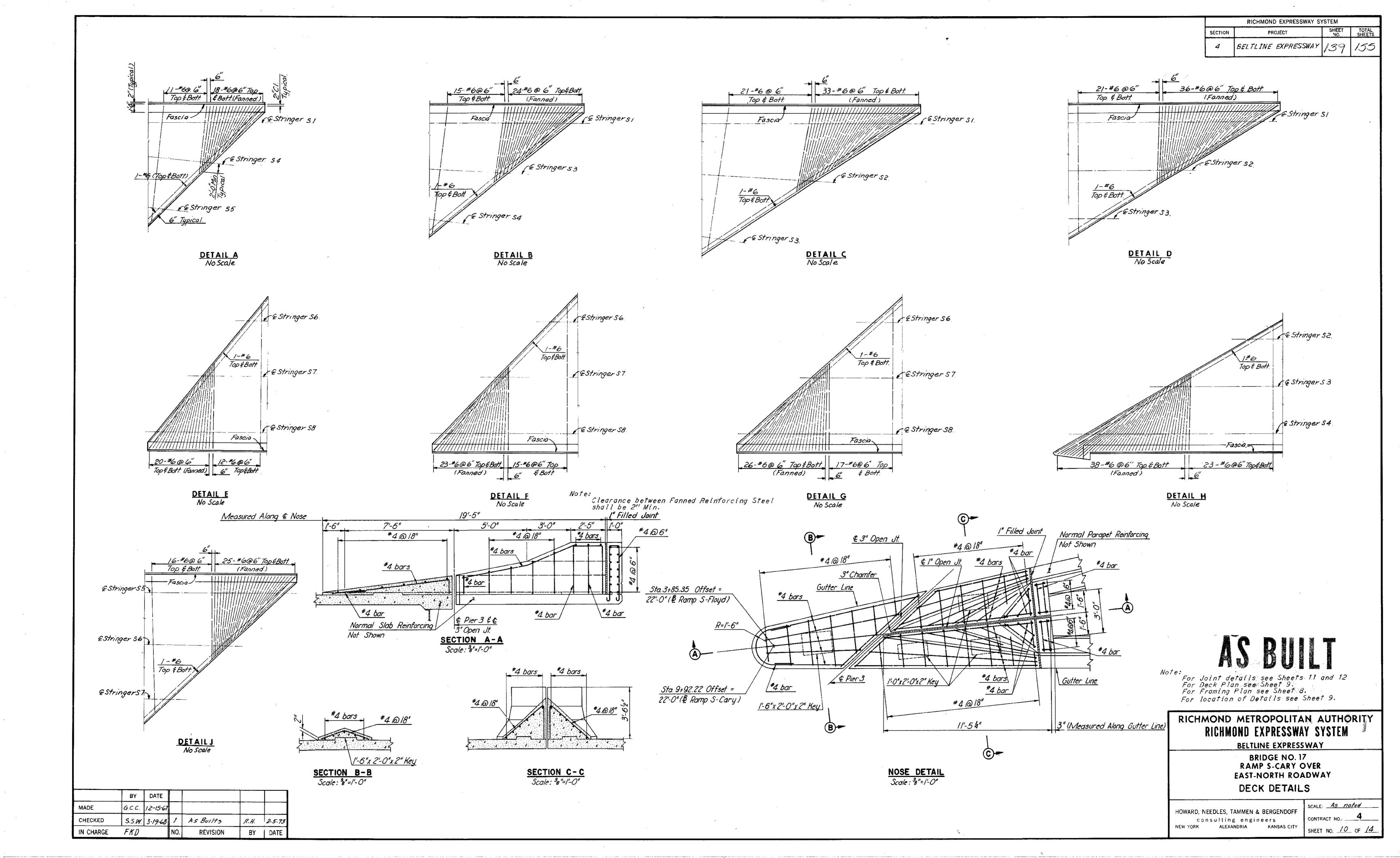




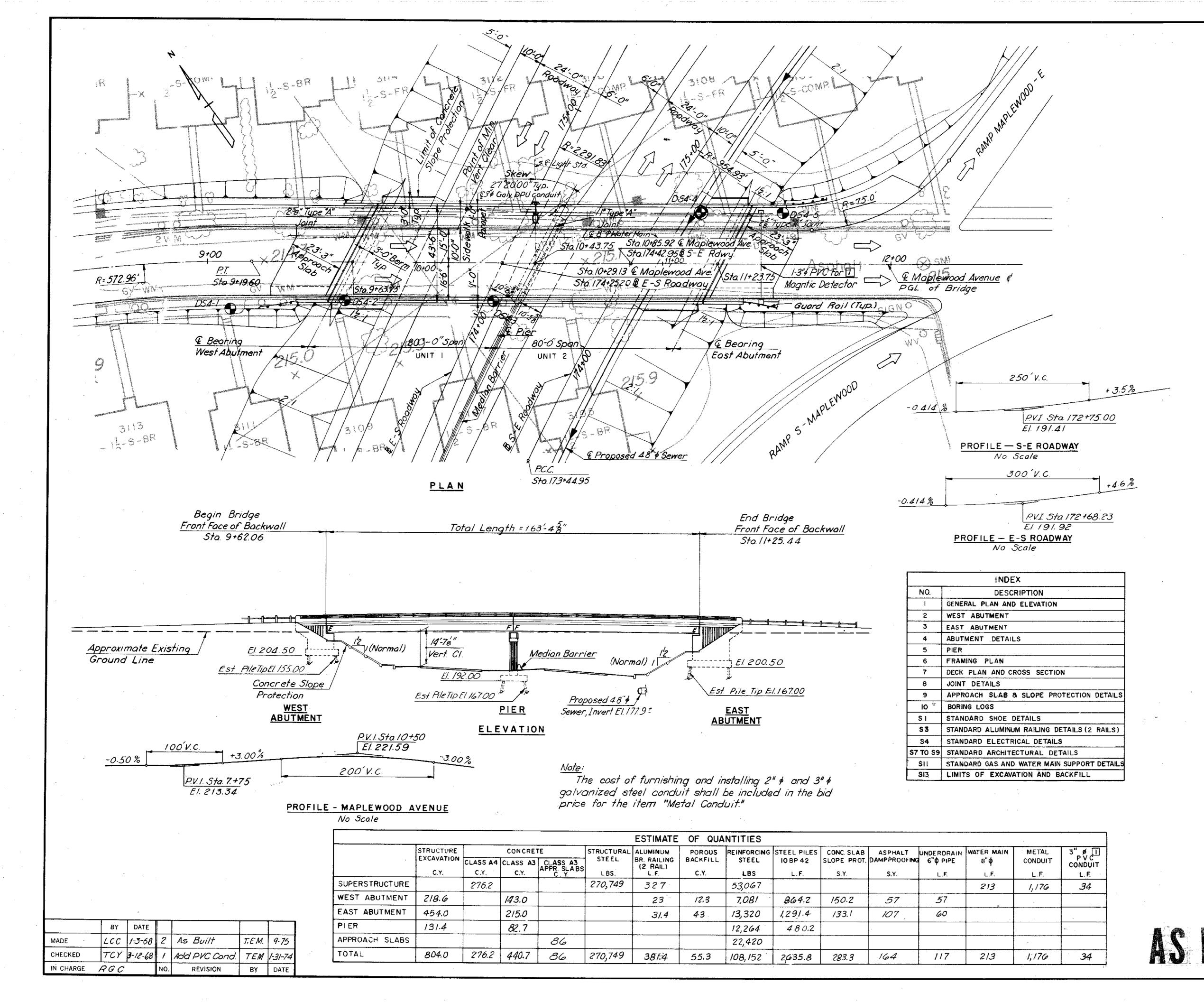








Maplewood Avenue
Over
The Connector to Downtown Expressway (VA 146)



RICHMOND EXPRESSWAY SYSTEM PROJECT 96 DOWNTOWN EXPRESSWAY

GENERAL NOTES

ROADWAY:

36-1" Face of rail to edge of sidewalk.

CAPACITY:

Dead Load-Includes 15 lbs. per sq. ft. for future wearing surface. Live Loads-HS20-44 loading and B.P.R. modified for military vehicles.

SPECIFICATIONS:

GENERAL-Virginia Department of Highway Road and
Bridge Specifications, 1970.

DESIGN-A.A.S.H.O. Standard Specifications for
Highway Bridges, 1969 and 1970-72 Interim Specifications, modified by Special Design Provisions.

WELDING 1972 Structural Welding Code of the American Welding Society.

CONTRACT SPECIAL PROVISIONS

Specifications and Contract Special Provisions referred to above are necessary to make these plans complete.

DATUM: CITY OF RICHMOND

EXCAVATION:

The normal temperature referred to on the plan is 60° F. The temperature range for movement is 0° F, to 120° F.

DIMENSIONS:

All dimensions are measured horizontally and vertically unless otherwise noted.

Excavation below subgrade and cut slope template shall be classified as Structure Excavation.

All excavation above these limits shall be classified as Regular Excavation and is not included in the Structural Quantities.

FOUNDATIONS: Footings shall rest on firm material. Found. mat'l. shall be kept dry & special attn. is called to Sec. 401.05 of the Gen. Specs. & to the Contract Special Prov. concerning preparation of found. for ftgs.

CONCRETE NOTES:

Concrete in superstructure shall be Class A4.
All other concrete shall be Class A3. All exposed edges and corners shall have a 3" chamfer or fillet unless otherwise noted. Care in the method of vibration, the use of low-slump concrete, and/or other means shall be employed to prevent downgrade movement of newly placed slab concrete. (When gradient is over 2%).

Finishing concrete surfaces: See the Standard Architectural Detail Sheets and the Contract Special Provisions for types and details.

All reinforcing steel shall be intermediate grade. All reinforcing bar dimensions on the detailed drawings are to centers of bars unless otherwise noted. Clear distance between reinforcing steel and face of concrete shall be as noted on the plans. All bar laps shall be 30 diameters of the smaller diameter bar unless otherwise noted.

STEEL NOTES: Structural steel shall conform to A.S.T.M. Specification A36 except as noted.

All Field connections shall be made with high strength bolts. High strength bolts shall be "diameter unless otherwise noted and shall conform to A.S.T.M. Specification A-325.

C-11. Copper Weld Rod, South side of Maplewood Ave. at Belmont Ave. Elev. 215.37.

Indicates 22" \$ cased hole boring. Indicates 4" & cased hole boring.

#### RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM DOWNTOWN EXPRESSWAY

BRIDGE NO. 36

MAPLEWOOD AVENUE OVER DOWNTOWN EXPRESSWAY GENERAL PLAN AND ELEVATION

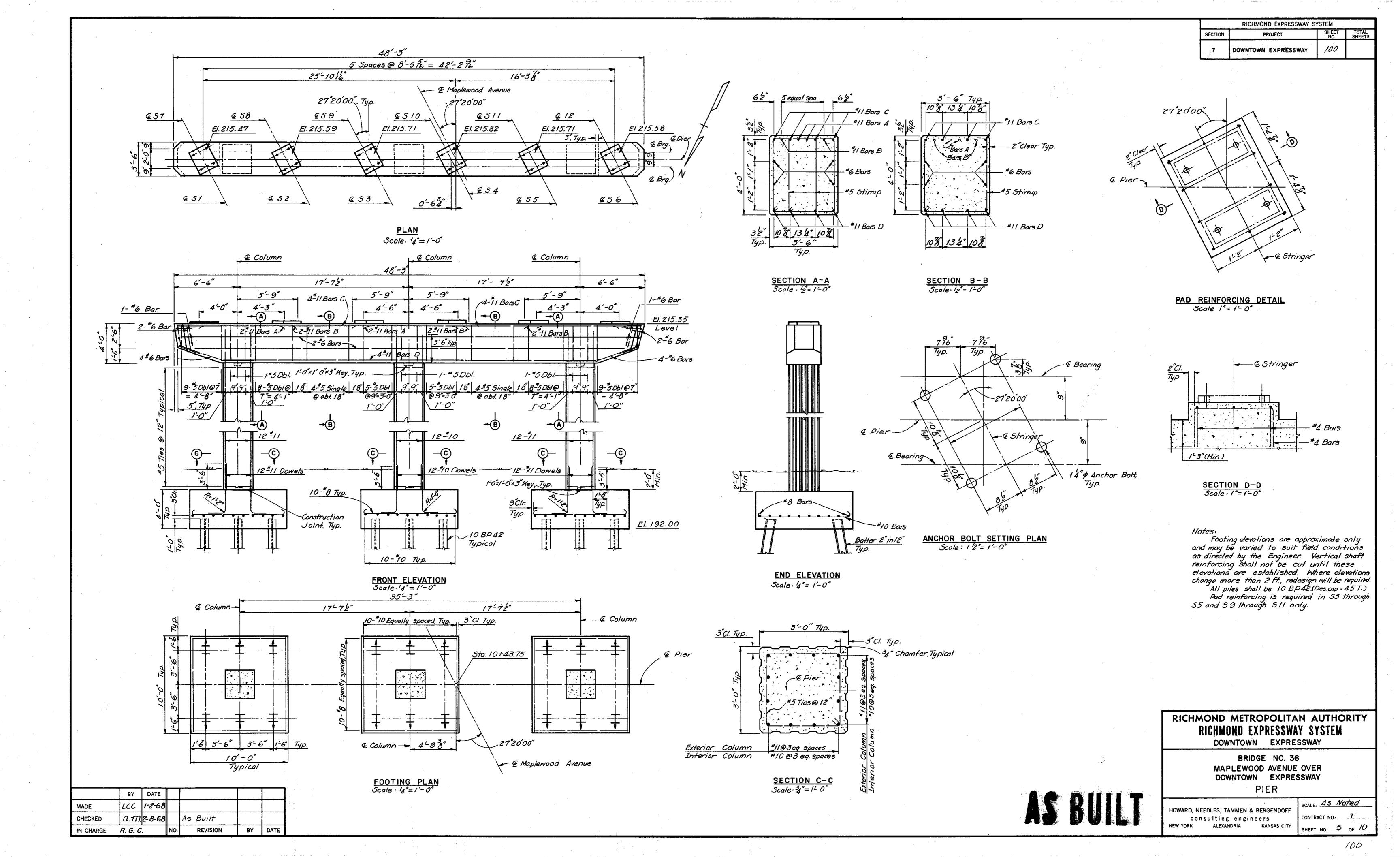
consulting engineers

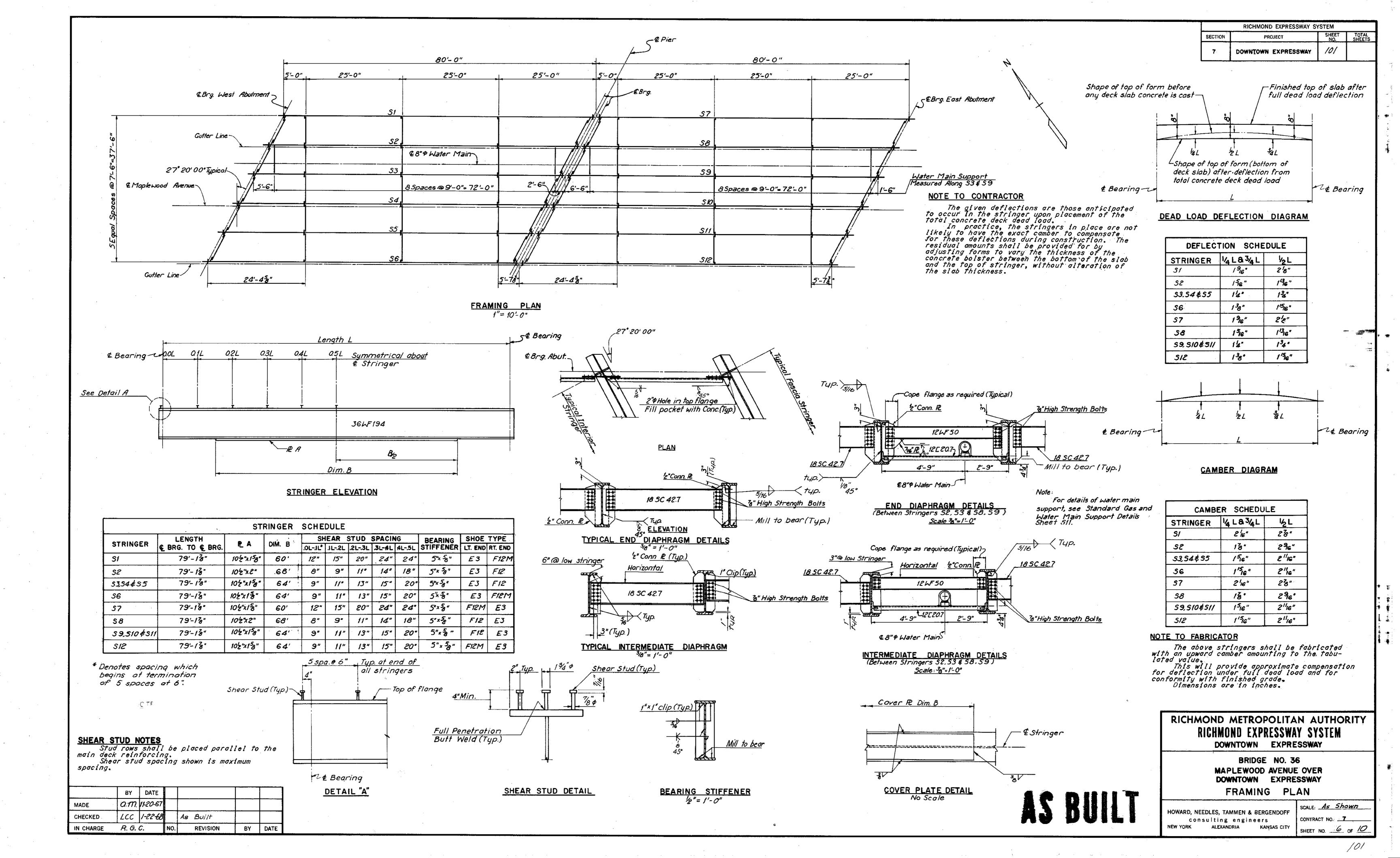
HOWARD, NEEDLES, TAMMEN & BERGENDOFF NEW YORK ALEXANDRIA KANSAS CITY

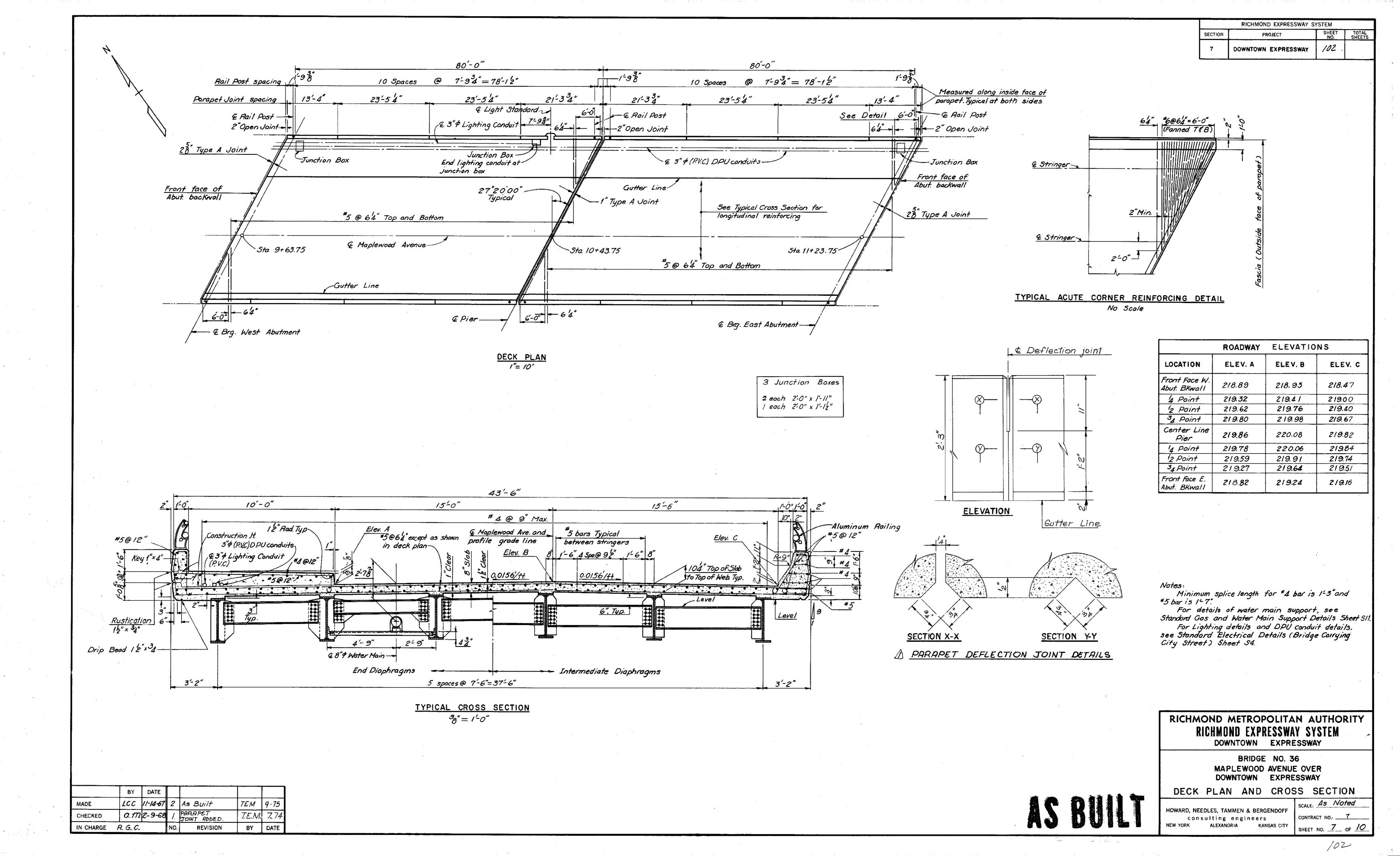
96

SCALE: I"= 20 UNLESS NOTE!

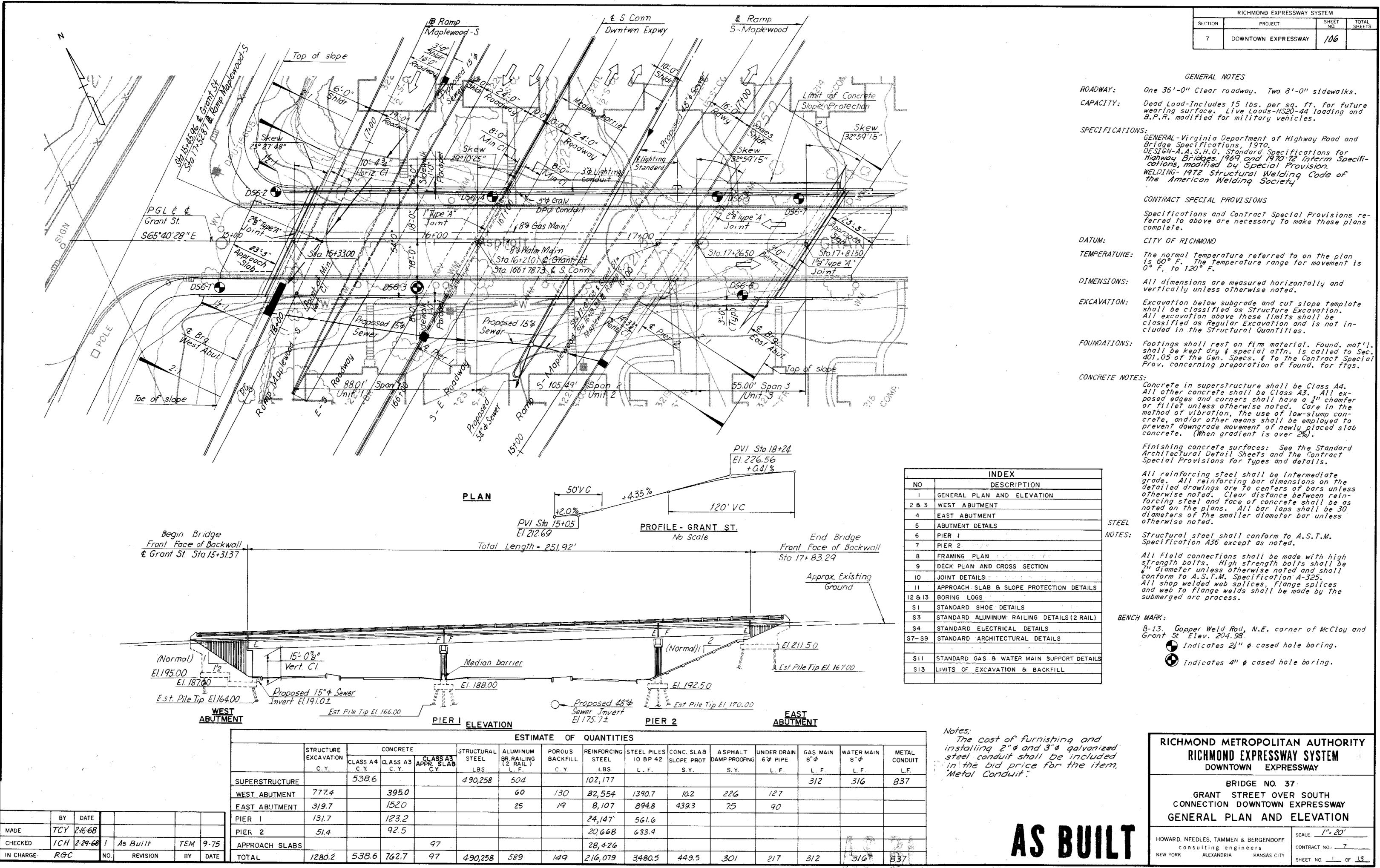
CONTRACT NO.: \_\_\_\_\_7

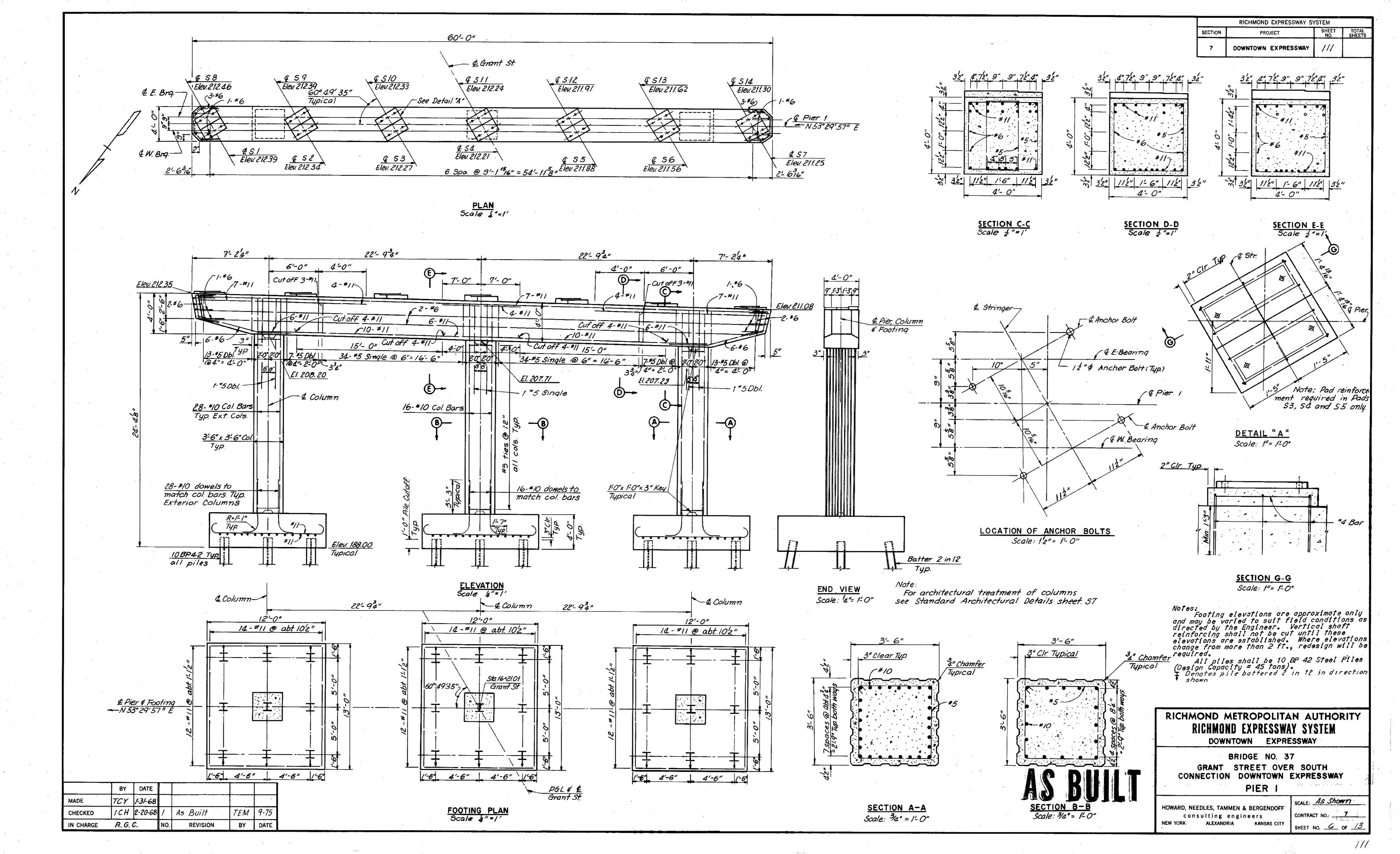


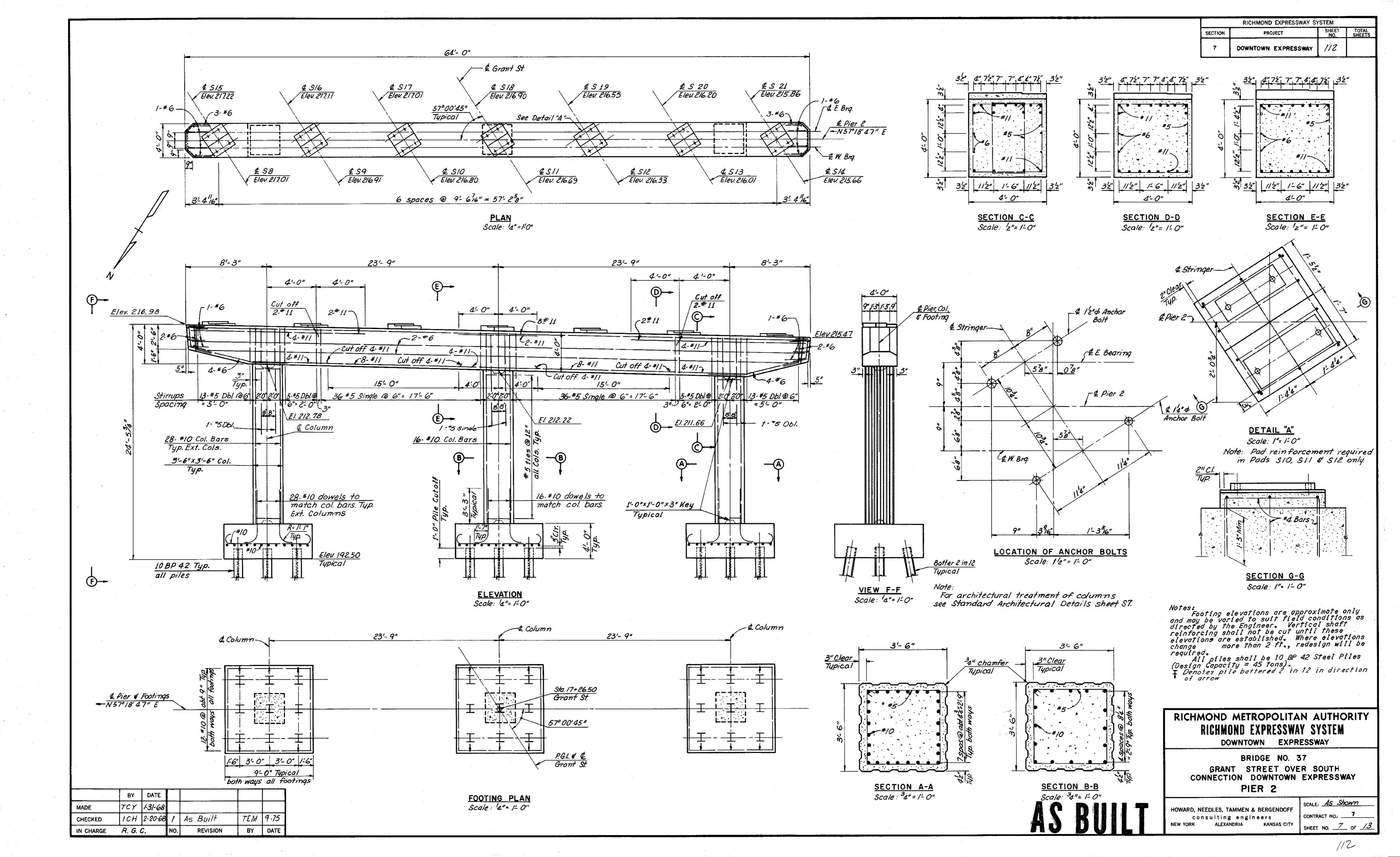


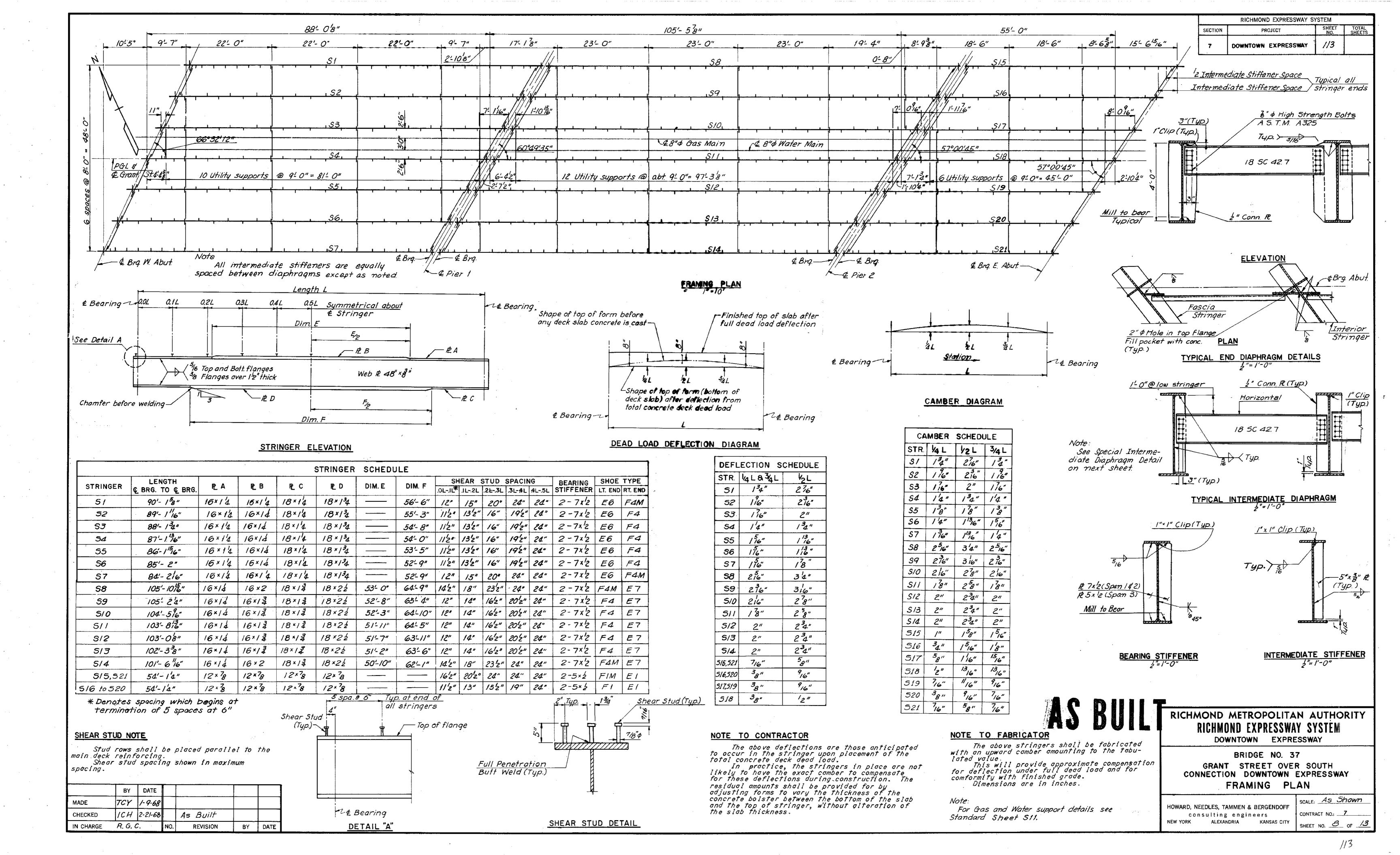


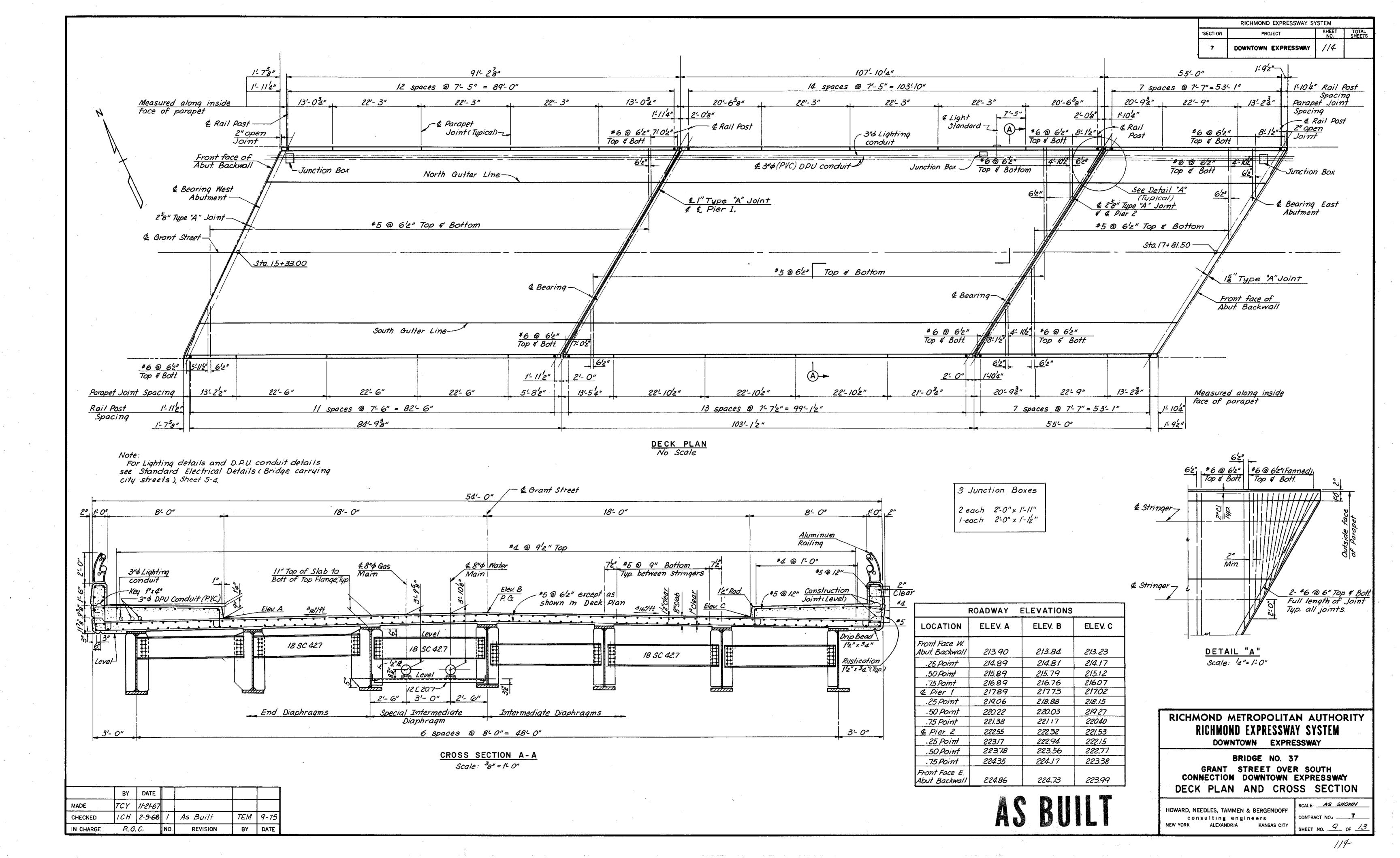
Grant Street
Over
The Connector to Downtown Expressway (VA 146)







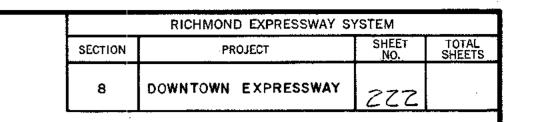




# Bridge 50

South Laurel Street
Over
Downtown Expressway (VA 195)

**Record Set Plans** 



**GENERAL NOTES:** 

ROADWAY:

One 30'0" Clear roadway. Two 8'-0" sidewalks.

Live Loads - H520-44 loading and B.P.R. modified for

military vehicles.

SPECIFICATIONS:

GENERAL-Virginia Department of Highway Road and Bridge

Specifications 1970

CONTRACT SPECIAL PROVISIONS:

Specifications and Contract Special Provisions referred to above are necessary to make these plans complete.

TEMPERATURE : The normal temperature referred to on the plan is 60 °F.

DIMENSIONS: All dimensions are measured horizontally and vertically

EXCAVATION: Excavation below subgrade and cut slope template shall be classified as Structure Excavation. All excavation

Excavation and is not included in the Structural Quantities.

FOUNDATIONS: Footings shall rest on firm material Foundation material shall be kept dry and special attention is called to Section 401.05 of the General Specifications, and to the Contract Special Provisions, concerning preparation of foundations

for footings.

Concrete in superstructure shall be Class A4. All other concrete shall be Class A3. All exposed edges and noted. Care in the method of vibration, the use of lowprevent downgrode movement of newly placed slab

All reinforcing steel shall conform to ASTM A615 Grade 40 All to centers of bars unless otherwise noted. Clear distance between reinforcing steel and face of concrete shall be as noted on the plans. All bar laps shall be 30 diameters

STEEL NOTES: Structural steel shall conform to ASTM. Specification A36 except as noted.

All field connections shall be made with high strength otherwise noted and shall conform to A.S.T.M. Specification

BENCH MARK: C-38. Monument located in walk N.E. Corner Cumberland and

CAPACITY:

Dead Load - Includes 15 lbs. per sq. ft. for future wearing

DESIGN -AASHO. Standard Specifications for Highway Bridges, 1961 modified by Special Design Provisions. WELDING - 1969 Standard Specifications for welded Highway and Railway Bridges of the American Welding Society.

CITY OF RICHMOND DATUM:

The temperature range for movement is 0°F, to 120°F.

unless otherwise noted.

above these limits shall be classified as Regular

CONCRETE NOTES

corners shall have a ? " chamfer or fillet unless otherwise slump concrete and for other means shall be employed to concrete (When gradient is over 2%) Finishing concrete surfaces : See the Standard Architectural Detail Sheets and the Contract Special Provisions for types

and details.

reinforcing bar dimensions on the detailed drawings are of the smaller diameter bar unless otherwise noted.

bolts. High strength bolts shall be 3" diameter unless

L.F.

420

South Laurel. Elev. 176.83.

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM DOWNTOWN EXPRESSWAY

> STRUCTURE B 50 LAUREL STREET OVER DOWNTOWN EXPRESSWAY

GENERAL PLAN AND ELEVATION SCALE: | "= 20" AMERICAN ENGINEERS

Richmond, Virginia CONTRACT NO.: 8 HOWARD, NEEDLES, TAMMEN & BERGENDOFF SHEET NO. 1 OF 9 General Consultants

INDEX DESCRIPTION General Plan and Elevation South Abutment 3 North Abutment 4 Pier Details 5 Framing Plan 6 Cross Section & Utility Details 1 Deck Plan & Joint Details 8 Approach Slabs 9 Boring Logs 5/ Standard Shoe Details 53 Standard Aluminum Railing Details 54 Stondard Electrical Details 51/58 Standard Architectural Details SID Standard Elect and Tele. Cond. Details SII Standard Utility Support details @ Abut.

Top of Pavement Elevations at ends of Deck along P.G.L. are given on Plan, Remaining Pavement Elevations are given on Sheet 7. € Indicates 2 % Cased Hole Boring. 1 Indicates 4 0 Cased Hole Boring

ESTIMATED QUANTITIES ASPHALT GINCH
DAMP- PIPE
PROOFING UNDERDRAIN
S.Y. L.F. POROUS CONC. SLAB GAS MAIN GAS MAIN SLOPE PROTECTION 12" \$ 8" \$ C.Y. S.Y. L.F. L.F. CONDUITS CONDUITS 3" PVC \_ 2" METAL Pedestrian 6" PVEPCO 4" FEL. CONDUIT CONDUIT Screen STRUCTURE CONCRETE CONCRETE REINFORCING STRUCTURAL ALUMINUM CLASS CLASS STEEL-A36 BRIDGE RAILING STEEL EXCAVATION CY LBS. LBS. C.Y. L.F. .960.5 382,078.4 208 481 1442 Superstructure 364.21 68,195 408 226 2/73 134 4,873 232.6 43 57 14 South Abutment 88.24 92.65 16,300 241 Pier 1 272 97.54 19,411 Pier 2 268:12 242.29 12,341 North Abutment 55 72 119 76.60 17,263 Approach Slabs 1442 960.5 4 364.21 226 217.3 208 Total 9/5/2 597.32 /38,383 382,078.4 408 162 232.6 481

DATE BY W.D.U. 8-67 MADE W.E.O. 10-67 HMW. 4-76 AS BUILT CHECKED BY DATE W.E.O. REVISION IN CHARGE

LAUREL STREET

PROFILE DATA

Front Face of Backwall

2-5-BR

Sta. 12+26.08

Elev. 172.96

Approach

Slab

Typical)

Beginning of Bridge at Front Face of Backwall

P.V.I.: Sta. 272+00

DOWNTOWN EXPRESSWAY

Elev. 157.32

V.C. 300'

Sta. 12+26.08

Cast in Place

Slope Protection

PIER I

SOUTH ABUTMENT

Concrete Slab

Slope Protection

South Abutment

2-5-88

12" Water Main

858

Sta. 12+01.08

Wood Pole

€ 3"+ Conduit for Bridge Lighting 221

& 12" & GOS Moin,

☑ Indicates Junction Box

By Others)

£86695 Main

Finished Grade

\_\_-0.64%

(Capped)

Elev. 172.55

3249"

90° Typ. @

Piers & Abuts

\_5 ! 0" Toe of Slope

74'-0"

36'-0" Roadway

& Pier 2

22'0"

Min. Horiz.

22'0"

18 Exp. Ut.

Elex 174.70

& Light Post

Light Post

14'-78" Min. Vert. Clearance

Varies

& Pier 1

Shidi:

5ta. 12+58.83

Elev 173.49

I" Fixed Jt.

Min. Horiz.

Clearance

Pedestrian Screen-

42" Storm Sewer

PLAN

P.G.L.

42" b Storm Sewer with

(See Roadway Plans)

Conc. Encasement.

/9343"

Exp. III Fix.

PIER 2

**ELEVATION** 

P.G.L.

& Downtown Expressway

86'-6"

48'-0" Raadway

Point of Min.

Sta. 13+32.83 - Laurel Street Sta. 272+70.75-Downtown Expwy.

Vertical Clear

14-0" Min Horis 4 5

Sta. 14+19.33

Elev. 176.11

Clearance

& Downtown Expressway

Front Face of Backwall

Retaining Wall W-810

& Brgs.

Approach

Retaining Wall W811

End of Bridge at

Sta. 14+19.33

NORTH ABUTMENT

Front Face of Backwall

Approximate Existing

Ground Along P.G.L.

£4-3" \* Electrical Conduits

£ and P.G.L. Lourel St.

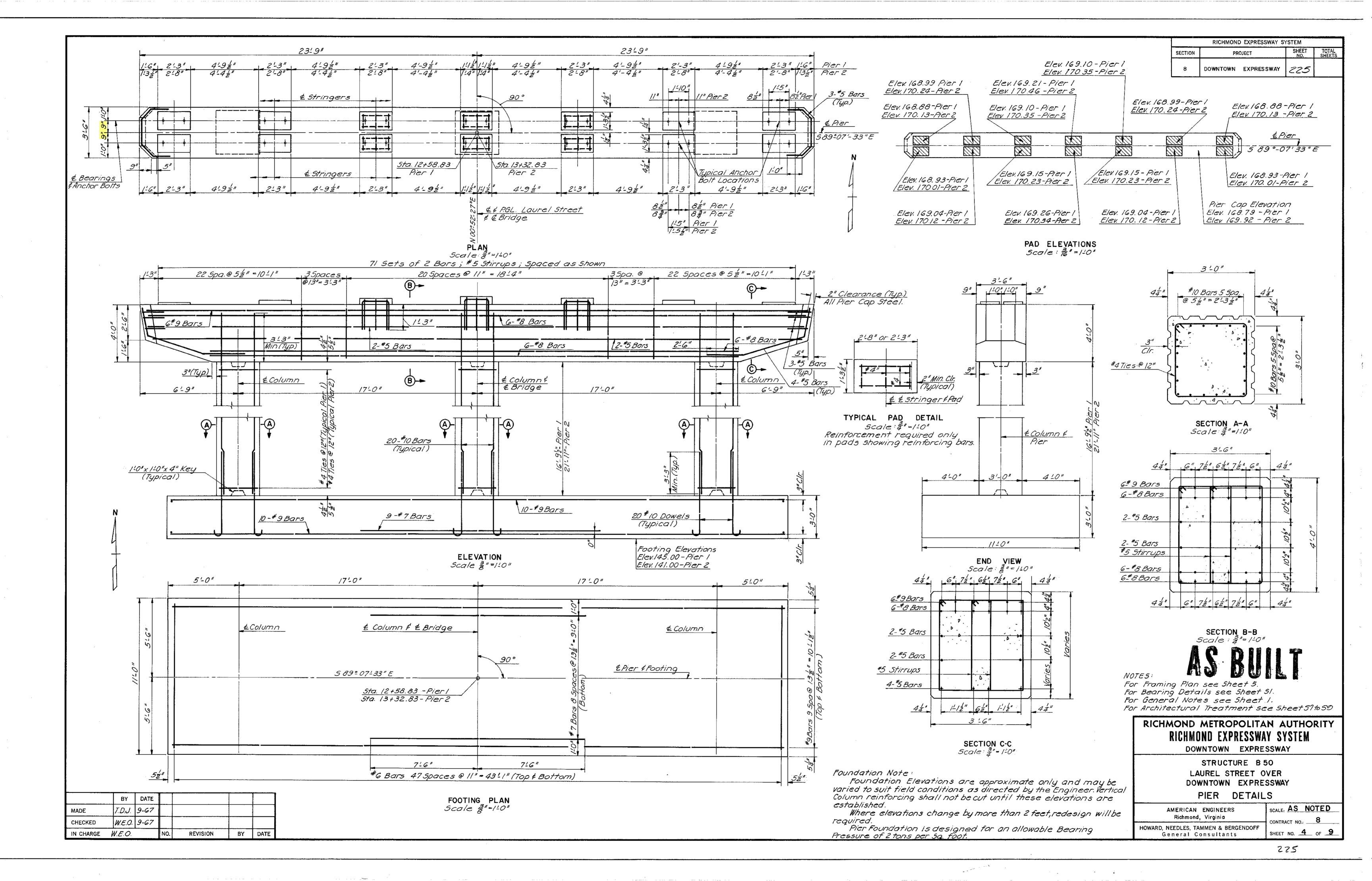
\$ 6-4" & Telephone Conduits

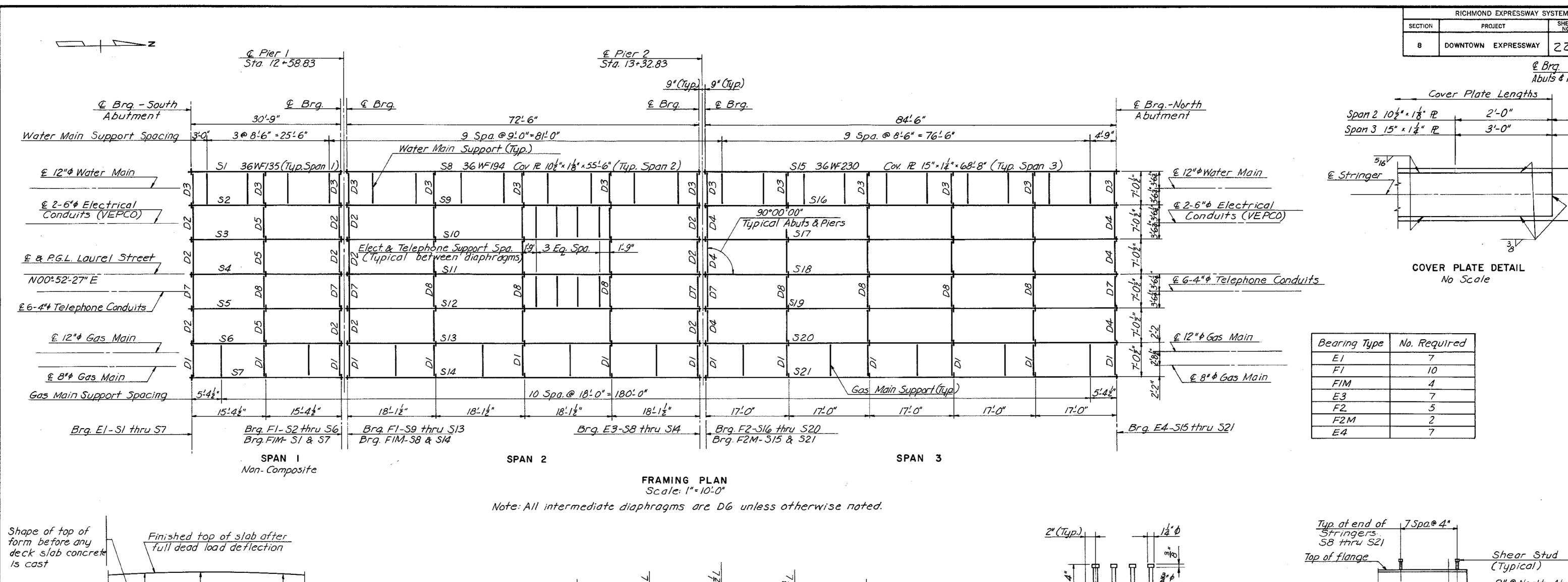
Sta. 14+39.33

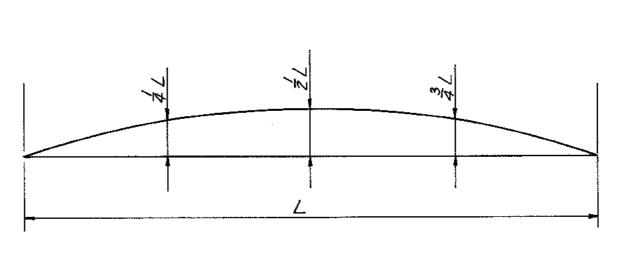
Elev. 176.43

N 003 52 27"E

North Abutment







CAMBER DIAGRAM

S9 thru S/3 176" S16 thru S20 1措\*

### Stringers SI thru S7 58, 514 59 thru S13 \$15, \$21 \$16 thru 520

.25L

.25L

DEAD LOAD DEFLECTION DIAGRAM

NOTE TO CONTRACTOR:

£ Brg.

The above deflections are those anticipated to occur in the stringer upon placement of the total concrete deck dead load. In practice the stringers in place are not likely to have the exact camber to compensate for these deflections during construction. The residual amounts shall be provided for by adjusting forms to vary the thickness of the concrete bolster between the bottom of the slab and the top of the stringer, without alteration of the slab thickness.

DATE

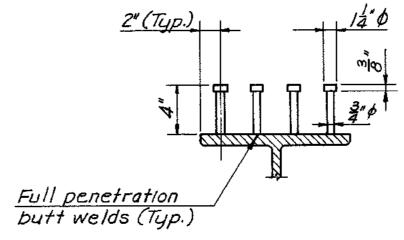
Shape of top of form (bottom of deck slab) after deflection from total concrete deck

dead load.

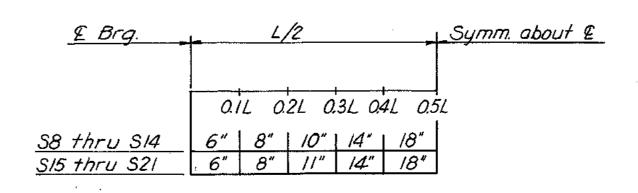
<u>E Brg.</u>

### NOTE TO FABRICATOR:

The above stringers shall be fabricated with an upward comber amounting to (see table). This will provide approximate compensation for deflection under full dead load and for conformity with finished grade. Stringers which are not required to be shop cambered shall be turned so that any mill tolerance deviation from straightness will be in the direction shown by the camber diagram above.



SHEAR STUD DETAIL Scale: 12"=1-0"

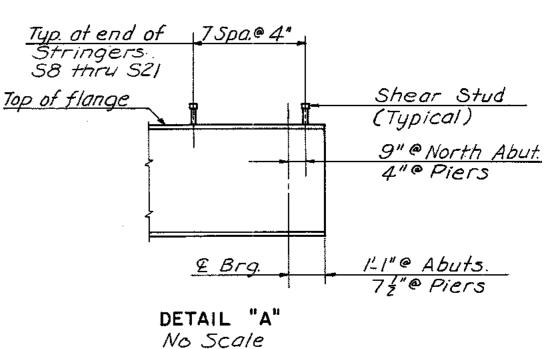


### SHEAR STUD SPACING

No Scale

Shear Stud spacing shown is maximum

Shear Stud Notes: Capacity = 3400 lbs. per stud. Three & diameter study may be substituted at the same longitudinal spacing as shown for four 3" diameter studs. Stud rows to be placed perpendicular to & stringer. For end condition see Detail "A".



PROJECT

2'-0"

3'-0"

£ Brg. Abuts & Piers

8-6"

7-//"

Structural steel shall conform to ASTM Specifications A36 (latest revision). For General Notes see Sheet 1.

For Superstructure Cross Section see Sheet 6.

For Diaphragm Details see Sheet 6. For Utility Support Details see Sheet 6.

For Bearing Shoe Dimensions see Standard Sheet SI.

# RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

STRUCTURE B 50

DOWNTOWN EXPRESSWAY

LAUREL STREET OVER DOWNTOWN EXPRESSWAY

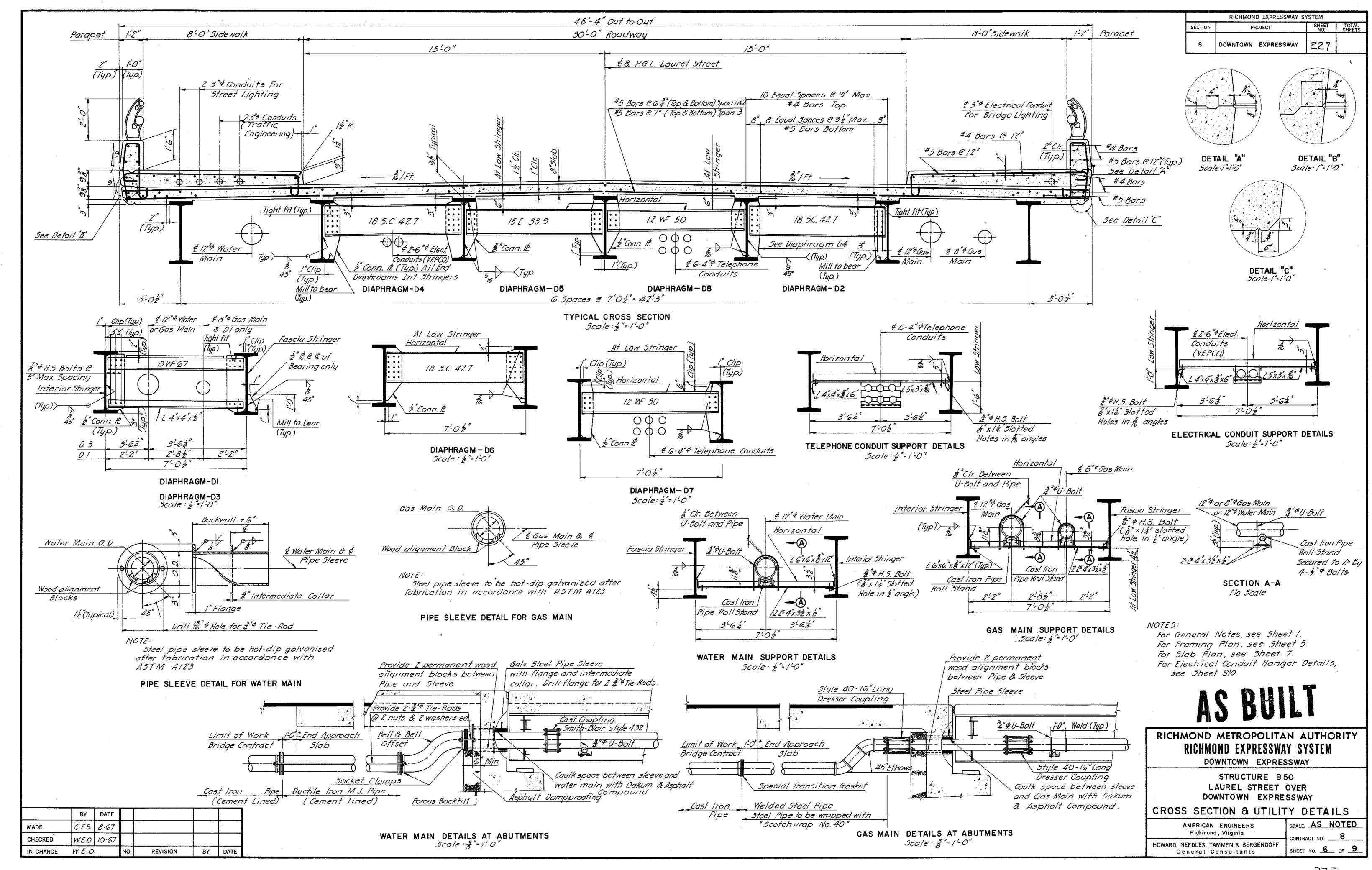
FRAMING PLAN SCALE: AS NOTED AMERICAN ENGINEERS

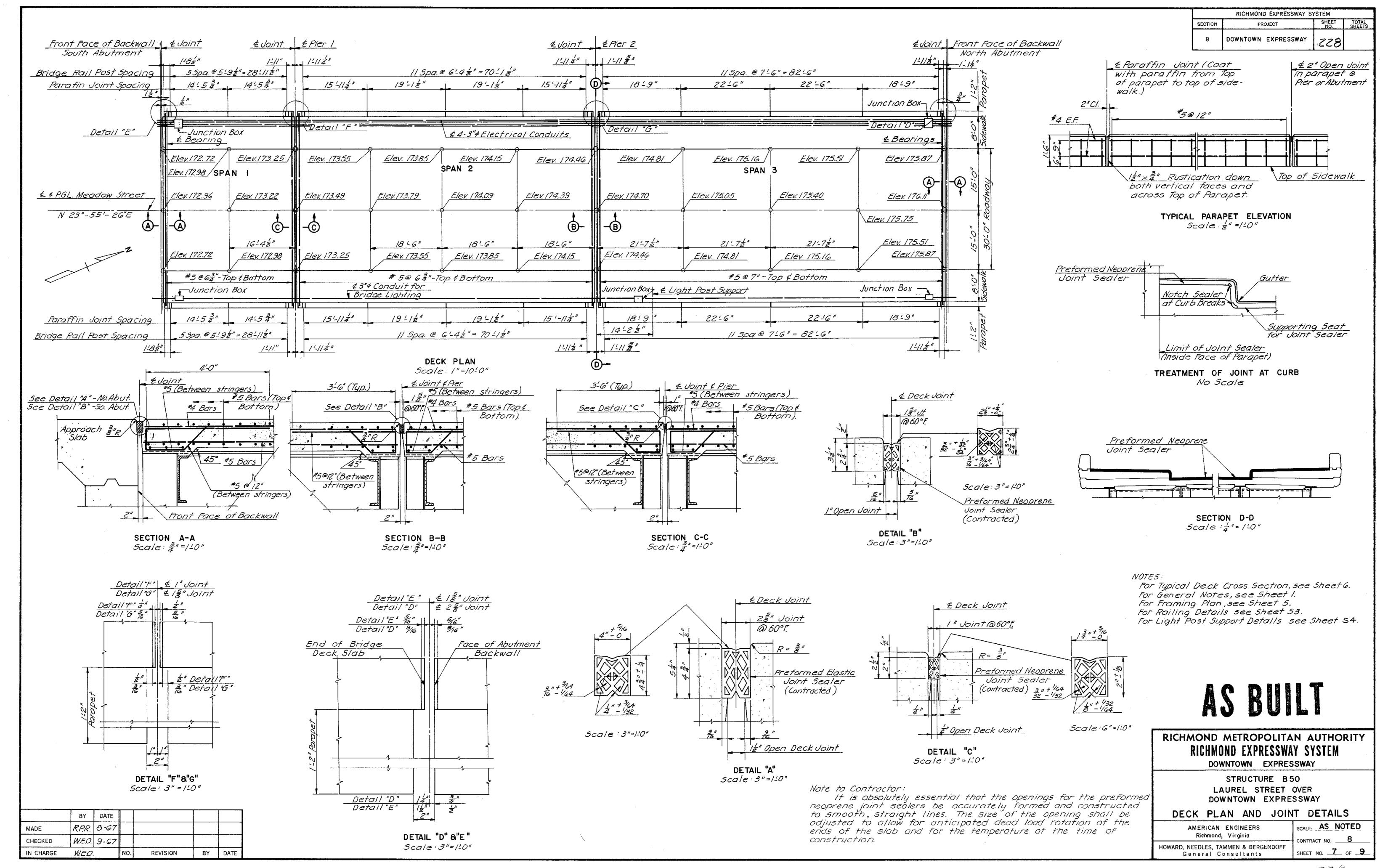
Richmond, Virginia CONTRACT NO.: 8 HOWARD, NEEDLES, TAMMEN & BERGENDOFF SHEET NO. 5 OF 9 General Consultants

F.B.C. 7-67 MADE W.E.O. 9-67 CHECKED IN CHARGE W.E.O. REVISION BY

BY DATE

226



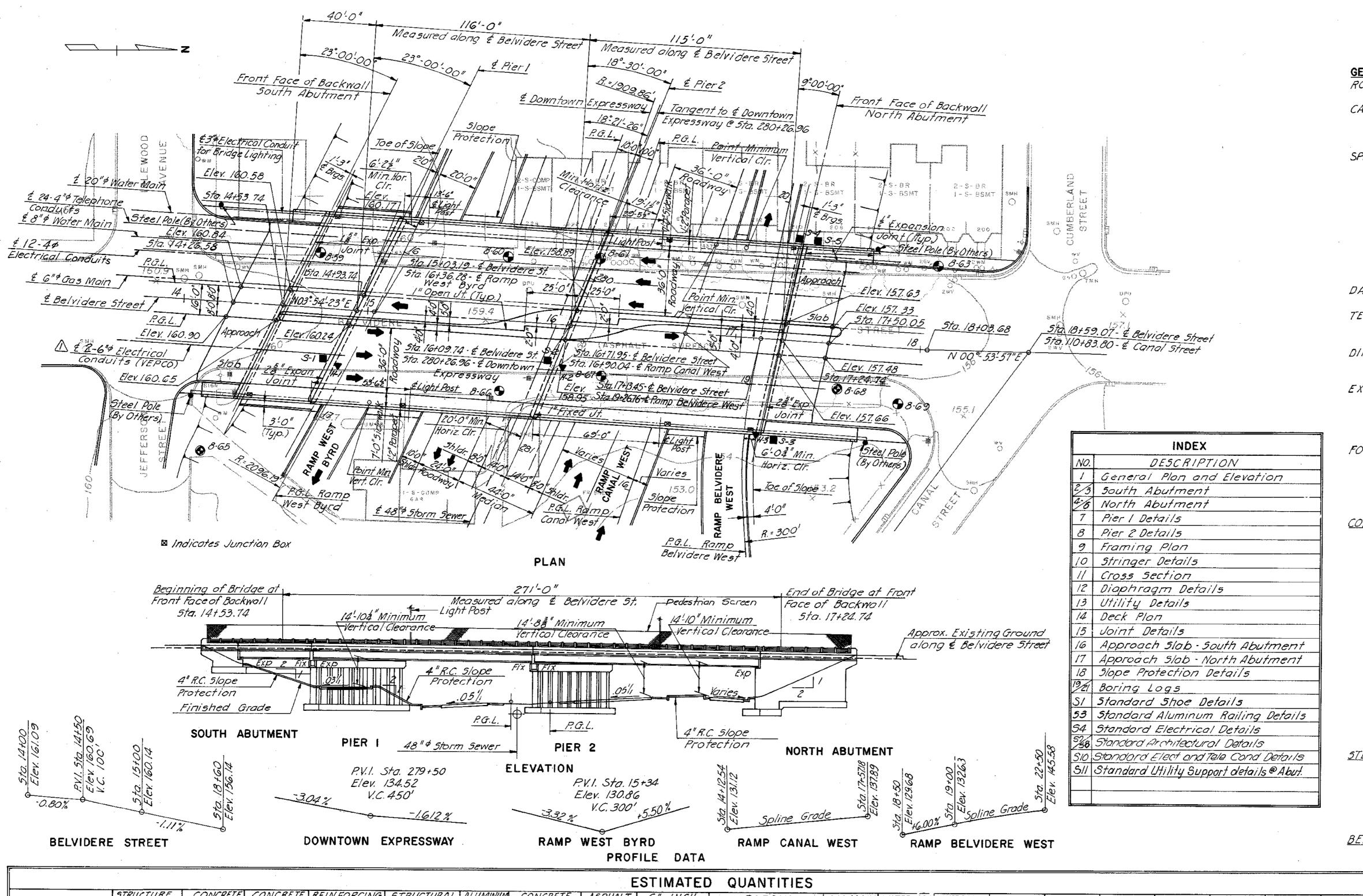


## Bridge 51

Belvidere Street (US 1 and US 301) Over Downtown Expressway (VA 195)

**Record Set Plans** 

RICHMOND EXPRESSWAY SYSTEM PROJECT DOWNTOWN EXPRESSWAY



**GENERAL NOTES:** 

ROADWAY: Two 36'-0" Clear roadways. Two 7'-0" Sidewalks. One

variable median. CAPACITY:

Dead Load - includes 15 lbs. per sq. ft. for future wearing

Live Loads - HS20-44 loading and B.P.R. modified for

military vehicles.

SPECIFICATIONS: GENERAL - Virginia Department of Highway Road and Bridge

Specifications 1970

DESIGN - Á. A. S.H.O. Standard Specifications for Highway Bridges, 1961 modified by Special Design Provisions. WELDING- 1969 Standard Specifications for welded Highway and Railway Bridges of the American Welding Society.

CONTRACT SPECIAL PROVISIONS:

Specifications and Contract Special Provisions referred to above are necessary to make these Plans complete.

CITY OF RICHMOND DATUM:

TEMPERATURE: The normal temperature referred to on the plan is 60°F. The temperature range for movement is 0°F to 120°F.

All dimensions are measured horizontally and vertically DIMENSIONS:

unless otherwise noted.

EXCAVATION:

Excavation below subgrade and cut slope template shall be classified as Structure Excavation. All excavation above these limits shall be classified as Regular Excavation and is not included in the Structural Quantities.

FOUNDATIONS: Footings shall rest on firm material. Foundation material shall be kept dry and special attention is called to Section 401.05 of the General Specifications, and to the Contract Special Provisions, concerning preparation of foundations

for footings.

CONCRETE NOTES:

Concrete in superstructure shall be Class A4. All other concrete shall be Class A3. All exposed edges and corners shall have a # chamfer or fillet unless otherwise noted. Care in the method of vibration, the use of low-slump concrete and for other means shall be employed to prevent downgrade movement of newly placed slab concrete. (When gradient is over 2%).

Finishing concrete surfaces : See the Standard Architectural Detail Sheets and the Contract Special Provisions for types

and details.

All reinforcing steel shall conform to ASTM A615 Grade 40. All reinforcing bar dimensions on the detailed drawings are to centers of bors unless otherwise noted. Clear distance between reinforcing steel and face of concrete shall be as noted on the plans. All bar laps shall be 30 diameters of the smaller diameter bar unless otherwise noted.

STEEL NOTES:

Structural steel shall conform to A.S.T.M. Specification A36 except as noted.

All field connections shall be made with high strength bolts. High Strength bolts shall be &" ounless otherwise noted and shall conform to A.S.T.M. Specification A-325.

BENCH MARK: C-40 Monument located at S.E. corner Idlewood Avenue and Belvidere Street. Elev. 160.76.

				; 					ESTIMATED	QUANTITIES					<del>"</del>					
	STRUCTURE EXCAVATION	CONCRETE CLASS A4	CONCRÉTÉ CLA55 A 3	REINFORCING STEEL	STRUCTURAL STEEL A36	BRIDGE	SLAB SLOPE	DAMP.	6" INCH PIPE UNDERDRAIN	POROUS BACKFILL	GAS MAIN 6"\$	WATER MAI	MWATER MAIN 8" P	CONDUIT 6" \$ VEPCO	CONDUIT 4" # TELE.	Pedestrian Screen	CONDUIT 4" OCITY	3" PVC CONDUIT	2"\$ METAL CONDUIT	
	C, Y.	C.Y.	<i>C.Y.</i>	L B5.	LB5	L. F.	5. Y.	5. Y.	L.F.	C.Y.	L. F.	L. F.	L. F.	L. F.	L.F.	L.F.	L.F.	L. F.	L.F.	
Superstructure		962.16		224,/4/	1,324,511.8	658					323	247	3./3	664 1	6,504	665	3,252	5.70	16	 
South Abutment	475		234.47	11,589			564.9	228	126	76				100,7 2					†	
Pier I	598		258.89	50,865			295.1			·										
Pier Z	3/8		258.71	48,366				1			· · · · · · · · · · · · · · · · · · ·	<u> </u>								 •
North Abutment	1,194		891.55	75,607			/3/.2	468	170	289									-	
Approach Slabs			258.08	58,162														<del> </del>		
Tota/	2,585	962.16	1,901.70	468,730	1, 324, 5/1.8	658	.991.2	696	296	365	323	247	3/3	664 /\	6504	665	3252	570	16	+

NOTES: Top of Pavement Elevations at ends of Deck along P.G.L.'s are given on Plan; Remaining Pavement Elevations are given on Sheet 14.

● Indicates Zz" Cased Hole Boring.

BY

WEO

CHECKED

IN CHARGE

WDU 10-67

DATE

AS BUILT

WEO 12-67 A Rev. VEPCO DGT 11-12-74

REVISION

HMW 4-76

1 Indicates 4" of Cased Hole Boring **▼-Inclicates Geonor Heave Points** 

■ Indicates Settlement Points

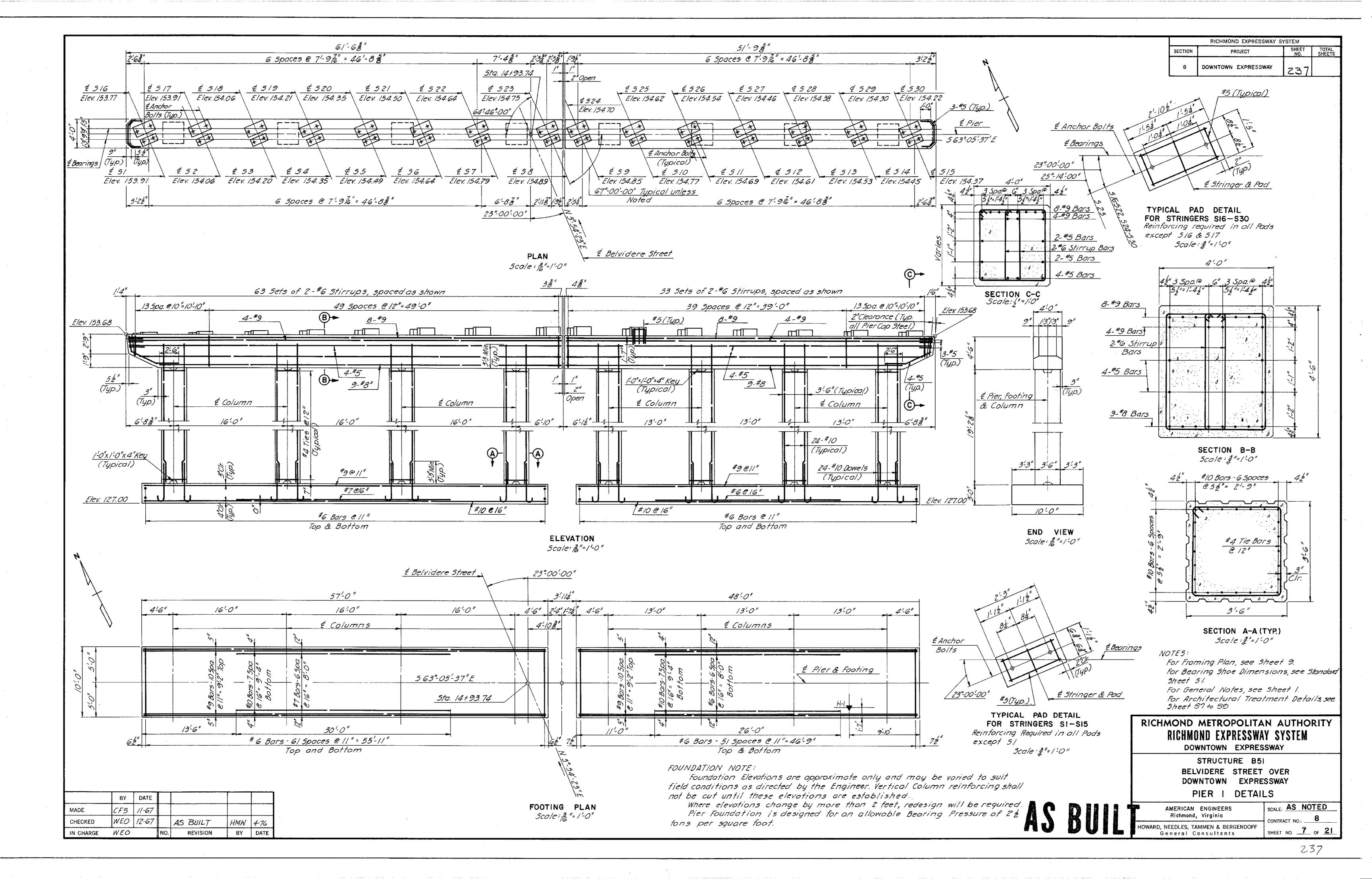
RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

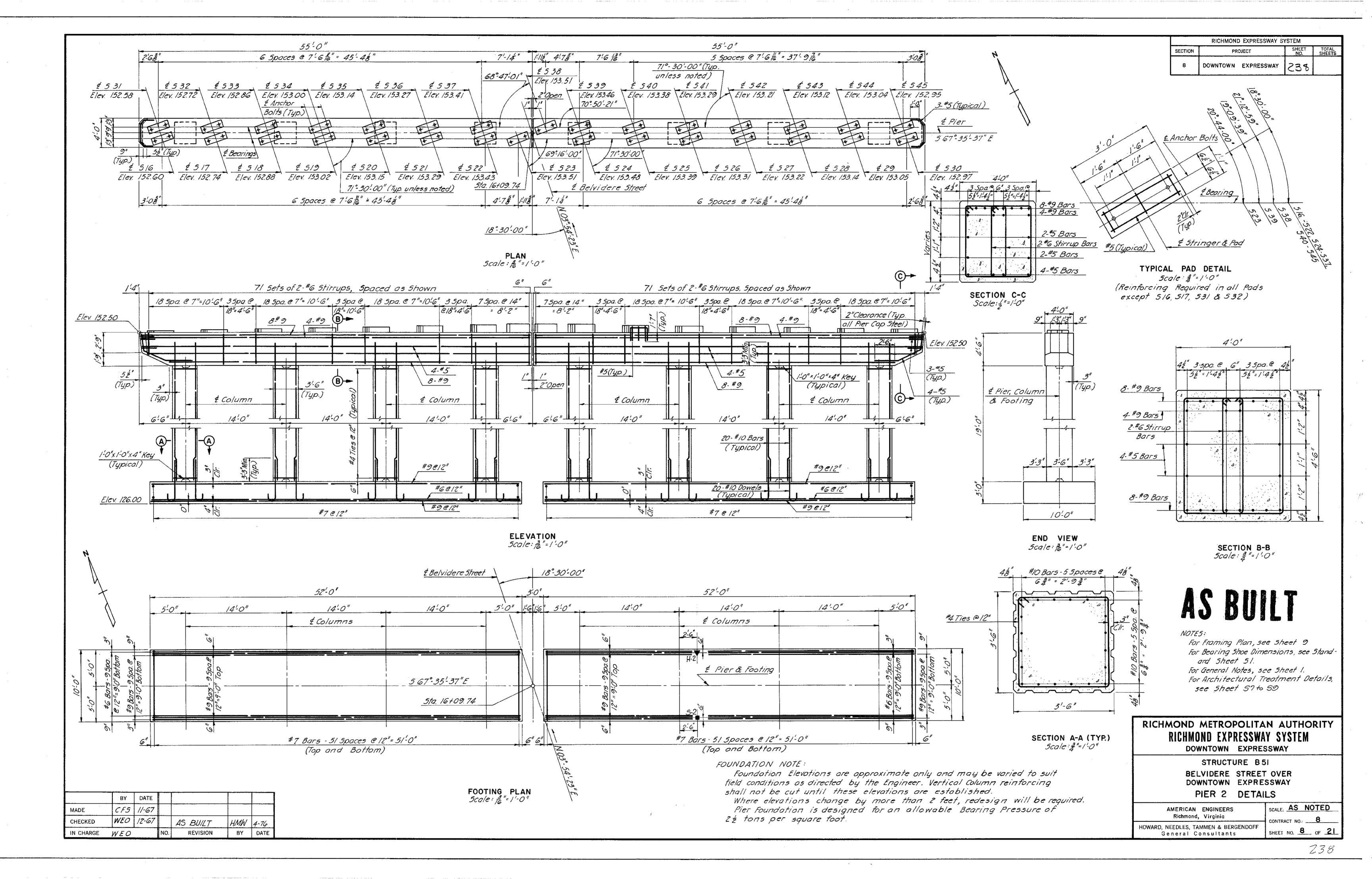
DOWNTOWN EXPRESSWAY

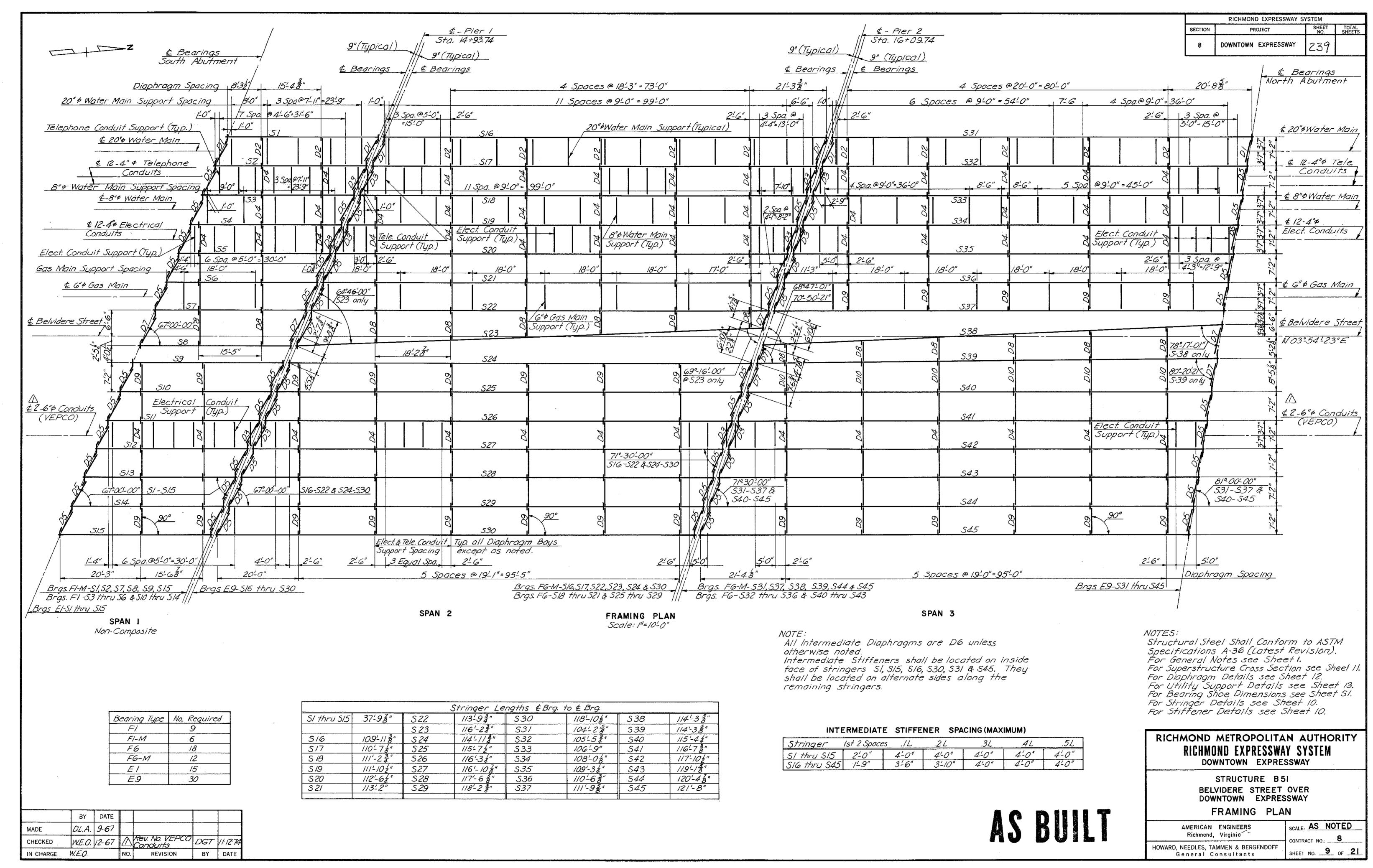
STRUCTURE B 51 BELVIDERE STREET OVER DOWNTOWN EXPRESSWAY

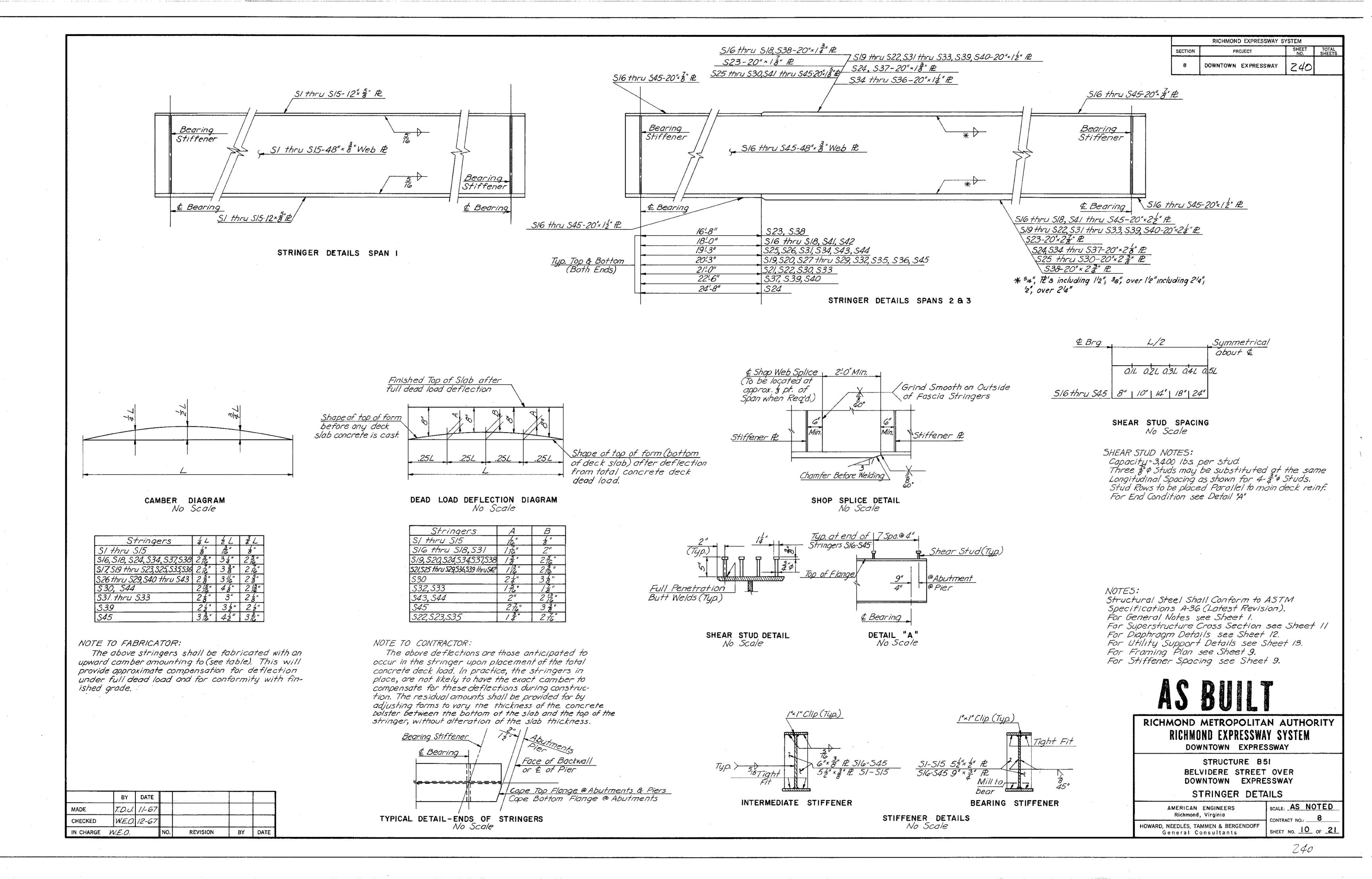
GENERAL PLAN AND ELEVATION

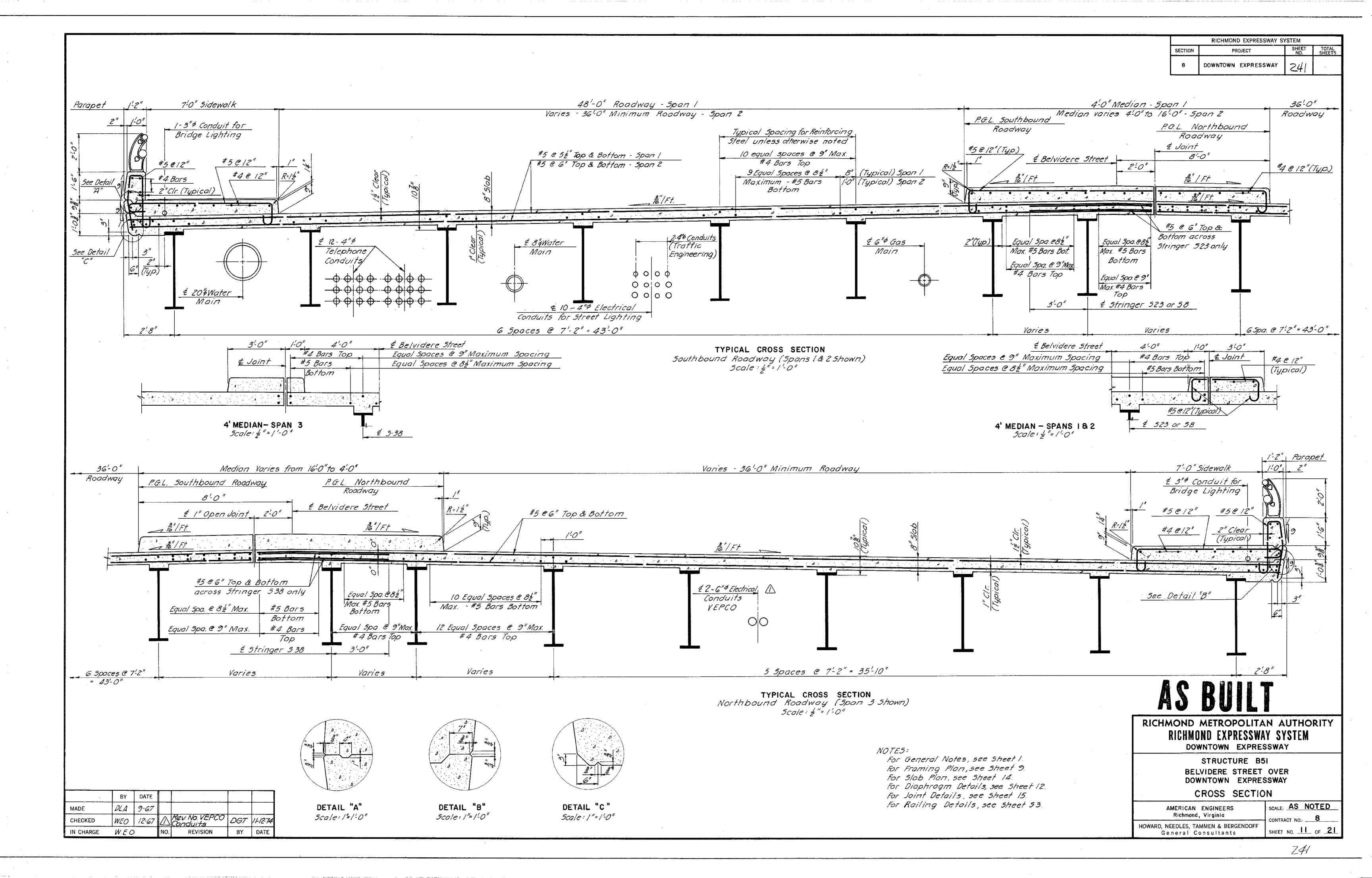
AMERICAN ENGINEERS	SCALE: ["=30'-0"
Richmond, Virginia	CONTRACT NO.: 8
HOWARD, NEFDLES, TAMMEN & BERGENDOFF	SHEET NO. 1 OF 21

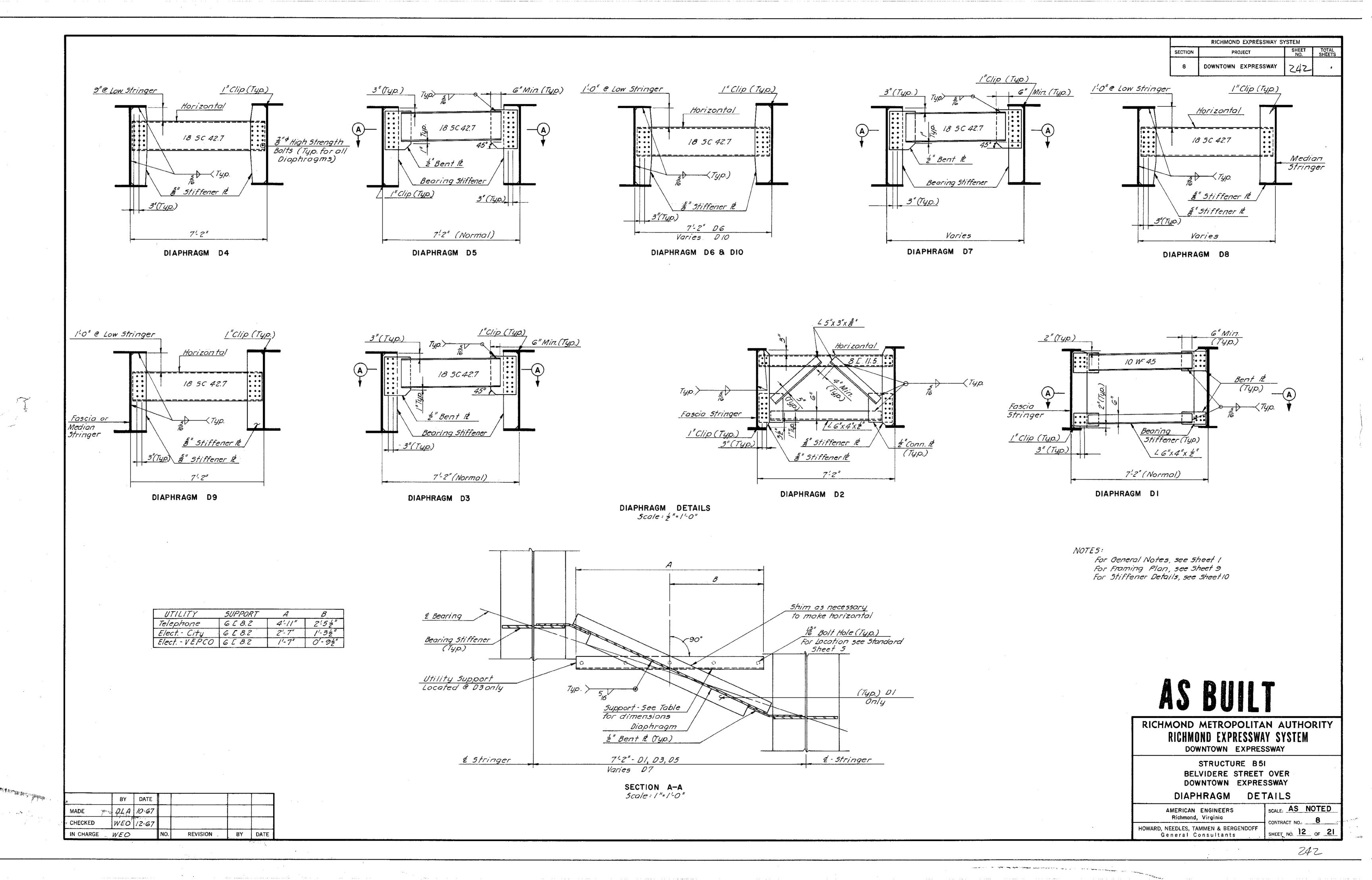


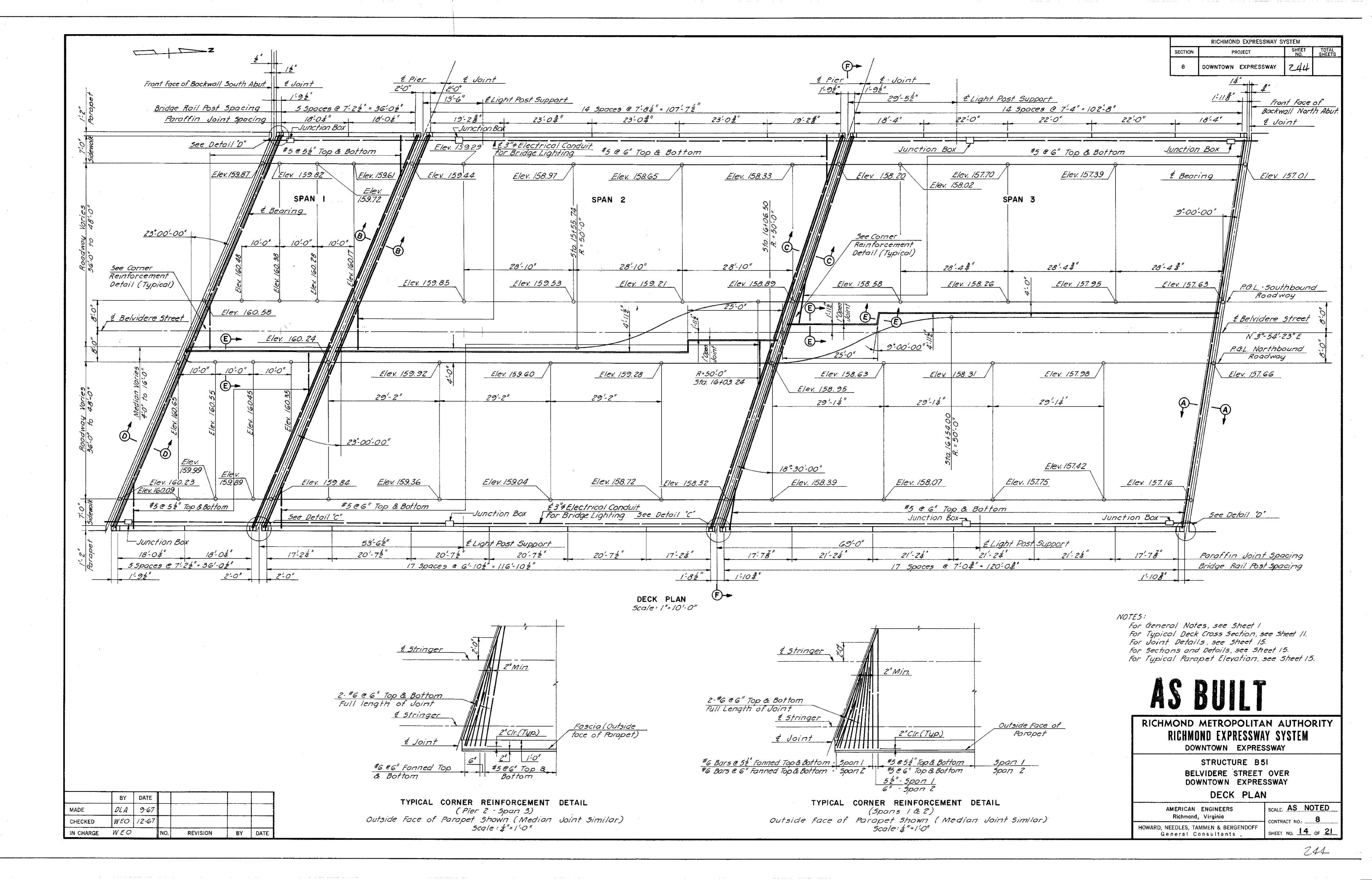




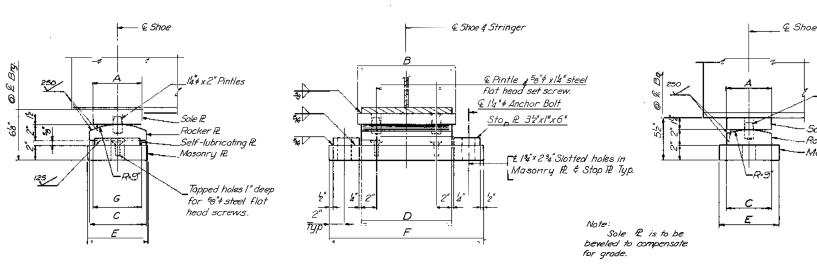


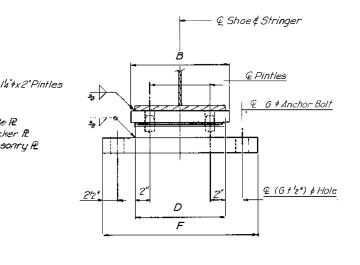


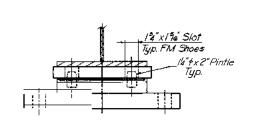




DOWNTOWN EXPRESSWAY







Note: Fixed Modified Shoes some as Fixed Shoes except as shown.

SIDE ELEVATION

END ELEVATION

SIDE ELEVATION

END ELEVATION

END ELEVATION

FIXED SHOE No Scale

-Rocker 12

3" \( x \ \ 8" \) Washer

3 <sup>1</sup>2" ¢ x <sup>3</sup>8" Washer

with 14" & Bolt

with le" & Bolt

G & Anchor Bolt

-Masonry R

FIXED MODIFIED SHOE No Scale

Shoe Notes Material for shoes (exclusive of self-lubricating ptates) shall be high strength low alloy structural steel conforming to ASTM specification A-588

Material for self-lubricating plates shall be Leaded Tin Bronze conforming to ASTM specification B22, Alloy I modified to the extent that 1.5 to 2.5 percent lead is allowable.

Shoes shall be included with structural steel item for payment.

Top or masonry plates, bottom of sole plates and top and bottom of sole plates shall be planed, straightened or otherwise treated to secure true plane surfaces. Contact surfaces noted on the plans with finish symbols shall be finished in occordance with the American Standard's Association surface roughness requirement as defined in ASA B45.1 Surface Roughness, Waviness and Lay, Port I.

The plates comprising the expansion shoes shall be set so as to be truly centered under full dead load at a temperture of 60°F.

Concrete pads shall be formed integral with abutment or pier and not less than 8" or more than 4" above finished elevation. Dress down pads by rubbing, grinding or as otherwise approved by the Engineer, to true level surfaces at the finished elevation.

Anchor bolt assemblies shall conform to A.S.T.M. A-307 and shall be hot-dip galvanized conforming to A.S.T.M. A-153.

Templates shall be used to accurately set the anchor bolts.

**EXPANSION SHOE** 

No Scale 1564 Typ. except FM Shoes, 13x 156 Slot Rocker R

PINTLE DETAIL

Scale: 34"=1"

Burr threads often nut has been <del>set</del> Heav., Hex Nut 3"∮x ³8"Washer Stop R Masonr., R ¢ canvas £ 3°x 2"x3"

EXPANSION SHOE

ANCHOR BOLT DETAIL No Scale

FIXED SHOE

Anchor Bolt for Fixed Shoes some as Anchor Bolt for Exp. Shoes except as shown.

					•	SH	ΟE	DII	MENS	IONS							
		EXP/	NSIO	N SH	OES	1				l	F	IXED	SHO	ES	1		
TYPE	<del>NO.</del> REQD.	Α	В	С	D	Ε	F	G	TYPE	REQD.	Α	В	С	D	Ε	F	G
ΕI	29	6"	/'-/"	7/2	1'-0"	8"	1-8/2	6"	FI	39	6"	/-/"	6"	1'-0"	71/2"	1'-9"	1/4
E 2	7	6"	/'-1"	8/2	1-0"	9"	1'-8/2	6/2	FIM	18	Ğ	/-/"	6"	1'-0"	7/2"	1:9"	144
E3	21	6"	1'-1"	9	1-1"	9%"	1-9/2	7"	F2	9	6"	1'-5/2	F 6"	1-4/2	7"	2'-2"	14"6
E4	14	6"	1'-5/2	∙౭"	1-4/2	' <i>8%</i> "	2-1"	9	F2M	6	6"	1-5%	6"	1-4/2	7"	2'-2"	144
E5	/	6"	1'-52,	9"	1'-4'z"	92"	2'-/"	7"	F4	//	Ğ	1-7"		1-6"	7"	2-4.	1/2"4
E7	20	6"	/'-7"	95"	1'-6"	105"	2:22:	72"	F4M	7	6"	1-7"	6"	1'-6"	7"	2'-4"	1/2"4
E9	40	6"	1'-9"	10"	1.8"	//"	2-4/2	7/2"	F6	18	6"	1-9.		1'-8"	7"	2'-6"	1/2"4
									F6M	12	6"	1'-9"	6"	1'-8"	7"	2'-6"	1/2"¢
i									F7	10	6"	1-9"	6"	1'-8"	8"	2'6'.	1/2"4
									F5	1	Ğ	/'-7"	6"	1-6"	8"	2'-4"	15"P
									F5M	1	6"	/-7"	6"	1'-6"	8"	2'-4"	12° ¢
i								i	i				i				

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

> STANDARD SHOE DETAILS Bridges 50, 51

HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers
ORK ALEXANDRIA KANSAS CITY

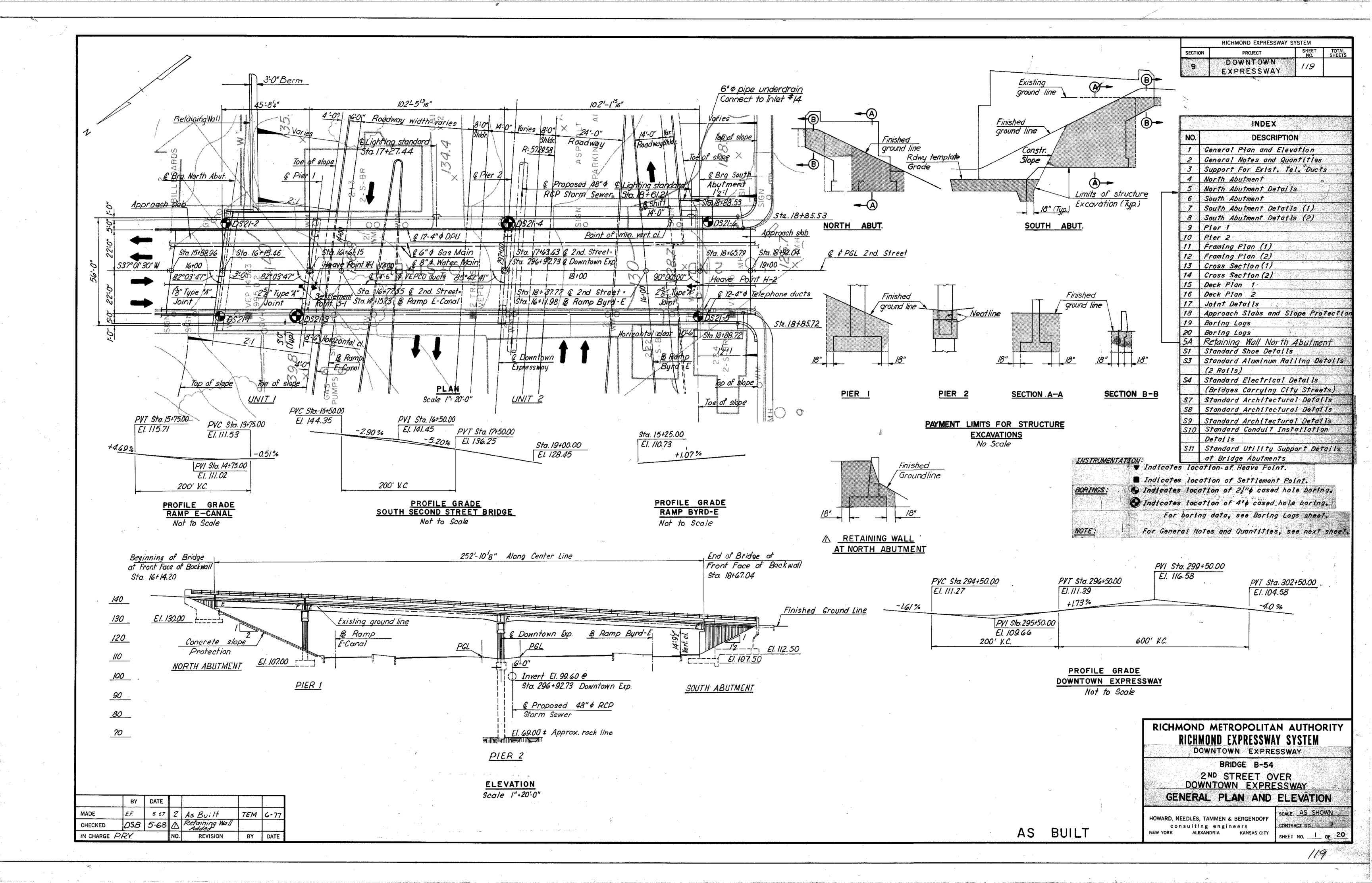
SHEET NO. SI OF

J.M.S. 10-6 W.E.O. 10-6 CHECKED IN CHARGE W.E.O.

# Bridge 54

2<sup>nd</sup> Street
Over
Downtown Expressway (VA 195)

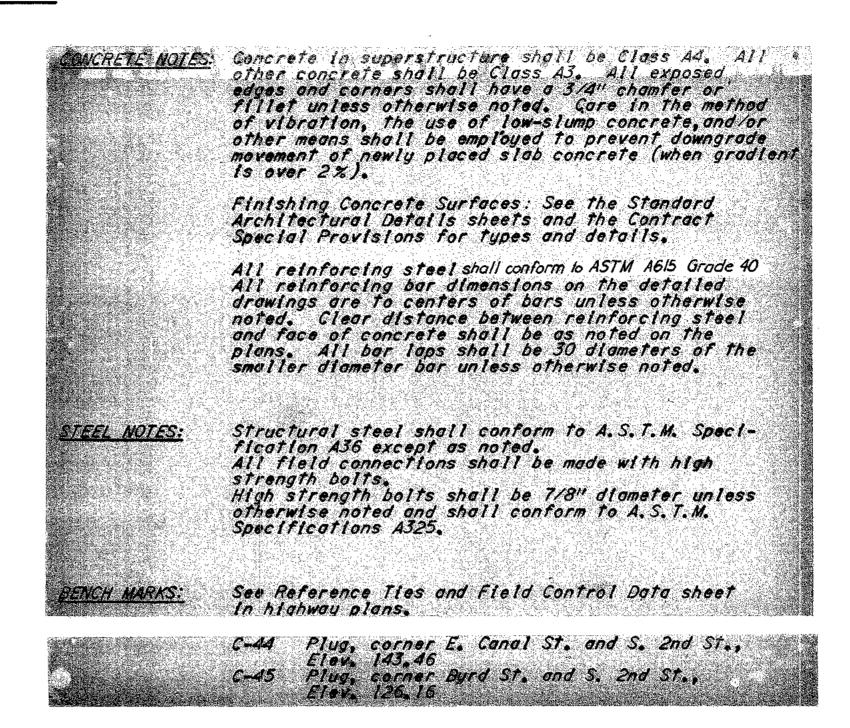
**Record Set Plans** 



### **GENERAL NOTES**

ROAENAY:	One 421-0" clear readway. Two 5'0" sidewalks.
	Dead Load-Includes 15 lbs. per sq.ft. for future wearing surface.
	Live Loads-HS20-44 loading and B.P.R. modified for military vehicles.
SPECIFICATIONS:	
	GENERAL-Virginia Department of Highway Road and Bridge Specifications, 1970.
	DESIGN-A.A.S.H.O. Standard Specifications for Highway Bridges, 1961, modified by Special
	Design Provisions. WELDING-1969 Standard Specifications for Welded Highway and Railway Bridges of the American Welding Society.
	CONTRACT SPECIAL PROVISIONS
	Specifications and Contract Special Provisions referred to above are necessary to make these plans complete.
DA <i>TUM:</i>	CITY OF RICHMOND
TEMPERATURE :	The normal temperature referred to on the plan is 60°F. The temperature range for movement is 0°F to 120°F.
<u>DIMENSIONS:</u>	All dimensions are measured horizontally and vertically unless otherwise noted.
<u>EXCAVATION:</u>	Excavation below subgrade and cut slope template shall be classified as Structure Excavation, All excavation above these limits shall be classified as Regular Excavation and is not included in the Structural Quantities.

Footings shall rest on firm material.
Foundation material shall be kept dry and special attention is called to Sections 401.05 and 401.06 of the General Specifications and to the Contract Special Provisions cancerning preparation of foundations for footings.



	FINAL QUANTITIES																				
	STRUCTURE EXCAVATION C.Y.	CONCRETE CLASS A3 CY	CONCRETE CLASS A3 BR APPR SLABS CY	CONCRETE CLASS A4 CXY	REINFORCING STEEL LBS:	STRUCTURAL STEEL LBS	ALUMINÚM BR. RAILING (2 RAILS) L.F	DRILLED HOLES FOR CAISSONS 4'0 LF	POROUS BACKFILL C.Y	CONC SLAB SLOPE PROTECTION S Y	DAMP- PROOFING S.Y	UNDERDRAIN 6"	GAS MAIN 6° P Lif	WATER MAIN 8"0 L.F	CONDUIT 6" Ø VEPCO L.F.	CONDUIT 4' 0 TELEPHONE LF **	CONDUIT 4" \$ D.PU. L.F	METAL CONDUIT 3" Ø LE	STEEL PLES IOBP42 L.F.		MAINT & SUPT EXISTING TEL CABLES LIS
SUPERSTRUCTURE				503.23	101,330	565,498	506						6/2	308	1224	4621	3625	7/7			
NORTH ABUTMENT	652	234,45			13,647		12		60	360.83	159	174				<u> </u>			1432.9		
PIER I	151	109.19			26,845																
PIER 2	38	113.26			22,500			80.3													
SOUTH ABUTMENT	329.43	321.55			28,511		63		95		194	132									
APPROACH SLABS			121.95		26,710																
TOTAL	1170.43	778.45	121.95	503.23	219,543	565,498	581	80.3	/55	360.83	353	306	6/2	308	1224	4621	3625	7/7	1432.9	,	L.S.

\* \* Does not include existing 9-duct bank to be maintained in service

		******	***************************************				
		BY	DATE				
	MADE	EV.R.		2	As Built	TEM	<b>6-77</b>
j	CHECKED	SBP <sub>PRY</sub>	5-68	$\triangle$	Quant N. Abut.	RBH	6-74
	IN CHARGE F	$Q_{i}Q_{i}Y_{i}$		NO.	REVISION	вү	DATE

RICHMOND EXPRESSWAY SYSTEM DOWNTOWN EXPRESSWAY

RICHMOND METROPOLITAN AUTHORITY

RICHMOND EXPRESSWAY SYSTEM

120

PROJECT

EXPRESSWAY

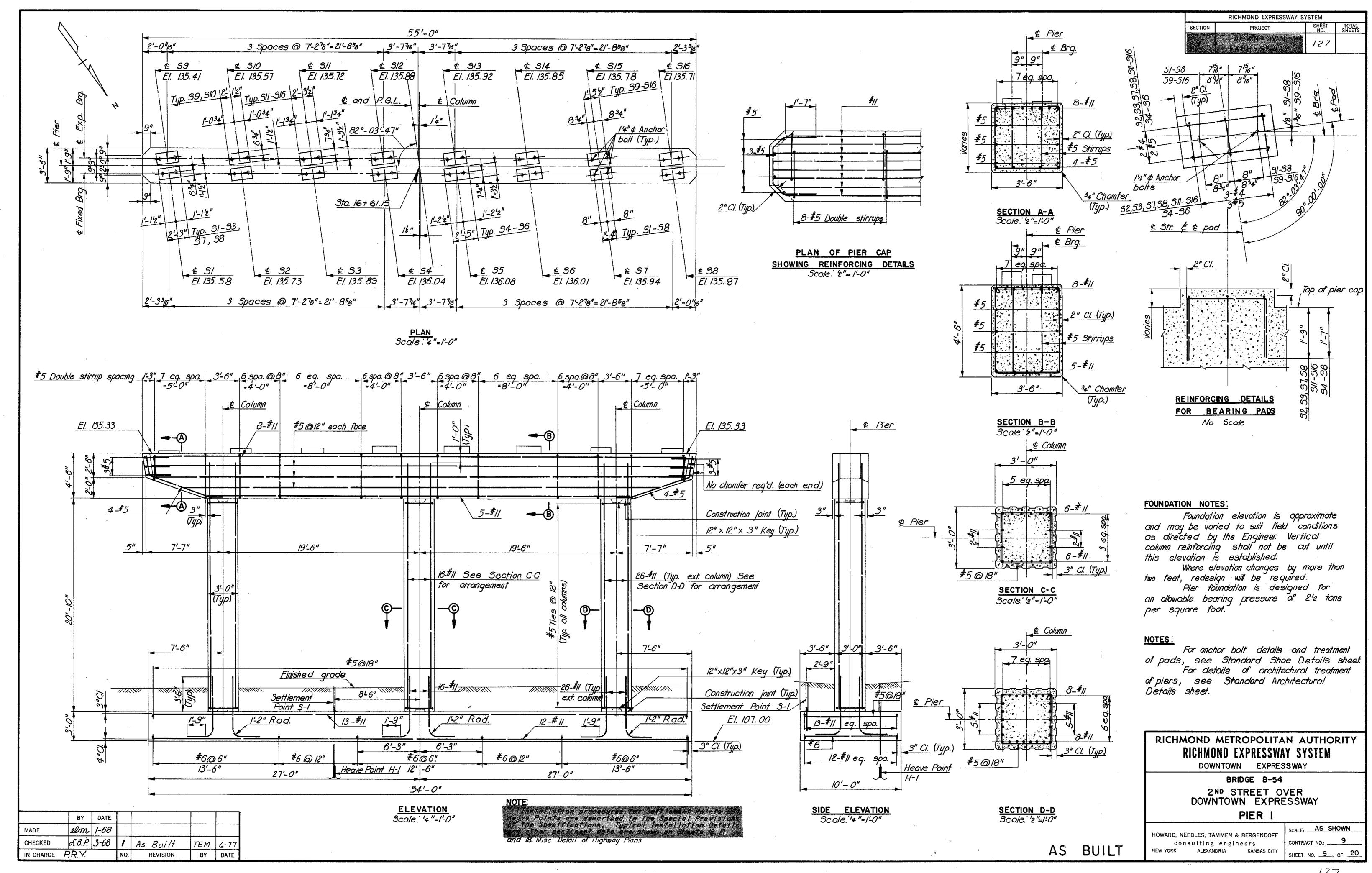
SECTION

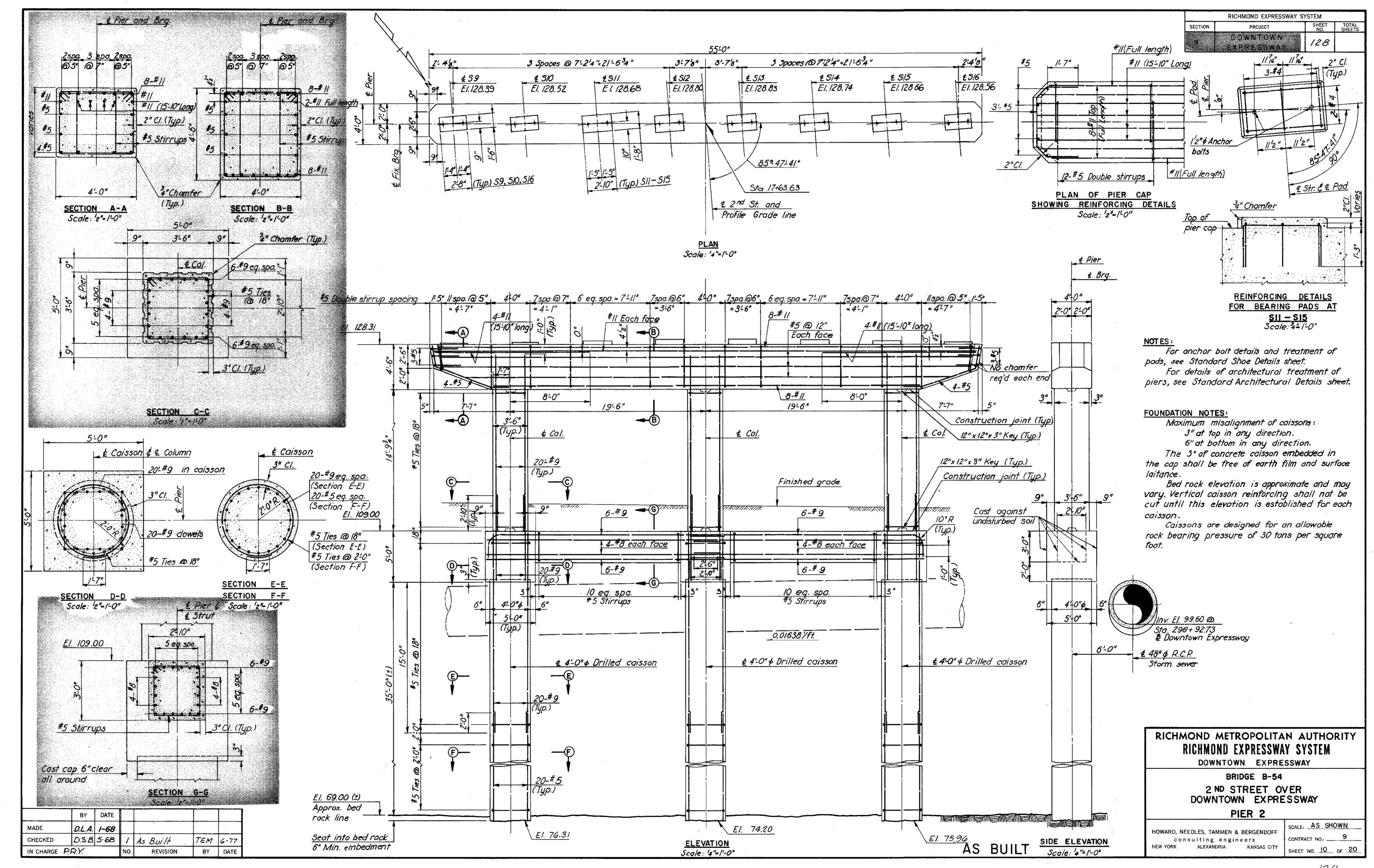
BRIDGE 8-54 - 2 ND STREET OVER DOWNTOWN EXPRESSWAY

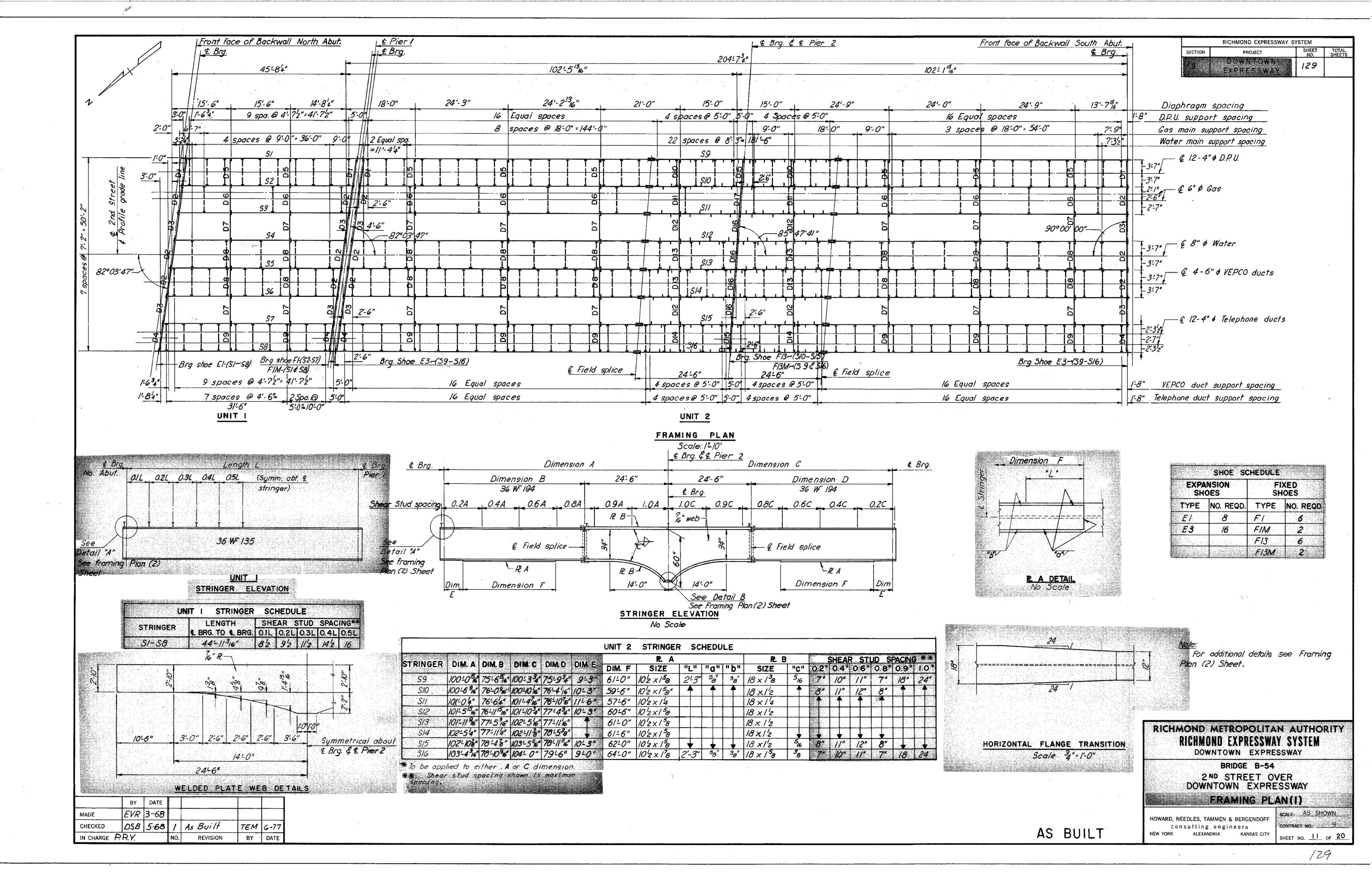
GENERAL NOTES AND QUANTITIES

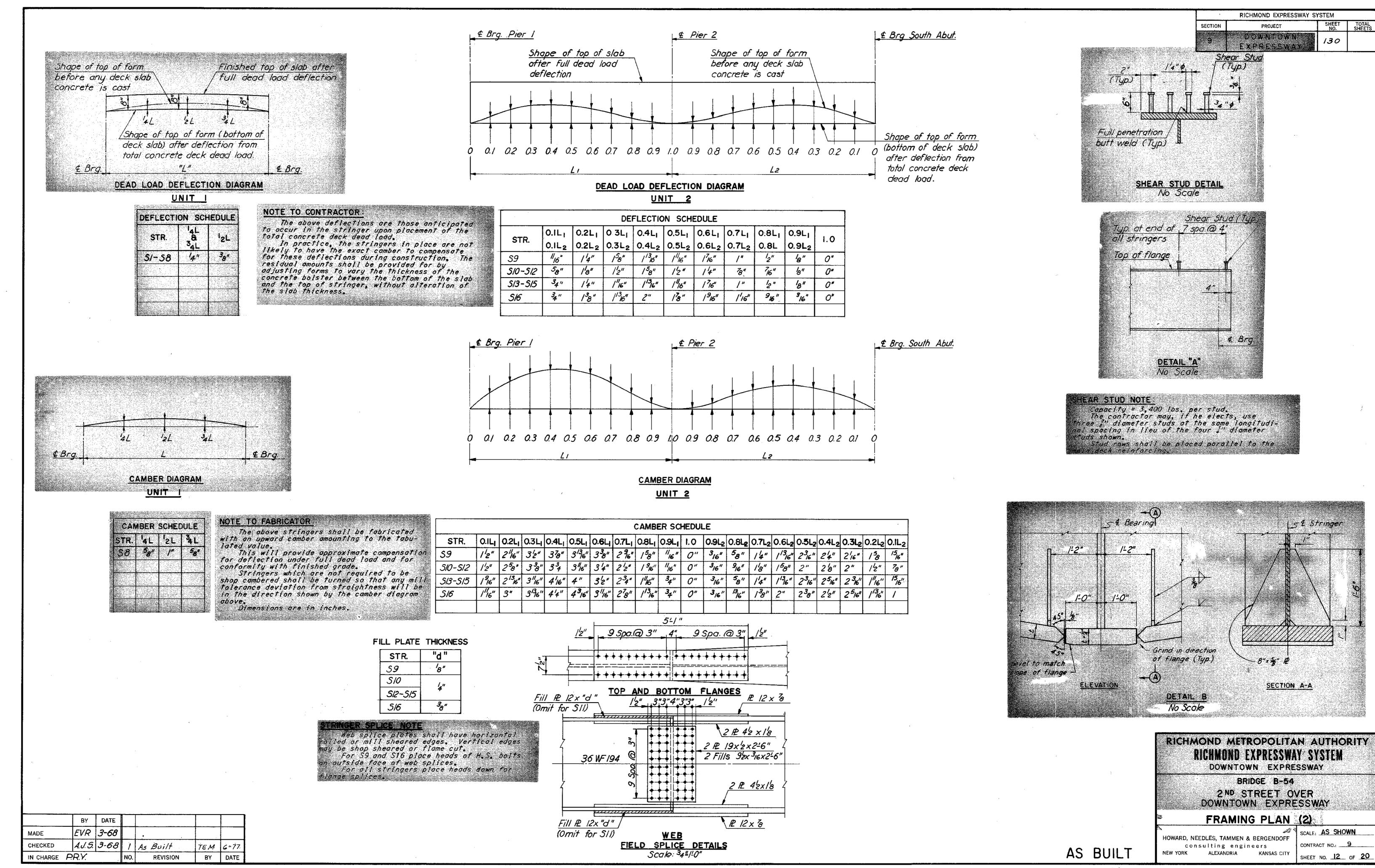
HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers YORK ALEXANDRIA KANSAS CITY

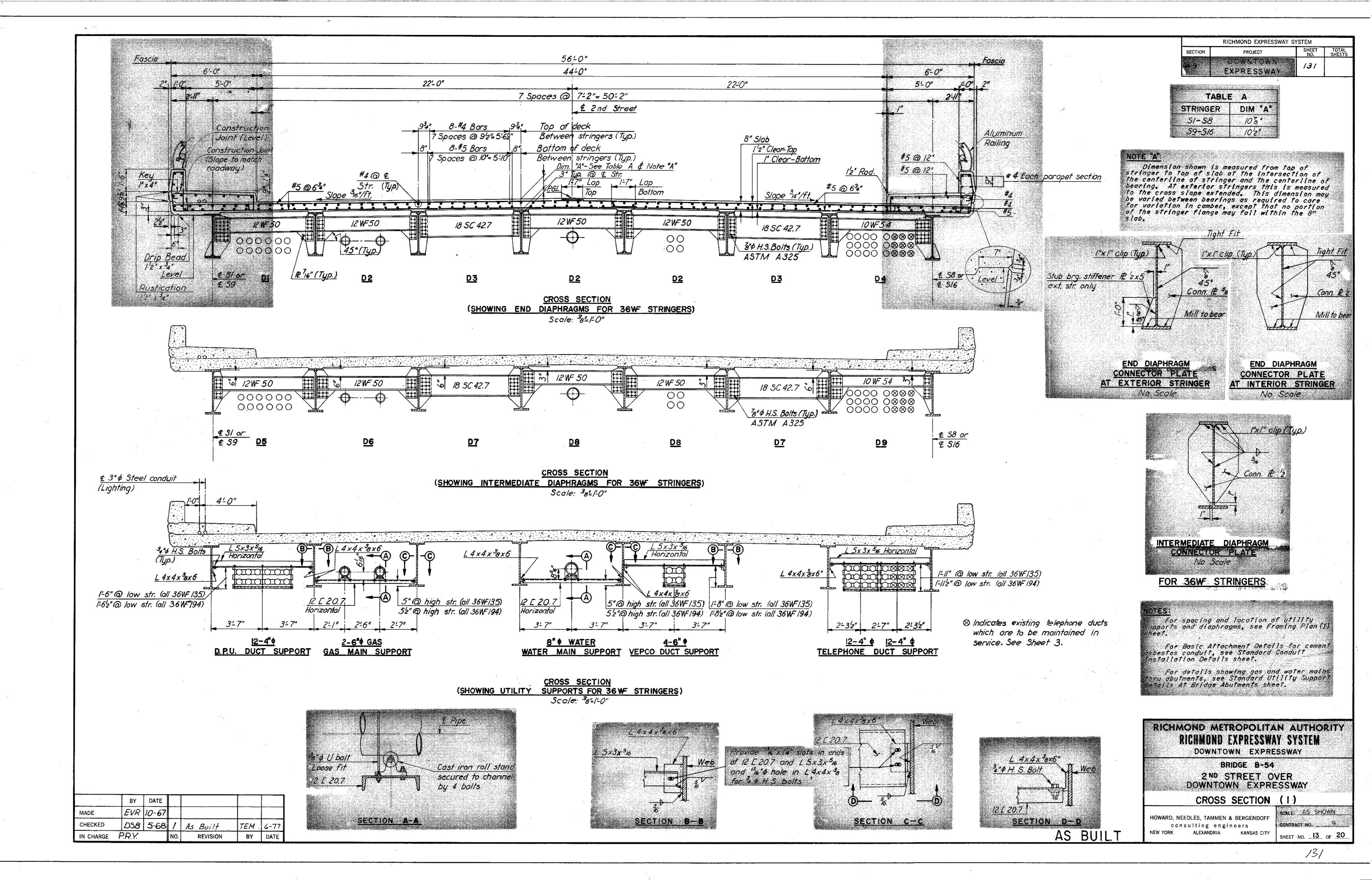
SHEET NO. 2 OF 20

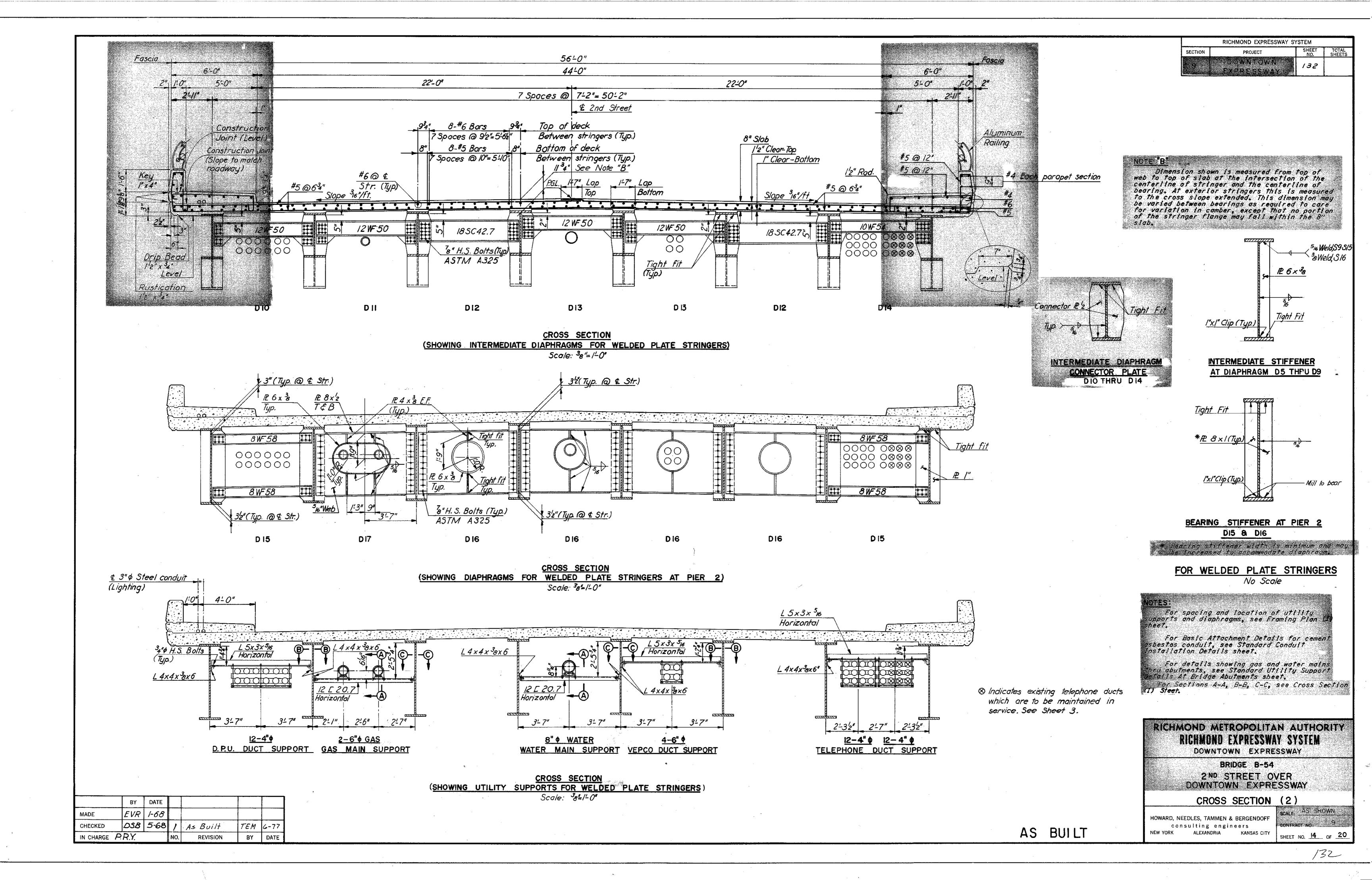


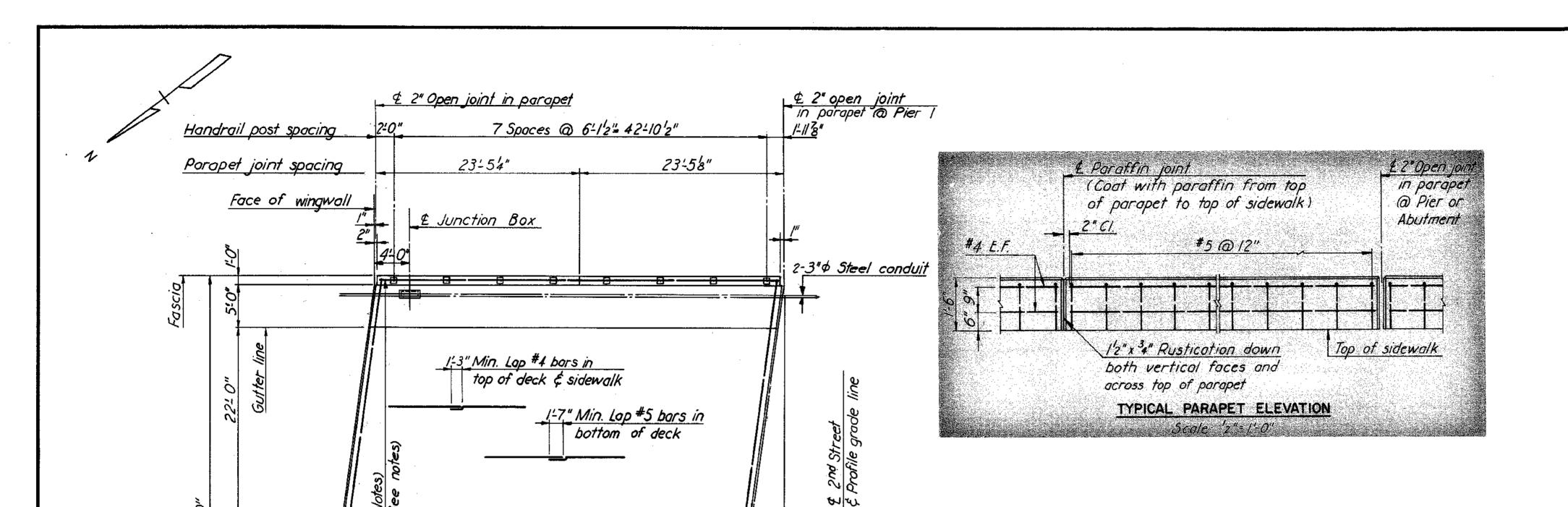












7-63"

1'-3" Min. Lop #4 bors in

top of deck & sidewalk

#5@ 634" Top & Bottom

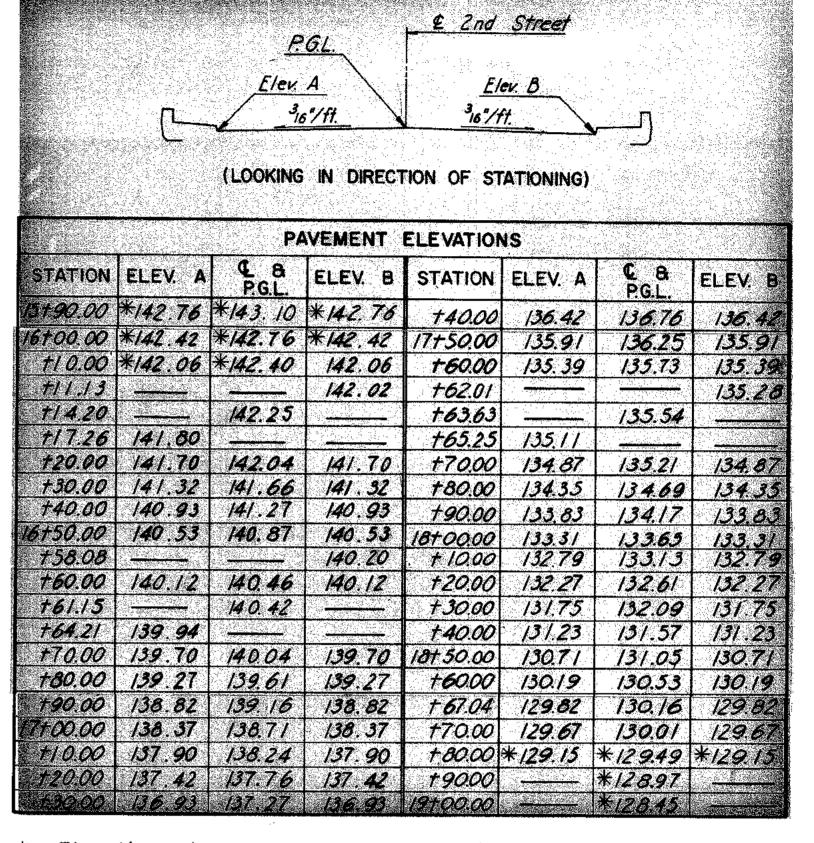
46'-118"

PLAN-UNIT I Scale: 18"=1-0"

1-7" Min. Lop #5 bors in

bottom of deck

RICHMOND EXPRESSWAY SYSTEM PROJECT DOWNTOWN /33 EXPRESSWAY



\* Elevations shown are given to top of bituminous surfacing.

AS BUILT

For location and spacing of deck, parapet and sidewalk reinforcing, see Cross Section (1) For location and spacing of reinforcing in haunch over and diaphragms, see Joint Details

For lighting standard base, junction box details and additional reinforcing, see Standard Electrical Details sheet S4.

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM DOWNTOWN EXPRESSWAY BRIDGE B-54 2ND STREET OVER

DOWNTOWN EXPRESSWAY DECK PLAN

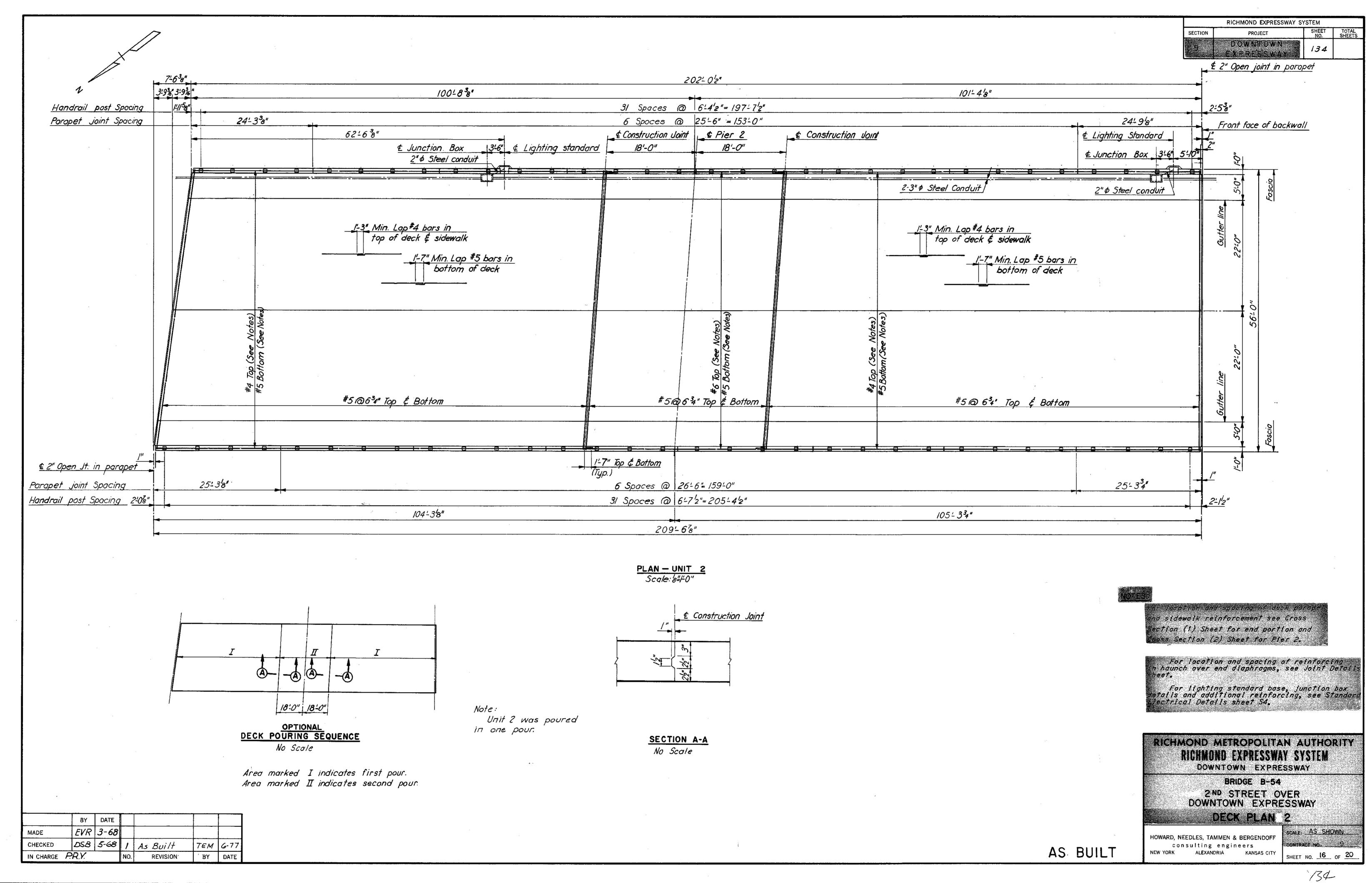
HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers

ALEXANDRIA KANSAS CITY NEW YORK

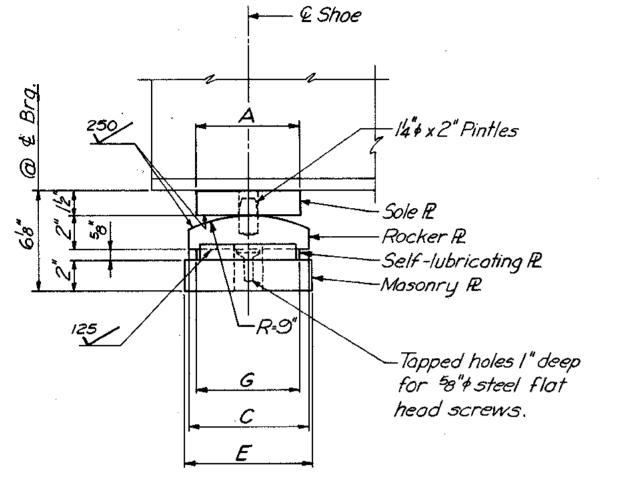
EVR 3-68 MADE OSB 5-68 CHECKED TEM 6-77 IN CHARGE PR.YDATE

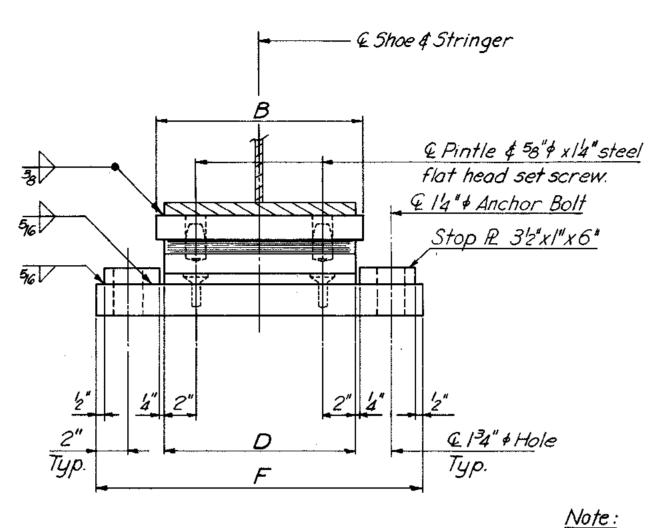
Face of wingwall

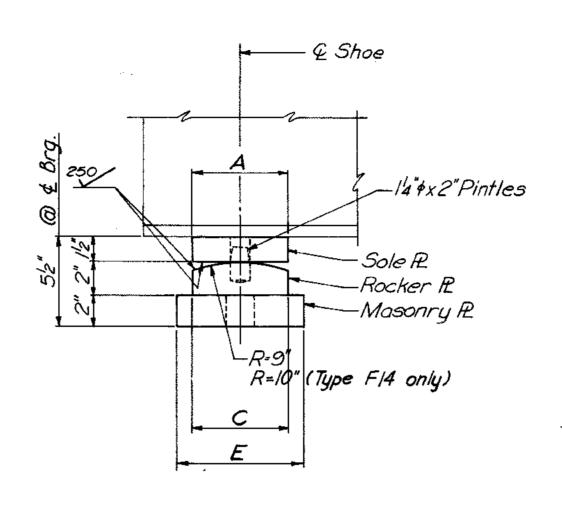
133

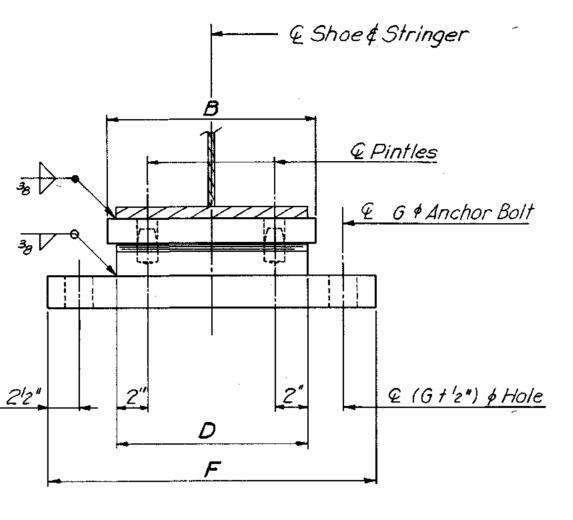


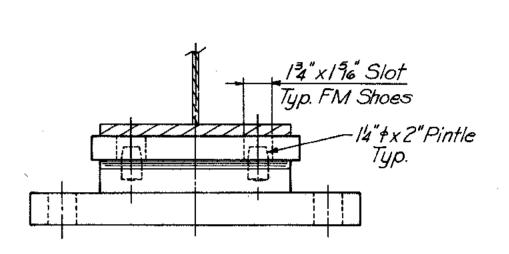
RICHMOND EXPRESSWAY SYSTEM SECTION **PROJECT** DOWNTOWN 242 EXPRESSWAY











Fixed Modified Shoes same as Fixed Shoes except as shown.

SIDE ELEVATION

END ELEVATION

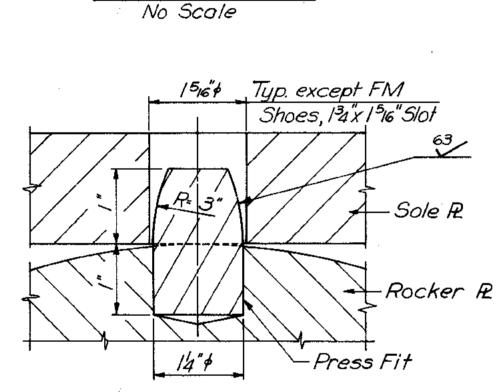
Sole plate is to be beveled to compensate for grade.

SIDE ELEVATION

END ELEVATION

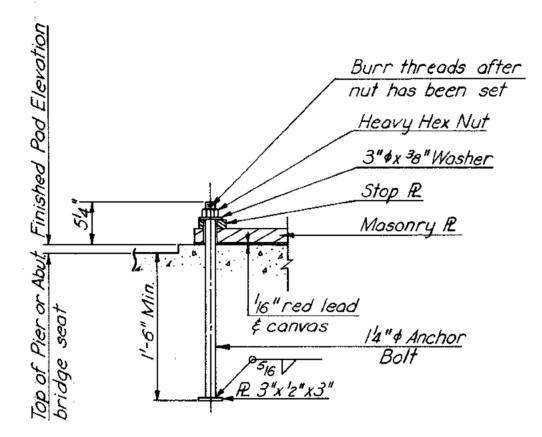
END ELEVATION

## **EXPANSION SHOE**

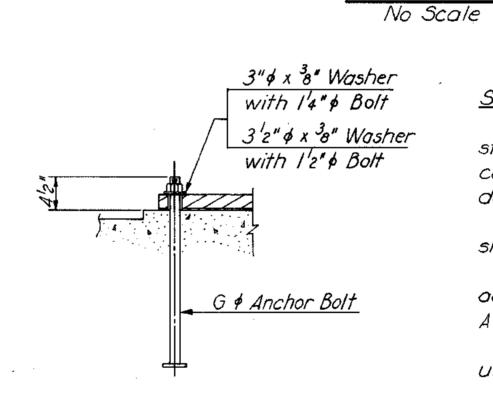


PINTLE DETAIL

Scale: 34"= |"



**EXPANSION SHOE** 



## ANCHOR BOLT DETAIL

No Scale

FIXED SHOE

Note: Anchor Bolt for Fixed Shoes same as Anchor Bolt for Exp. Shoes except as shown.

FIXED SHOE

FIXED MODIFIED SHOE No Scale

Shoe Notes: Material for shoes (exclusive of self-lubricating plates) shall be structural carbon steel conforming to requirements of A.S.T.M. Specifications A242-66T and shall have a corrosion resistance equal to a minimum of four times that of A.S.T.M.-A7 steel as determined by a salt spray test conforming to A.S.T.M. Method B287-62.

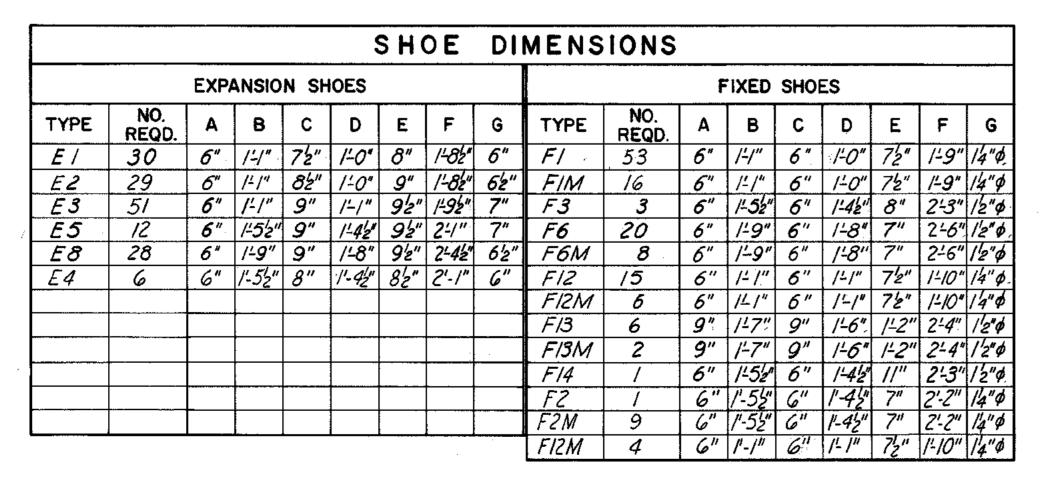
Top of masonry plates, bottom of rocker plates and top and bottom of sole plates shall be planed, straightened or otherwise treated to secure true plane surfaces. Contact surfaces noted on the plans with finish symbols shall be finished in accordance with the American Standards Association surface roughness requirement as defined in

ASA B46.1-55, Surface Roughness, Waviness and Lay, Port I. The plates comprising the expansion shoes shall be set so as to be truly centered under full dead load at a temperture of 60°F.

Concrete pads shall be formed integral with abutment or pier and not less than 18" or more than 14" above finished elevation. Dress down pads by rubbing, grinding or as otherwise approved by the Engineer, to true level surfaces at the finished elevation. Anchor bolt assemblies shall conform to A.S.T.M. A-307-66 and shall be hot-dip galvanized conforming to A.S.T.M. A-153-66.

AS BUILT

Templates shall be used to accurately set the anchor bolts.



# — ⊈ Shoe -- 14" \$\tau 2" Pintles Sole R - Rocker R -Masonry R \\_R=12"

#### SIDE ELEVATION FIXED SHOE TYPE FI3 No Scale:

Note: For details and dimensions not shown see Fixed Shoe details above

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM DOWNTOWN EXPRESSWAY

STANDARD

SHOE DETAILS Bridges 54, 55, 56, 57, 58, 60

KANSAS CITY

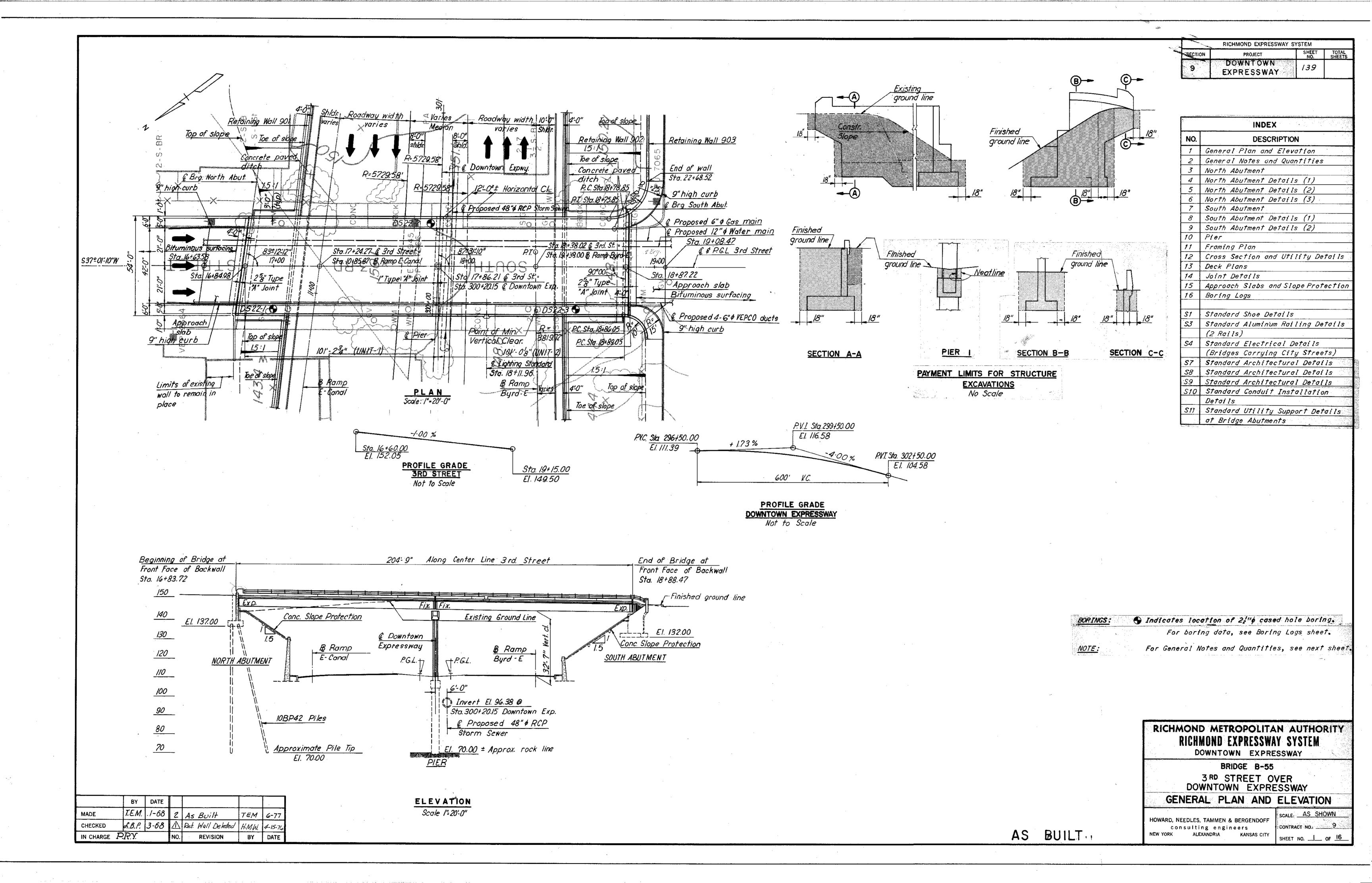
HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers

ALEXANDRIA

SCALE: AS SHOWN CONTRACT NO.: SHEET NO. SI OF \_\_\_

	ВҮ	DATE				
MADE	б.В.Р.	3-68				
CHECKED	A.J.5.	3-68	1	As Built	TEM	7-77
IN CHARGE	PR Y		NO.	REVISION	BY	DATE

3<sup>rd</sup> Street
Over
Downtown Expressway (VA 195)



One 421-0" clear roodway. Two 5'0" sidewalks. Dead Load-Includes 15 lbs. per sq.ft. for future wearing surface.
Live Loads-HS20-44 loading and B.P.R. modified for military vehicles. SPECIFICATIONS GENERAL-Virginia Department of Highway Road and Bridge Specifications, 1970.
DESIGN-A.A.S.H.O. Standard Specifications for Highway Bridges, 1961, modified by Special Design Provisions.
WELDING-1969 Standard Specifications for Welded Highway and Railway Bridges of the American Welding Society. CONTRACT SPECIAL PROVISIONS Specifications and Contract Special Provisions referred to above are necessary to make these plans complete. CITY OF RICHMOND DATUM: The normal temperature referred to on the plan is 60°F. The temperature range for movement is 0°F to 120°F. TEMPERATURE: All dimensions are measured horizontally and vertically unless otherwise noted. DIMENSIONS: Excavation below subgrade and cut slope template shall be classified as Structure Excavation. All excavation above these limits shall be classified as Regular Excavation and is not included in the Structural Quantities. EXCAVATION: Footings shall rest on firm material,
Foundation material shall be kept dry and
special attention is called to Sections
401.05 and 401.06 of the General Specifications and to the Contract Special Provisions
concerning preparation of foundations for

CONCRETE NOTES: Concrete in superstructure shall be Class A4. All other concrete shall be Class A3. All exposed edges and corners shall have a 3/4" chamfer or fillet unless otherwise noted. Care in the method of vibration, the use of low-slump concrete, and/or other means shall be employed to prevent downgrade movement of newly placed slab concrete (when gradient is over 2%). Finishing Concrete Surfaces: See the Standard Architectural Details sheets and the Contract Special Provisions for types and details. All reinforcing steel shall conform to ASTM A6/5 Grade40 All reinforcing bar dimensions on the detailed drawings are to centers of bars unless otherwise noted. Clear distance between reinforcing steel and face of concrete shall be as noted on the plans. All bar laps shall be 30 diameters of the smaller diameter bar unless otherwise noted. Structural steel shall conform to A.S.T.M. Speci-fication A36 except as noted. STEEL NOTES: All field connections shall be made with high strength bolts.
High strength bolts shall be 7/8" diameter unless otherwise noted and shall conform to A.S.T.M.
Specifications A325. See Reference Ties and Field Control Data sheet in highway plans. Plug, corner Byrd St. and S. 3rd St., Elev. 149.69 Plug, corner E. Canal St. and S. 3rd St., Elev. 152.70

								FIN	IAL QUA	NTITIES								
	STRUCTURE EXCAVATION C.Y	CONCRETE CLASS A3 C Y	CONCRETE CLASS A3 BR APPR SLABS CX	CONCRETE CLASS A4	REINFORCING STEEL LBS.	STRUCTURAL STEEL LBS.		DRILLED HOLES FOR CAISSONS 4'0 L.F	POROUS BACKFILL C.Y	STEEL RLES IOBP42 LIF	CONC. SLAB SLOPE PROTECTION S Y	DAMP- PROOFING S.Y	UNDERDRAIN 6" Ø L.F	GAS MAIN 6°0 L.F	WATER MAIN 12"¢ L.F	CONDUIT 6"¢ VEPCO L'F	METAL CONDUIT 3" Q LF	
SUPERSTRUCTURE				384.39	84,396	418,496	409.9		And American (A. C. Opening Supp. Mys. A Mandels A (C. Opening Supp. Mys. A (C. Opening Supp. Mys. A Mandels A (C. Opening Supp. Mys. A (C. Opening Supp. My				Special control - Land Tracks of the Both State State Special State Stat	251	249	997	1278	againty (10)(3)
NORTH ABUTMENT	299	175.67			12,201		22.7		29	2134	178.68	111	129					
PIER I	38	140.94			39,985			95.3										
SOUTH ABUTMENT	623	209.43			16,751		28.9		41		246.75	//8	90					
APPROACH SLABS	-		100.37		22,618													
TOTAL	960	526.04	100.37	384.39	175,951	418,496	461.5	95.3	70	2134	425.43	229	219	251	249	997	1278	

TEM 6-77 IN CHARGE PRY

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM

RICHMOND EXPRESSWAY SYSTEM

PROJECT

DOWNTOWN EXPRESSWAY

SECTION

DOWNTOWN EXPRESSWAY

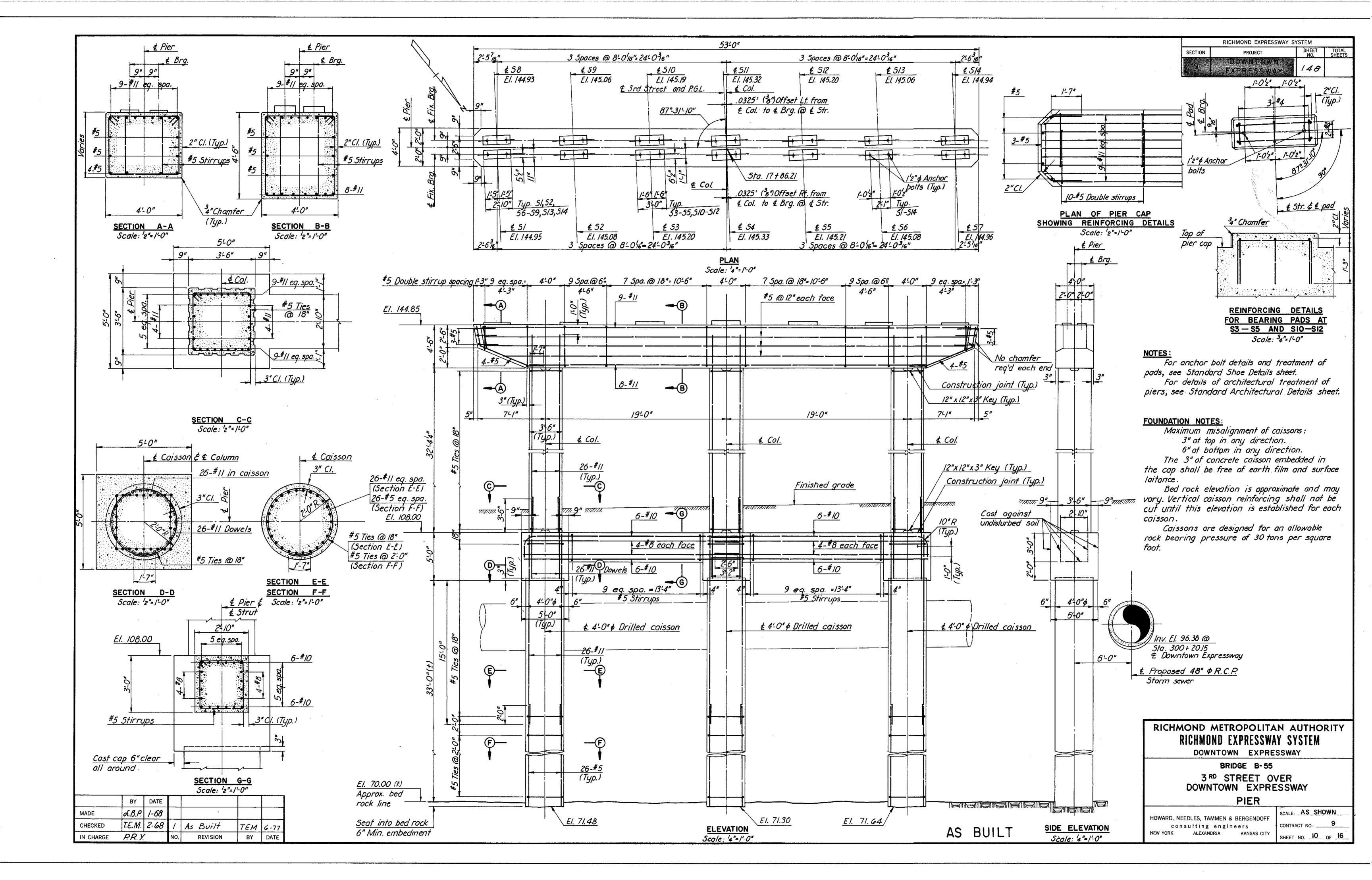
BRIDGE B-55 3 RD STREET OVER DOWNTOWN EXPRESSWAY

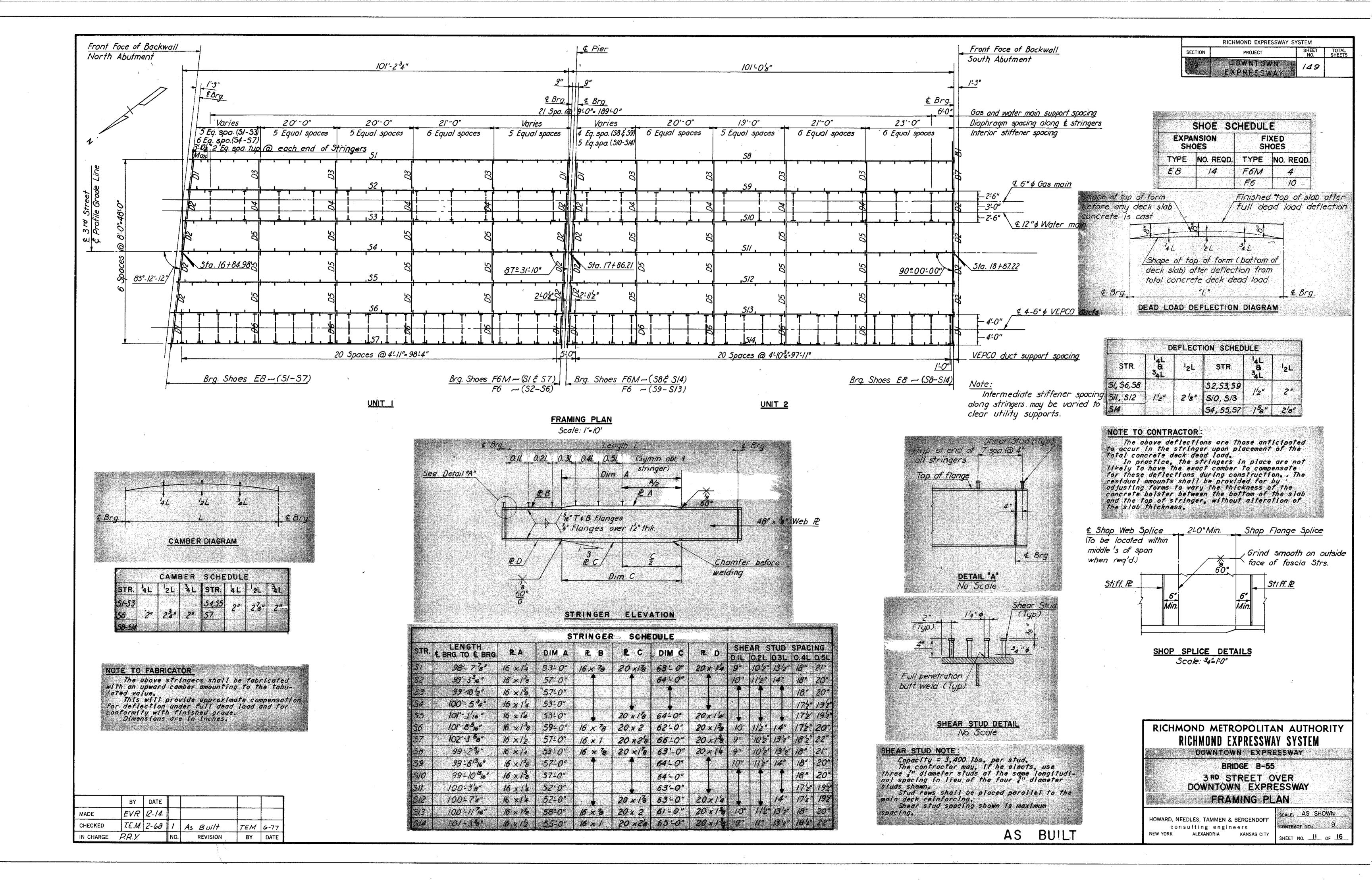
GENERAL NOTES AND QUANTITIES

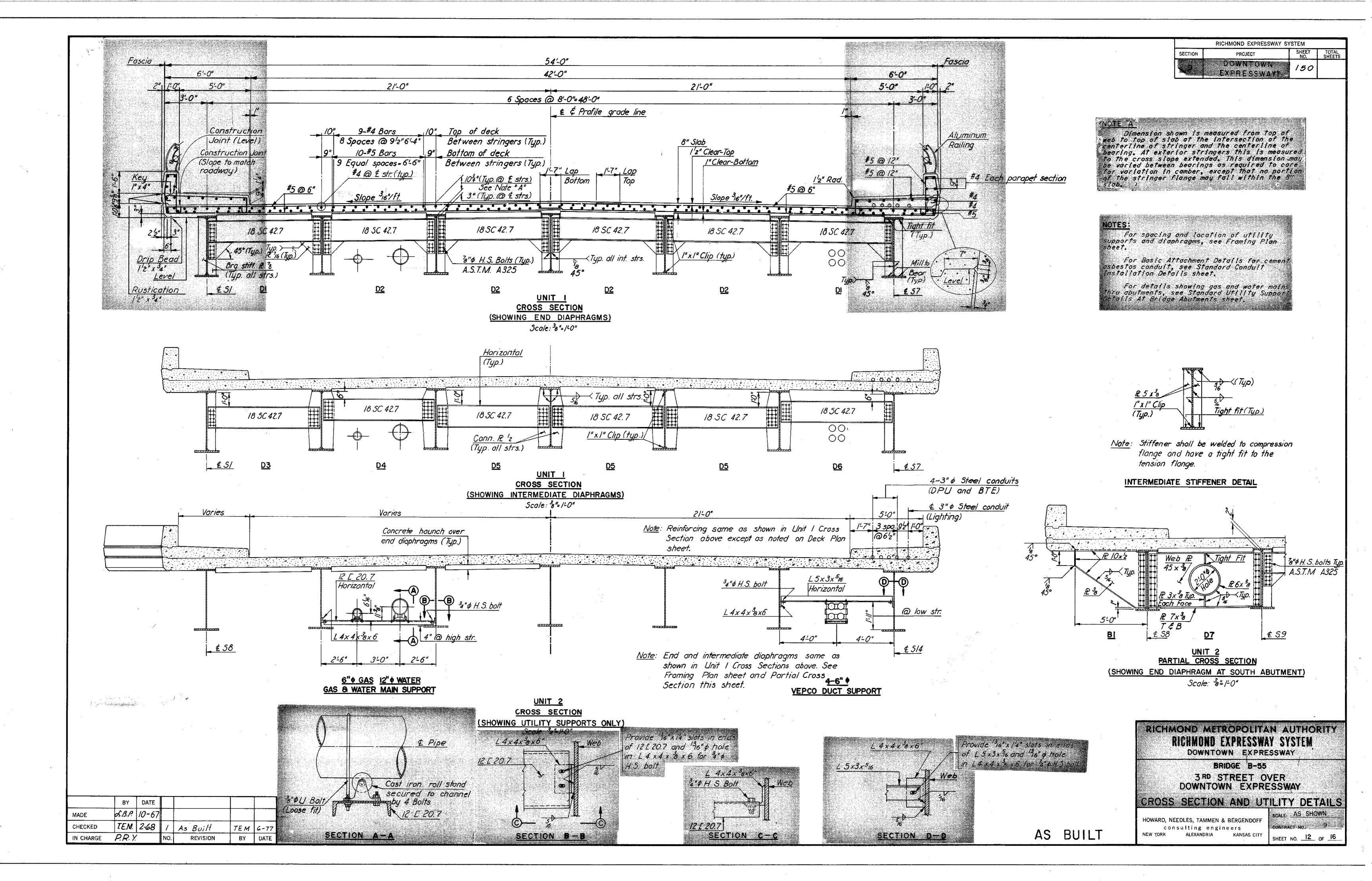
HOWARD, NEEDLES, TAMMEN & BERGENDOFF

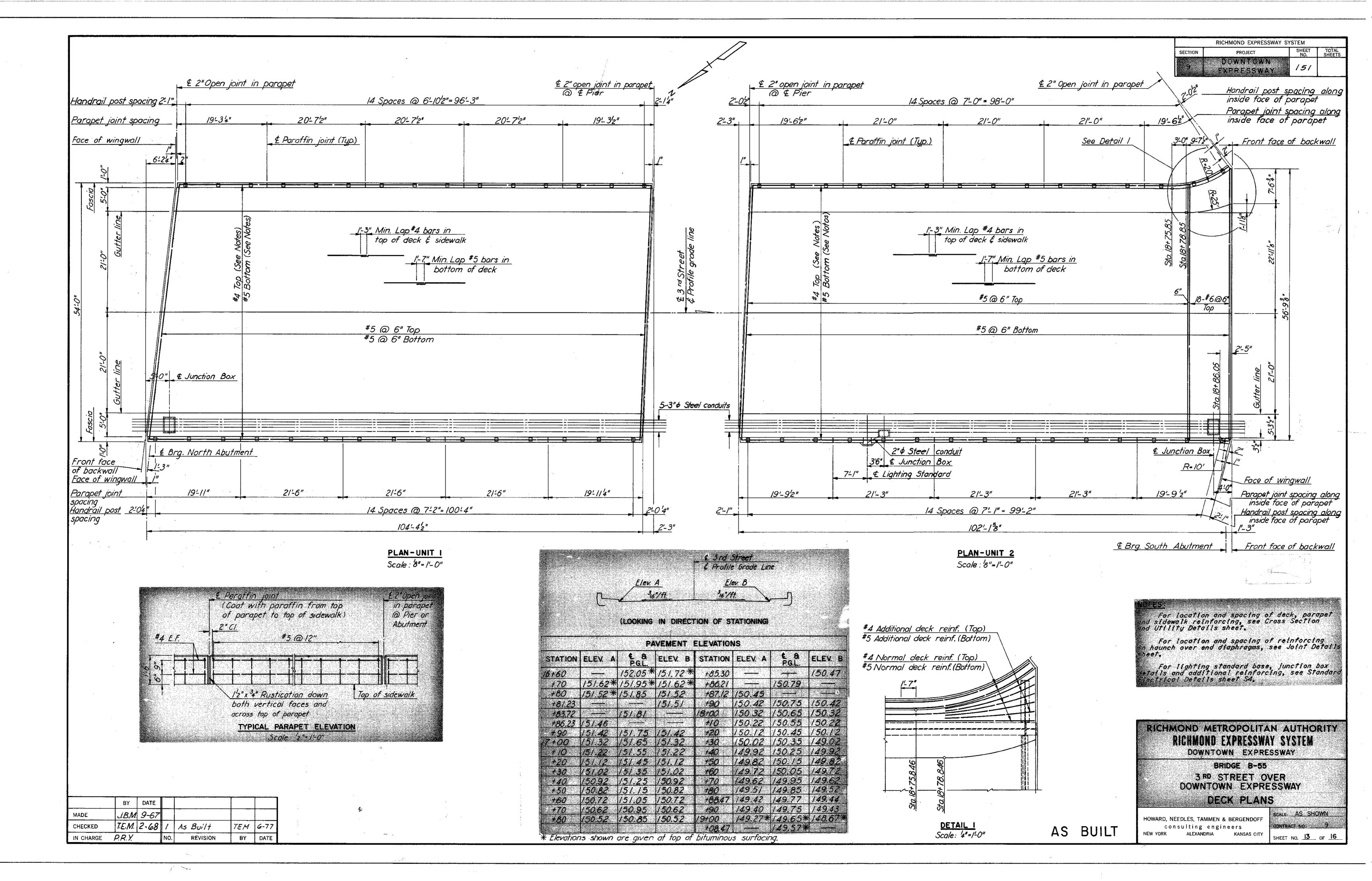
consulting engineers ORK ALEXANDRIA KANSA

AS BUILT

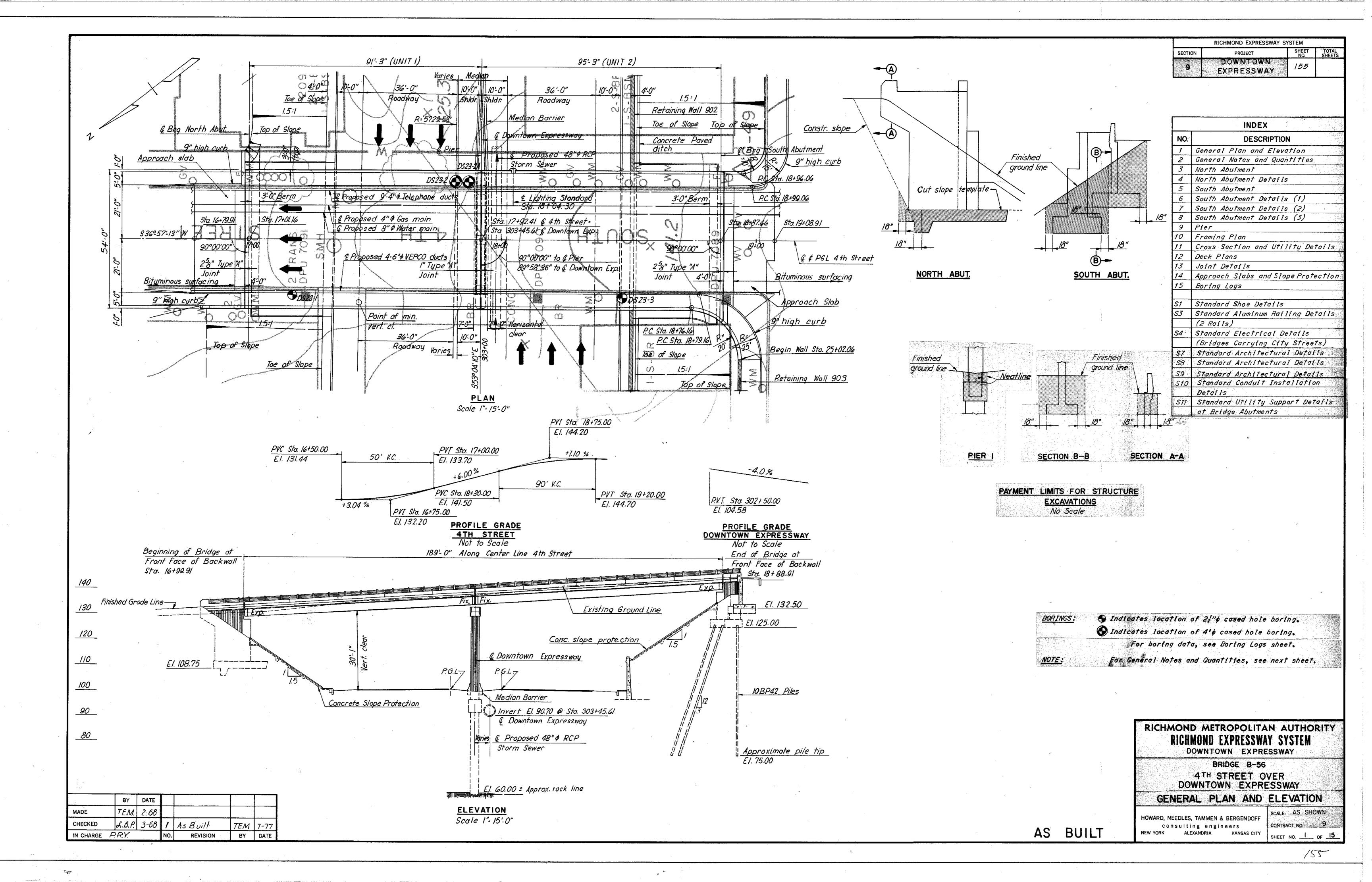








4<sup>th</sup> Street
Over
Downtown Expressway (VA 195)



One 42'-0" clear roadway. Two 5'0' sidewalks.
Dead Load-Includes 15 lbs. per sq.ft. for
future wearing surface.
Live Loads-HS20-44 loading and B.P.R. modified
for military vehicles. SPECIFICATIONS: GENERAL-Virginia Department of Highway Road and Bridge Specifications, 1970. DESIGN-A.A.S.H.O. Standard Specifications for Highway Bridges, 1961, modified by Special Design Provisions. WELDING-1969 Standard Specifications for Welded Highway and Railway Bridges of the American Welding Society. CONTRACT SPECIAL PROVISIONS Specifications and Contract Special Provisions referred to above are necessary to make these plans complete. <u>DATUM:</u> CITY OF RICHMOND TEMPERATURE: The normal temperature referred to on the plan is 60°F. The temperature range for movement is 0°F to 120°F. DIMENSIONS: All dimensions are measured horizontally and vertically unless otherwise noted. Excavation below subgrade and cut slope template shall be classified as Structure Excavation. All excavation phove these limits shall be classified as Regular Excavation and is not included in the Structural Quantities. EXCAVATION: Footings shall rest on firm material.
Foundation material shall be kept dry and special attention is called to Sections 401.05 and 401.06 of the General Specifications and to the Contract Special Provisions concerning preparation of foundations for footings.

CONCRETE NOTES: Concrete in superstructure shall be Class A4. All other concrete shall be Class A3. All exposed edges and corners shall have a 3/4" chamfer or fillet unless otherwise noted. Care in the method of vibration, the use of low-slump concrete, and/or other means shall be employed to prevent downgrade movement of newly placed slab concrete (when gradient is over 2%). Finishing Concrete Surfaces: See the Standard Architectural Details sheets and the Contract Special Provisions for types and details. All reinforcing steel shall conform to ASTM A615 Grade 40.
All reinforcing bar dimensions on the detailed drawings are to centers of bars unless otherwise noted. Clear distance between reinforcing steel and face of concrete shall be as noted on the plans. All bar laps shall be 30 diameters of the smaller diameter bar unless otherwise noted. Structural steel shall conform to A.S.T.M. Speci-fication A36 except as noted. STEEL NOTES: All field connections shall be made with high strength bolts. High strength boits shall be 7/8" diameter unless otherwise noted and shall conform to A.S.T.M.
Specifications A325. See Reference Ties and Field Control Data sheet in highway plans. Copper weld rod, corner E. Canal St. and S. 4th St., Elev. 129.23 Copper weld rod, corner Byrd St. and S. 4th St., Elev. 143.97

									FINAL	QUANTIT	IES								***.
	STRUCTURE EXCAVATION C.Y. *	CONCRETE CLASS A3 CY	CONCRETE CLASS A3 BR. APPR SLABS C Y	CONCRETE CLASS A4 CY	REINFORCING STEEL LBS	STRUCTURAL STEEL LBS	ALUMINUM BR. RAILING (2 RAILS) LF.	DRILLED HOLES FOR CAISSONS 4'0 LF	POROUS BACKFILL CY	STEEL PILES 10BP42 LF	CONC SLAB SLOPE PROTECTION S.Y	DAMP- PROOFING S.Y	UNDERDRAIN 6"0 L.F	GAS MAIN 4'0 L.F.	WATER MAIN 8"Q L.F	CONDUIT 6' 0 Vepco L.F	CONDUIT  4"   TELEPHONE  L   F	METAL CONDUIT 3" Ø LF	
SUPERSTRUCTURE				352.49	78,469	346,311	380.1							237	234	924	2079	//85	D TO STAN WOODLAND
NORTH ABUTMENT	/52	263,40		,	26,390		14.7		83		250.99	/5/	7/						
PIER 1	41	132.70			36,71.0			76.88				·							
SOUTH ABUTMENT	526	247.23			17,593		37.6		34	2,666.1	250.92	156	101						
APPROACH SLABS			96.85		22,208														
TOTAL	719	<i>643.3</i> 3	96.85	352.49	181,370	346,3//	432.4	76.88	117	2,666.1	501.91	<b>3</b> 07	172	237	234	924	2079	/ 18 <del>5</del>	
		7.												Į:		m m mm.		:	

d.B.P. 12-68 1 As Built TEM 7-77 IN CHARGE PRY BY DATE

BRIDGE B-56 4TH STREET OVER DOWNTOWN EXPRESSWAY GENERAL NOTES AND QUANTITIES

HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers

RICHMOND METROPOLITAN AUTHORITY

RICHMOND EXPRESSWAY SYSTEM

DOWNTOWN EXPRESSWAY

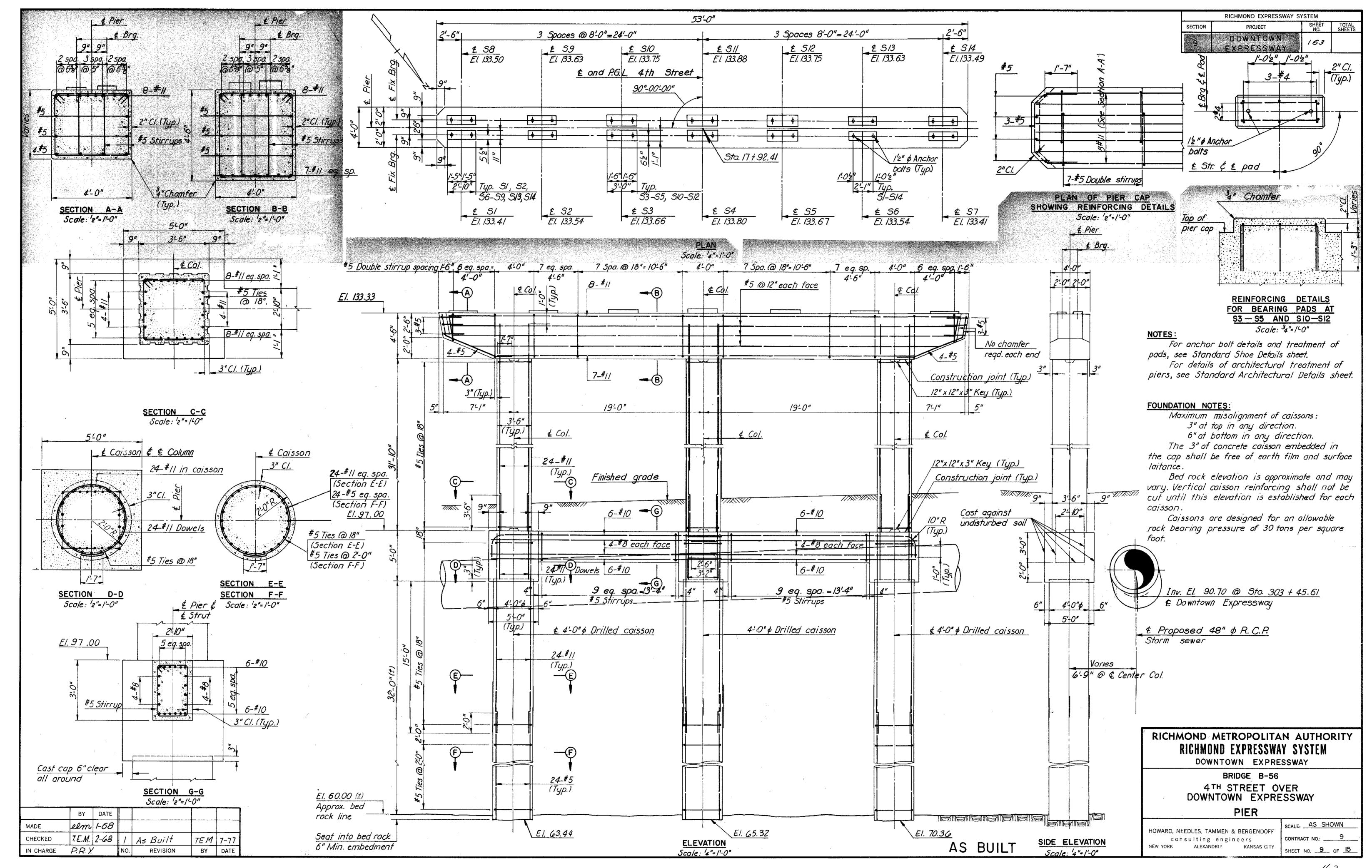
RICHMOND EXPRESSWAY SYSTEM

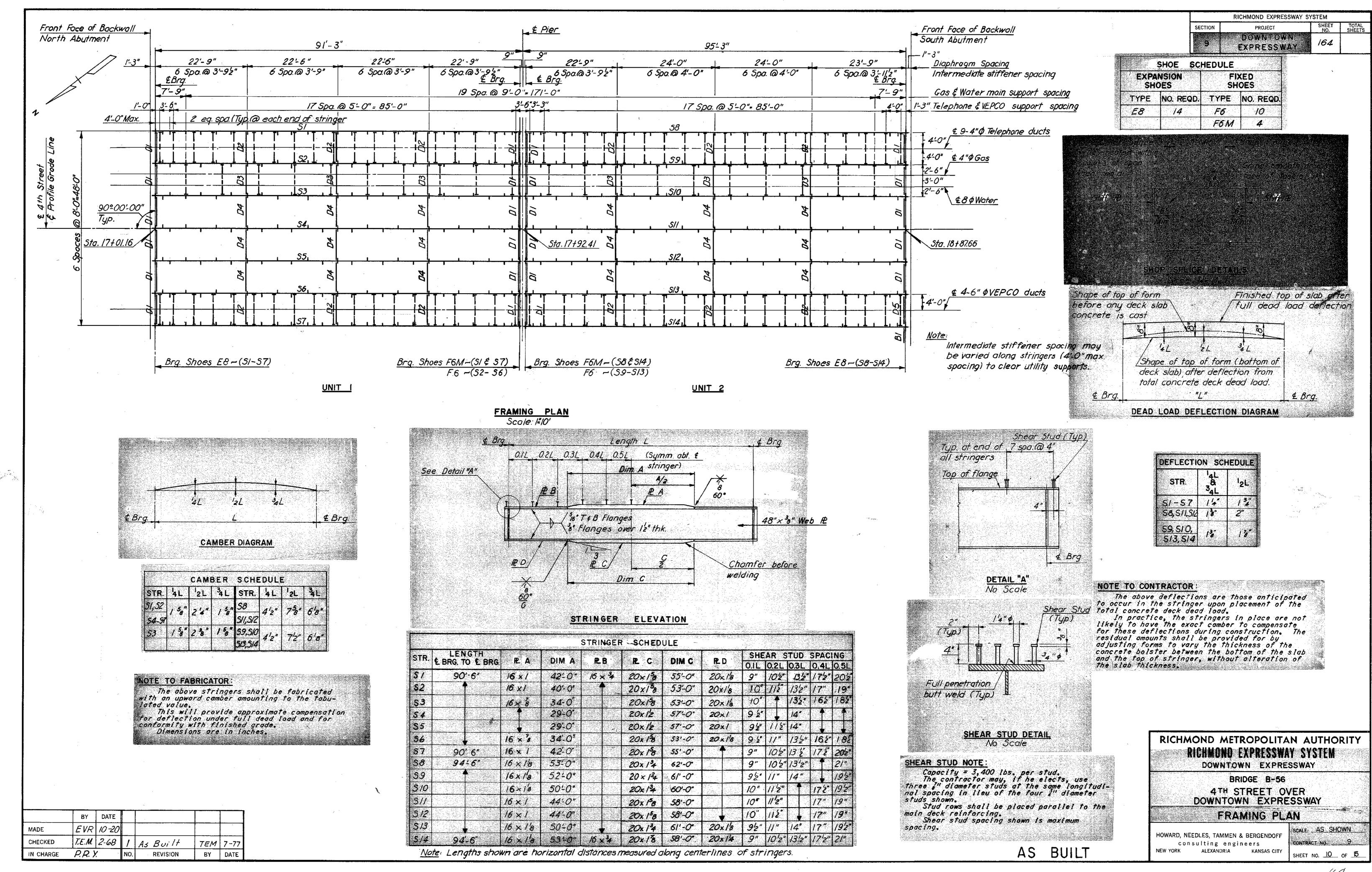
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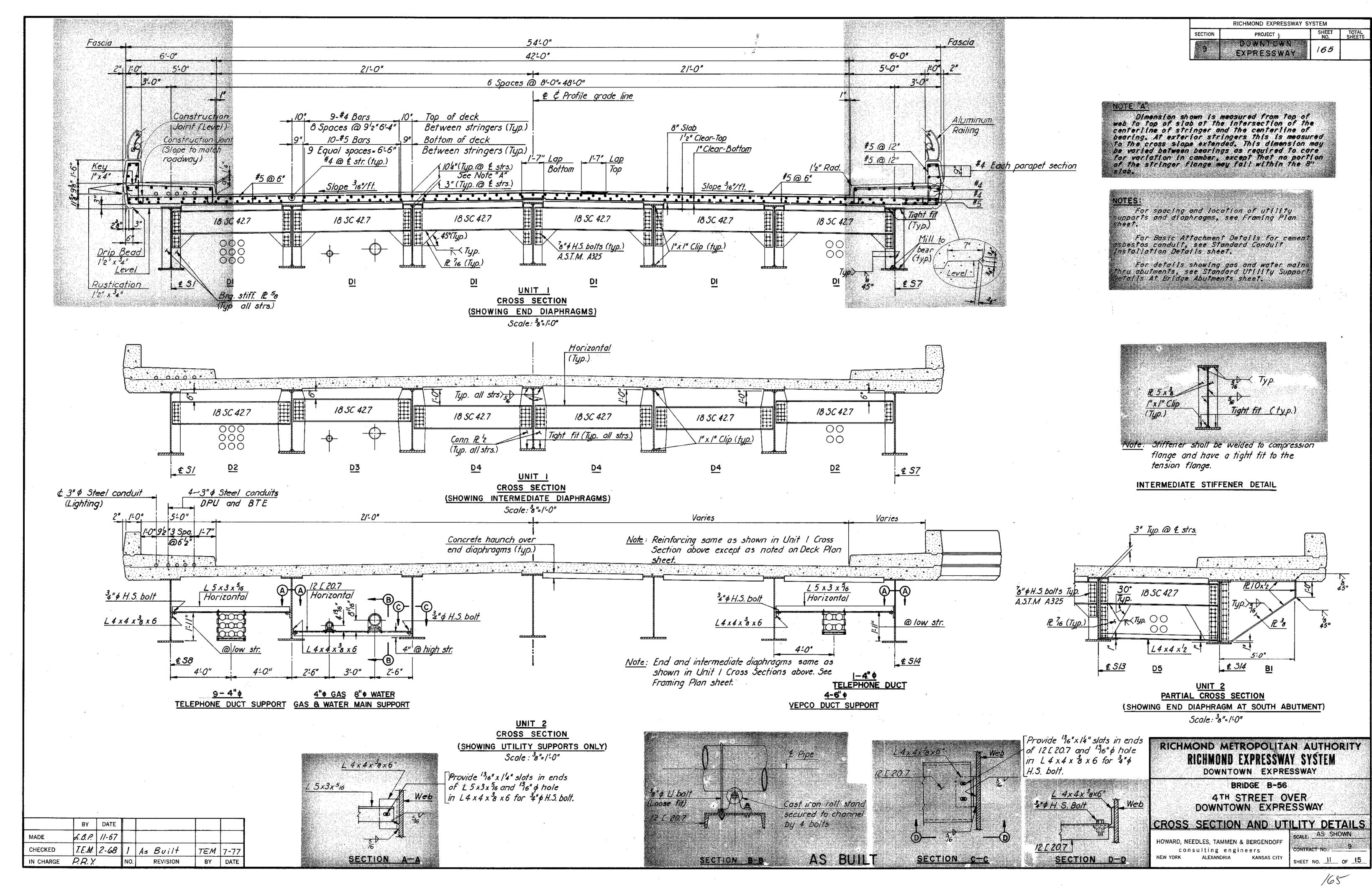
PROJECT

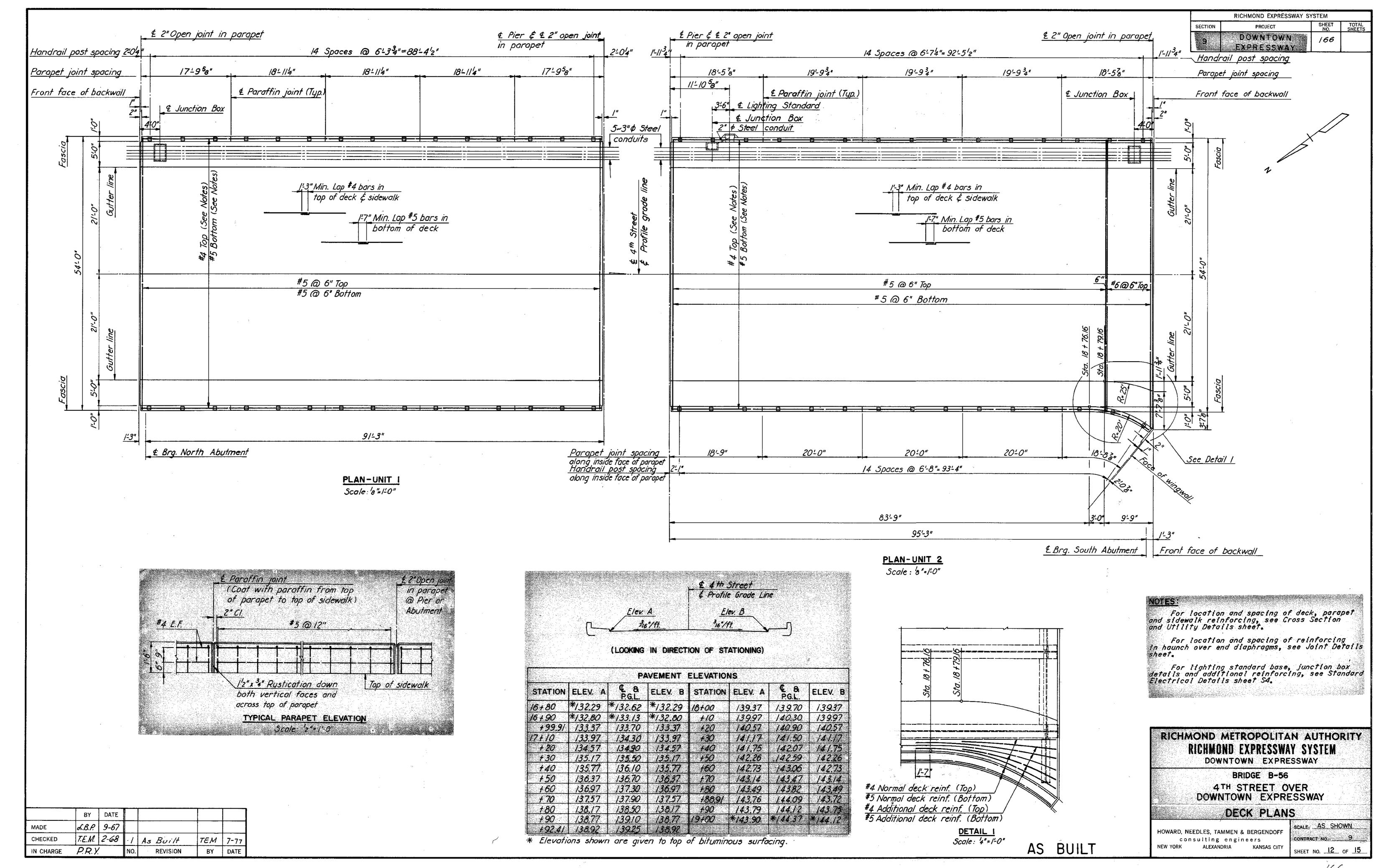
EXPRESSWAY

SECTION

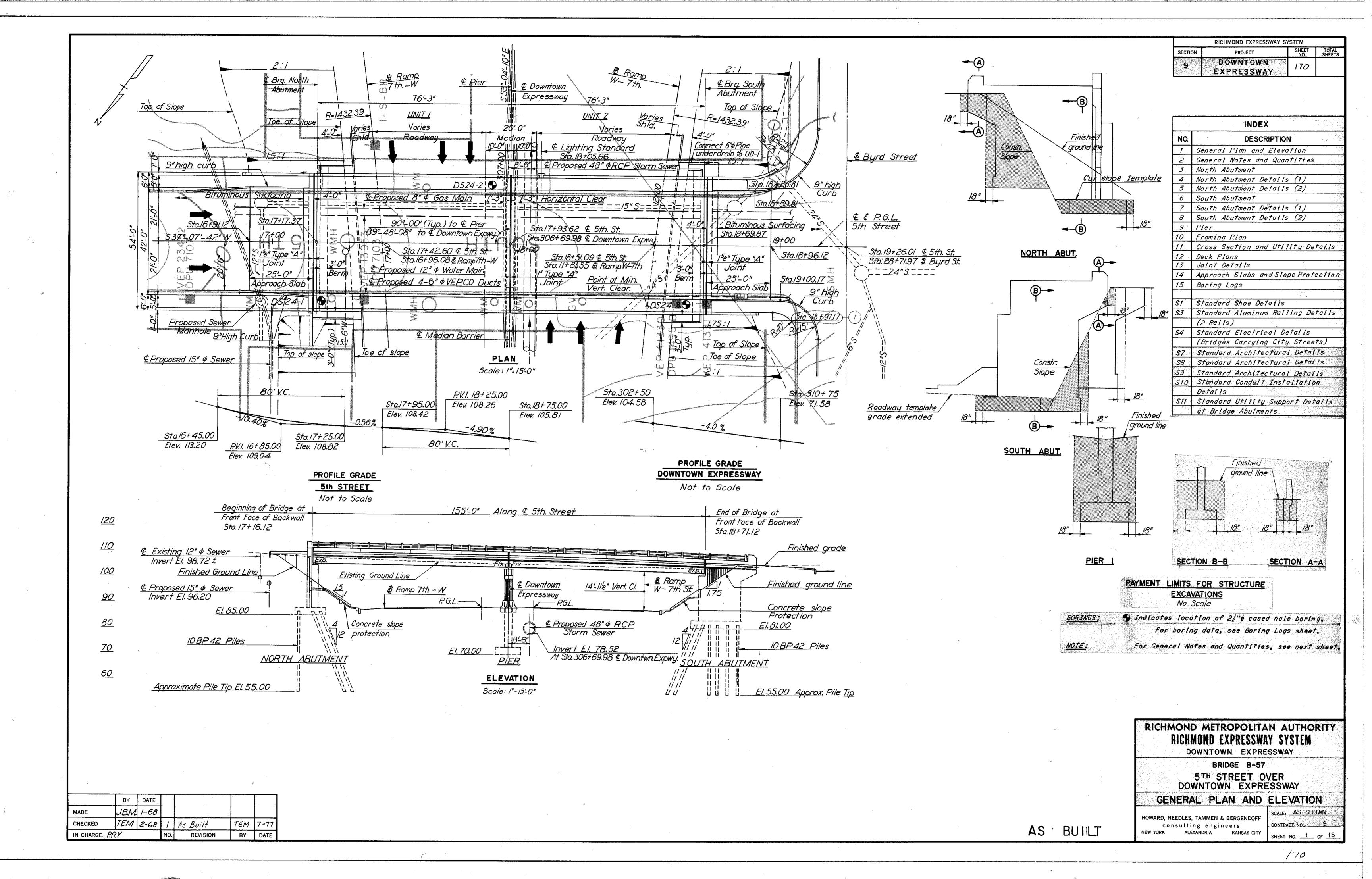


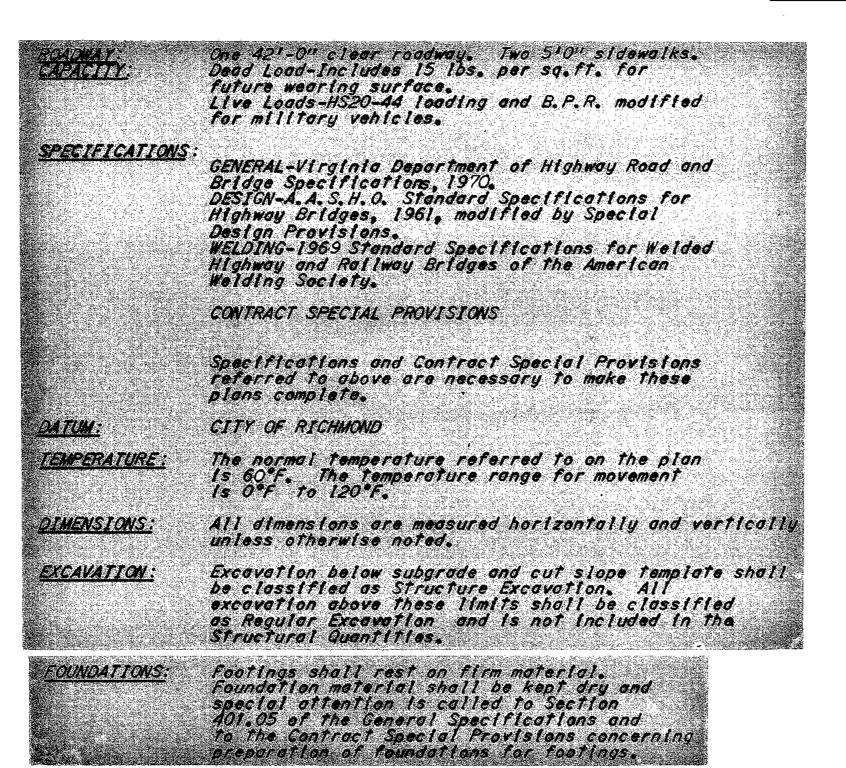


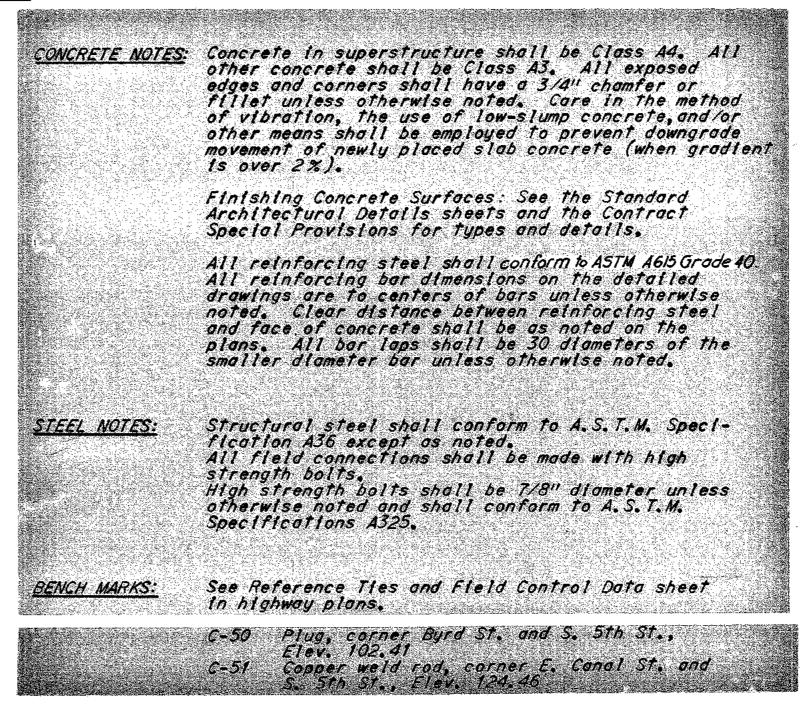




5<sup>th</sup> Street Over Downtown Expressway (VA 195)







					_		F	INAL	QUANTIT	IES							
	STRUCTURE EXCAVATION C.Y.	CONCRETE CLASS A3	GONGRETE CLASS A3 BF. APPR SLABS	CONCRETE CLASS A4	REINFORGING STEEL LBS	STRUCTURAL STEEL LBS	ALUMINUM BR. RAILING (2 RAILS) LF	POROUS BACKFILL C:Y	STEEL PILES (OBP42 LF	CONC. SLAB SLOPE PROTECTION S.Y	DAMP- PROOFING S.Y	UNDERDRAIN 6" () L.F.	GAS MAIN 8"0 LF	WATER MAIN 12"¢	CONDUIT 6"\$ VEPCO * L.F	METAL Conduit 3"0 Le	
SUPERSTRUCTURE				286.22	63,335	300,021	309,4			on (e.g.) (hallot has been a law interest and the second s			213	2/0	828	1065	
NORTH ABUTMENT	20/	251.08			25,224		24.6	76	1,254.2	110.73	148	106					
PIER I	440	123.74			24,376												
SOUTH ABUTMENT	/52	280.89			29,631		42.3	92	9.75.8	8.7.19	171	//3					
APPROACH SLABS			//5.3		25,732		,										<u> </u>
TOTAL	793	655.37	//5.3	286.22	168,298	300,021	376.3	168	2,230.0	197.92	3/9	219	213	210	828	1065	: : :
			. '				<u>;</u>										

	ВҮ	DATE				
MADE	EVR	3-68				
CHECKED	d.B.P.	5-68	1	As Built	TEM	7-77
IN CHARGE	PRX		NO.	REVISION	BY	DATE

AS BUILT

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RICHMOND EXPRESSWAY SYSTEM

PROJECT

DOWNTOWN EXPRESSWAY

SECTION

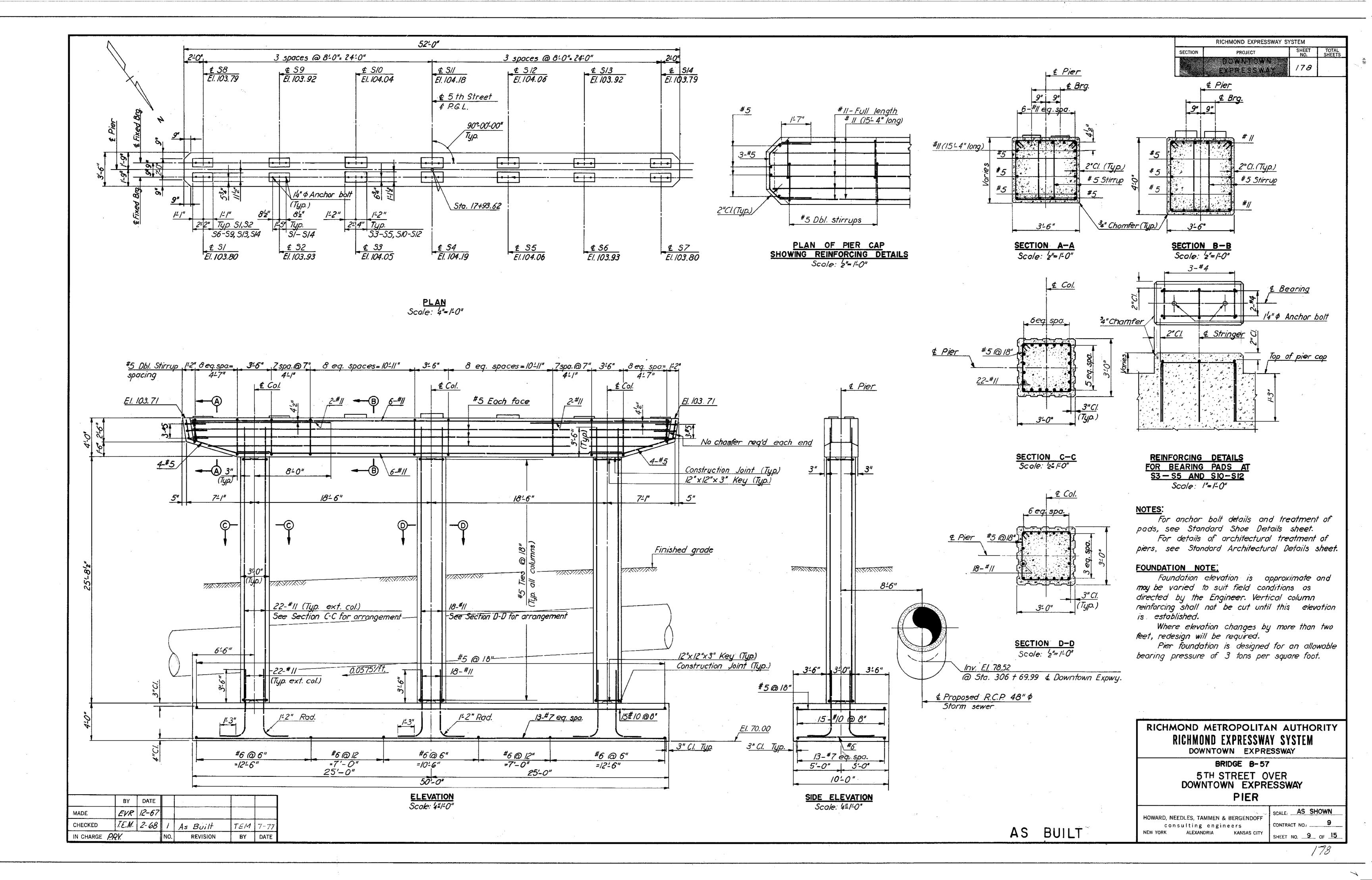
5TH STREET OVER DOWNTOWN EXPRESSWAY

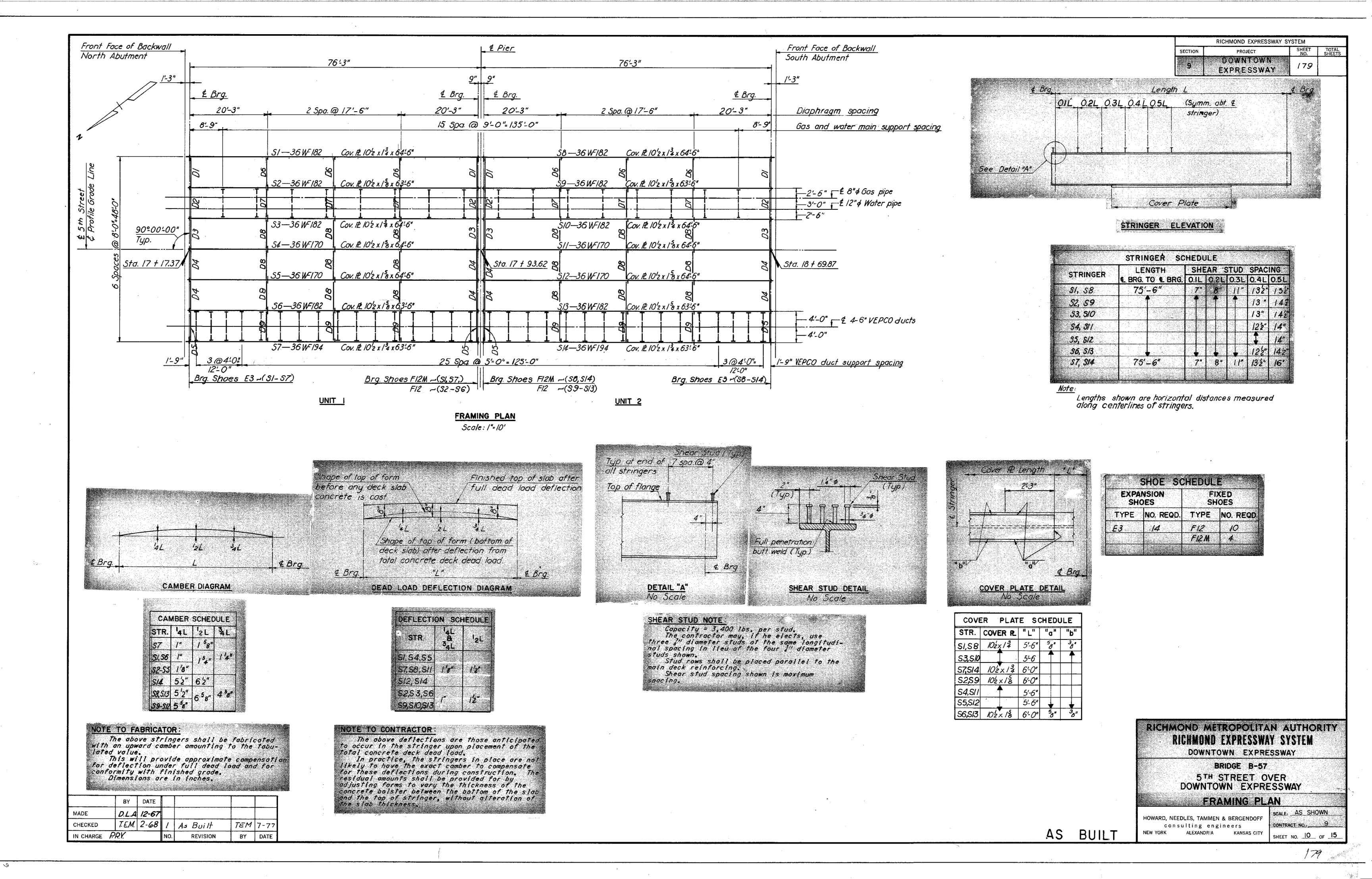
WARD, NEEDLES, TAMMEN & BERGENDOF
consulting engineers

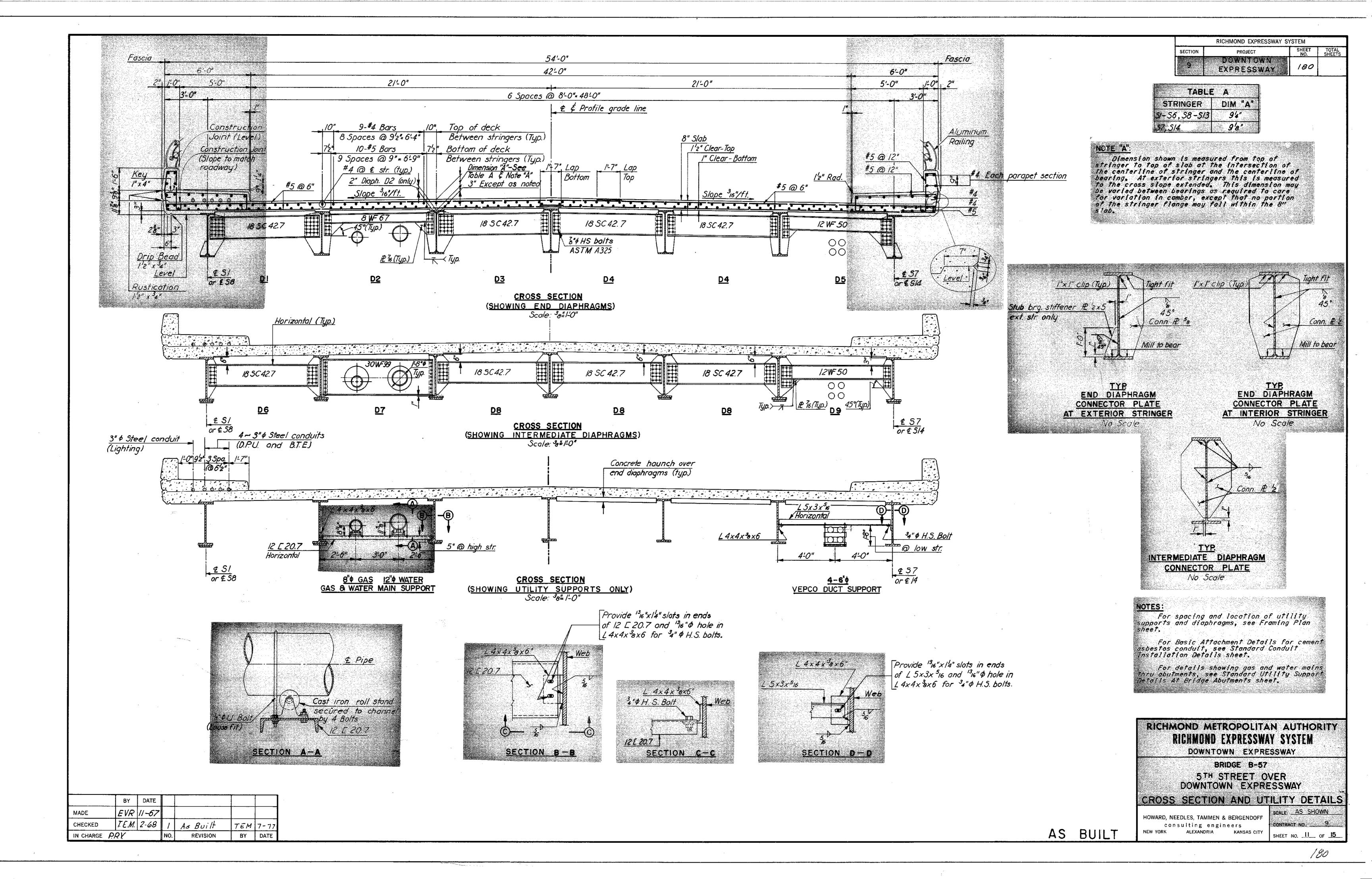
SCALE: A3 300VN

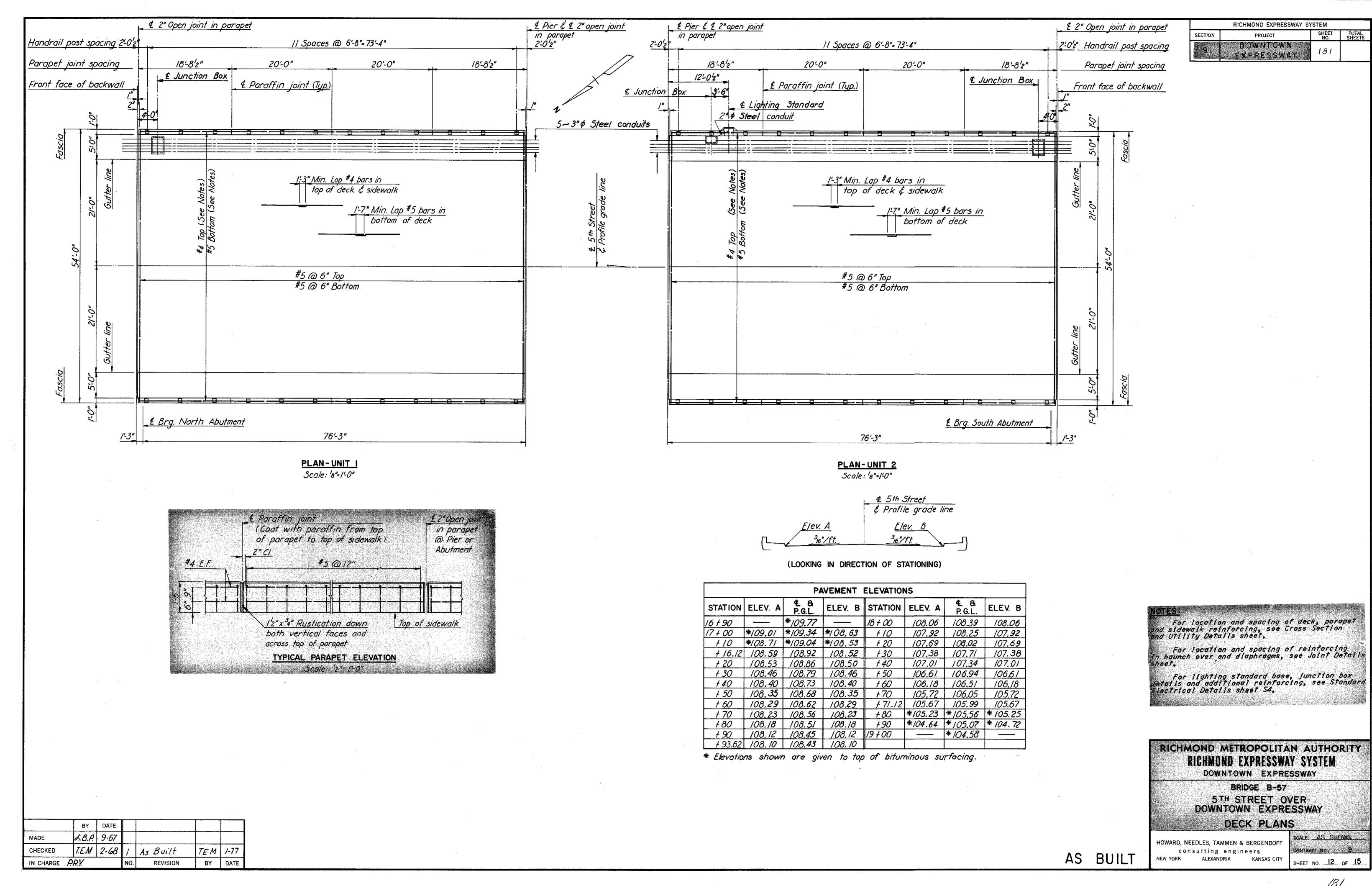
CONTRACT NO. 9

SHEET NO. 2 OF 15

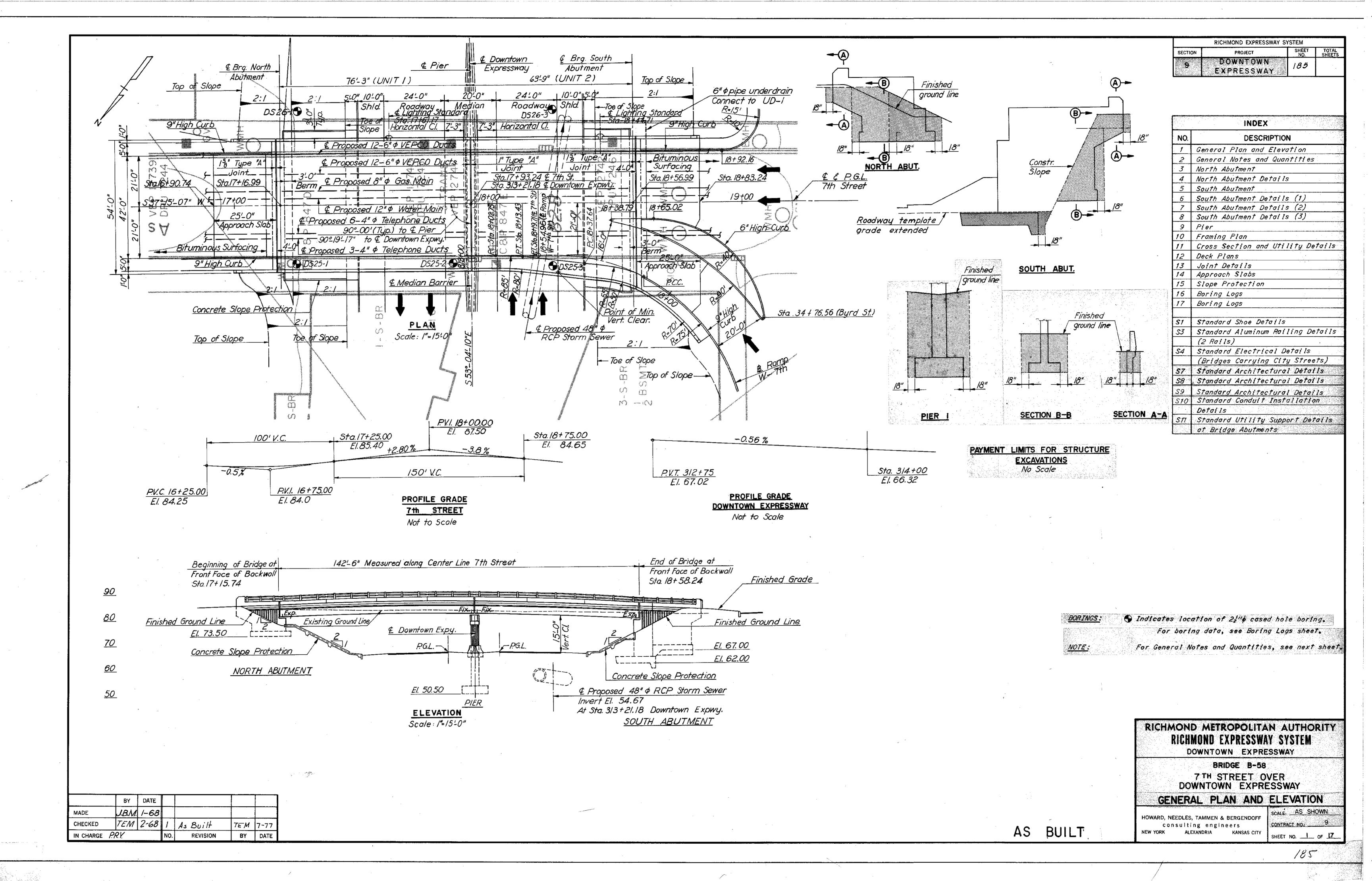








7<sup>th</sup> Street Over Downtown Expressway (VA 195)



FINAL

POROUS

BACKFILL

/3

96

109

259

344.2

214

200.5

CAPACITY:

Dead Load-Includes 15 lbs, per sq.ft. for future weating surface. Live Loads-HS20-44 loading and B.P.R. modified for military vehicles.

SPECIFICATIONS:

GENERAL-Virginto Department of Highway Road and Bridge Specifications, 1970.
DESIGN-A.A.S. H. D. Stendard Specifications for Highway Bridges, 1961, modified by Special Design Provisions.
WELDING-1969 Standard Specifications for Welded Highway and Reliewsy Bridges of the American Welding Society.

CONTRACT SPECIAL PROVISIONS

Specifications and Contract Special Provisions referred to above are necessary to make these plans complete.

DATUM:

CITY OF RICHMOND

IEMPERATURE: The normal temperature referred to an the plan is 60°F. The temperature range for movement is 0°F to 120°F.

DIMENSIONS: All dimensions are measured horizontally and vertically unless otherwise noted.

EXCAVATION:

Excavation below subgrade and cut slope template shall be classified as Structure Excavation. All excavation above these limits shall be classified as Regular Excavation and is not included in the Structural Quantities.

Footings shall rest on firm material.
Foundation material shall be kept dry and special attention is called to Section 401.05 of the General Specifications and to the Contract Special Provisions concerning preparation of foundations for footings.

CONCRETE CONCRETE REINFORCING STRUCTURAL ALUMINUM

60,408

5,818

23,306

29,430

30,322

149,284

STEEL

270,998

270,998

288.5

27.2

51.5

367.2

CLASS A3 CLASS A4 STEEL

276.41

276.41

EXCAVATION CLASS A3

121.19

353.77

586.36

112.44

112.44

262

402

1,110

SUPERSTRUCTURE

NORTH ABUTMENT

SOUTH ABUTMENT

APPROACH SLABS

<u>CONCRETE NOTES:</u>	Concrete in superstructure shall be Class A4. All other concrete shall be Class A3. All exposed edges and corners shall have a 3/4" chamfer or fillet unless otherwise noted. Care in the method of vibration, the use of low-slump concrete, and/or other means shall be employed to prevent downgrade
	movement of newly placed slab concrete (when gradien is over 2%).  Finishing Concrete Surfaces: See the Standard Architectural Details sheets and the Contract
	Special Provisions for types and details.
	All reinforcing steel shall conform to ASTM A615 Grade 40 All reinforcing bar dimensions on the detailed drawings are to centers of bars unless otherwise noted. Glear distance between reinforcing steel and face of concrete shall be as noted on the plans. All bar laps shall be 30 diameters of the smaller diameter bar unless otherwise noted.
STEEL NOTES:	Structural steel shall conform to A.S.T.M. Speci- fication A36 except as noted. All field connections shall be made with high strength bolts.
	High strength bolts shall be 7/8" diameter unless otherwise noted and shall conform to A.S.T.M. Specifications A325.
<u>BENCH MARKS:</u>	See Reference Ties and Field Control Data sheet In highway plans.
	C-52 Plug, corner E. Canal St. and S. 7th St., Elev. 84,90 C-53 Plug, S. 7th St., Elev. 83.83

		C-53	Elev. 84. Plug, S. Elev. 83,	90 77h St., 83				
								·
QUANTITI	IES				,			
CONC. SLAB SLOPE PROTECTION S.Y	DAMP- PROOFING S.Y.	UNDERDRAIN 6" Ø L. F	GAS MAIN e'¢ Le	WATER MAIN 12"¢) L F	CONDUIT 6"\$ VEPCO L.F.	CONDUIT 4" \$ TELEPHONE LF	METAL CONDUIT 3" 0 LEF	STEEL PILE IOBP42 L.F.
			200.5	197.5	4,668	1,750.5	1,203	
213.2	54	78						:
/3/.0	205	/36						195.3

197.5

	<del></del>		_			
	BY	DATE				
MADE	EVR	3-68				
CHECKED	S.B.P.	12-68	1	As Built	TEM	7-77
IN CHARGE	P.R.Y.		NO.	REVISION	BY	DATE

RICHMOND EXPRESSWAY SYSTEM
DOWNTOWN EXPRESSWAY

BRIDGE B-58

7 TH STREET OVER
DOWNTOWN EXPRESSWAY

FOR EXPRESS WAY

FOR EXPRE

RICHMOND METROPOLITAN AUTHORITY

RICHMOND EXPRESSWAY SYSTEM

PROJECT

DOWNTOWN EXPRESSWAY

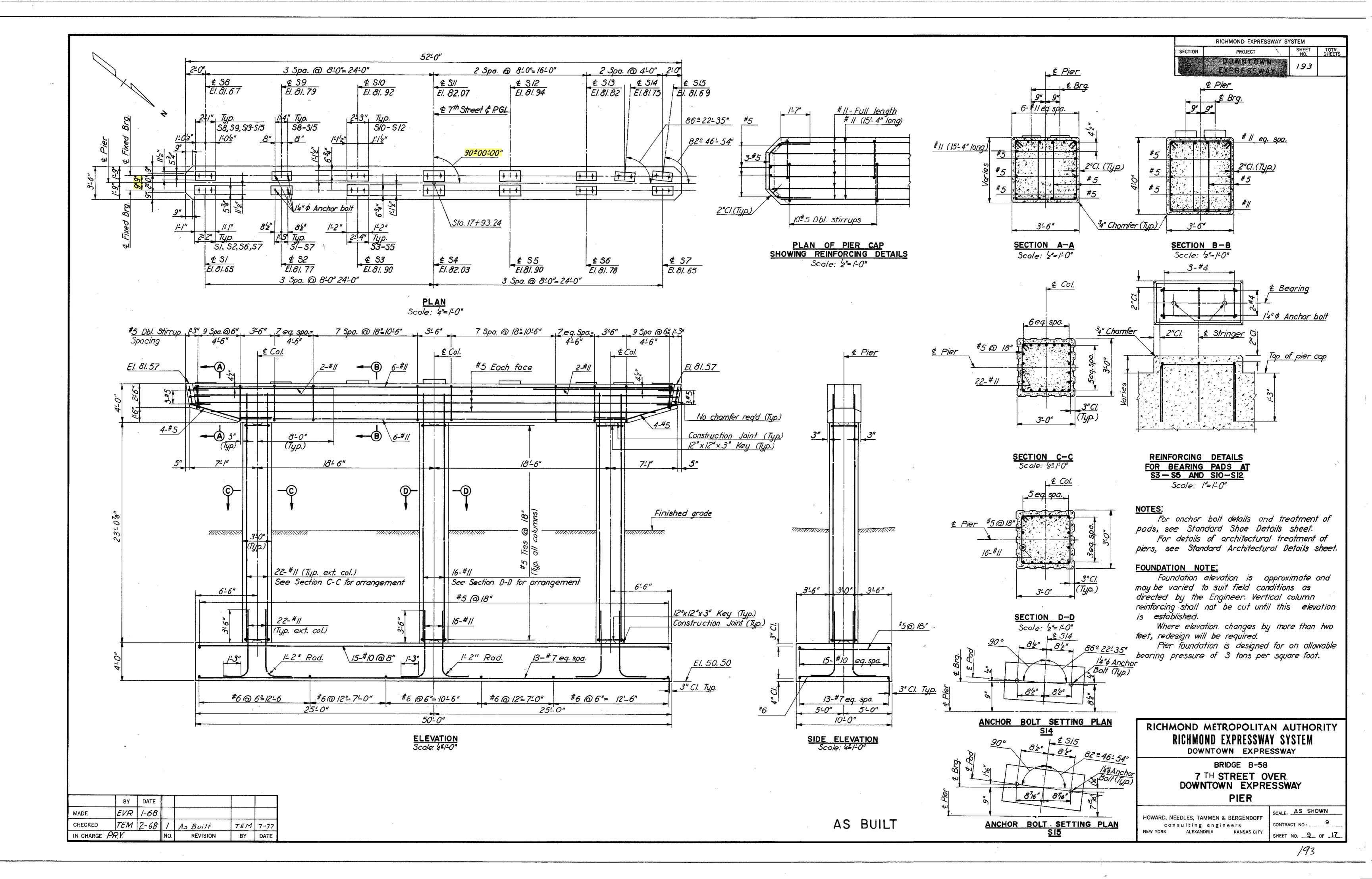
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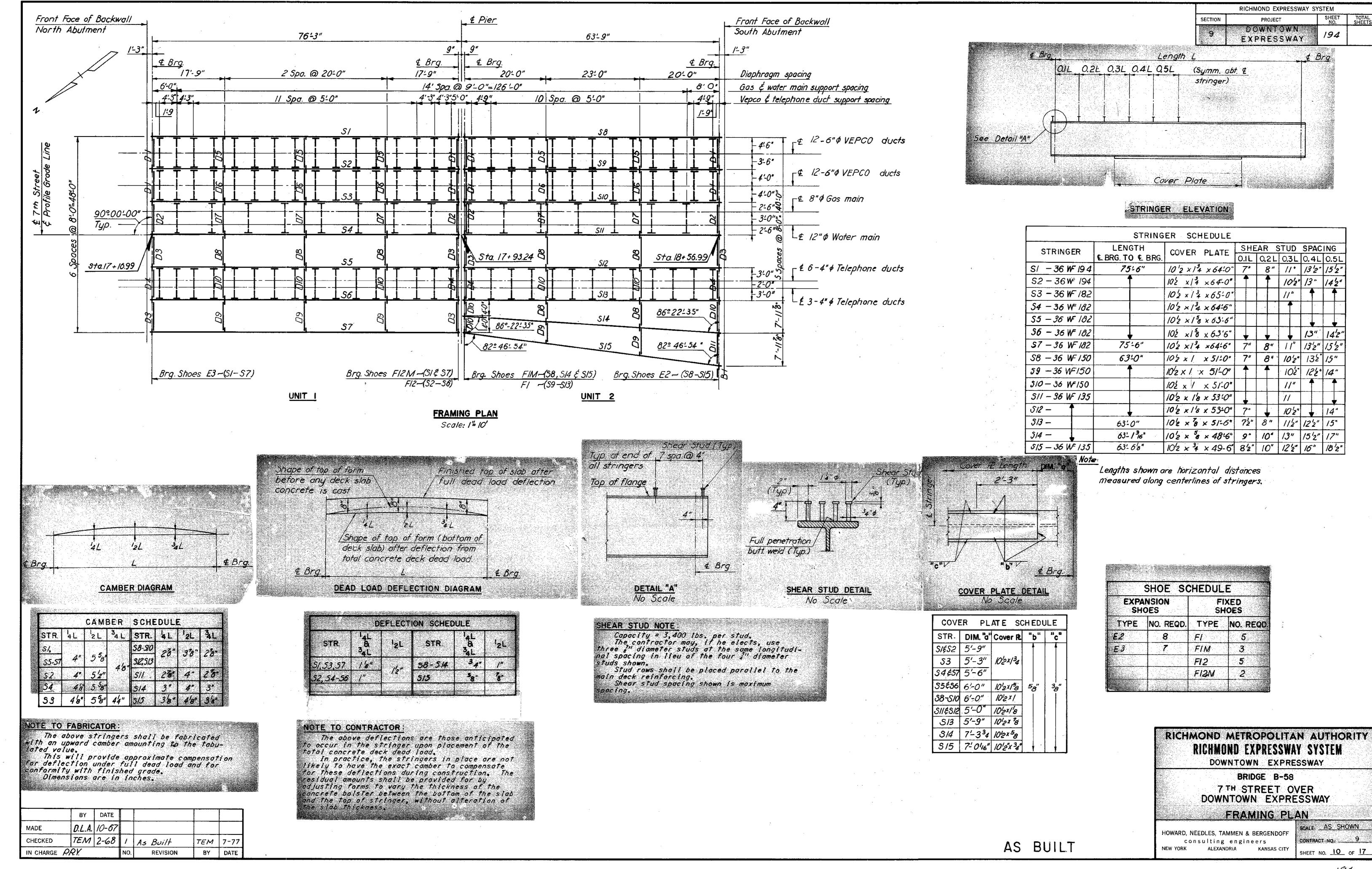
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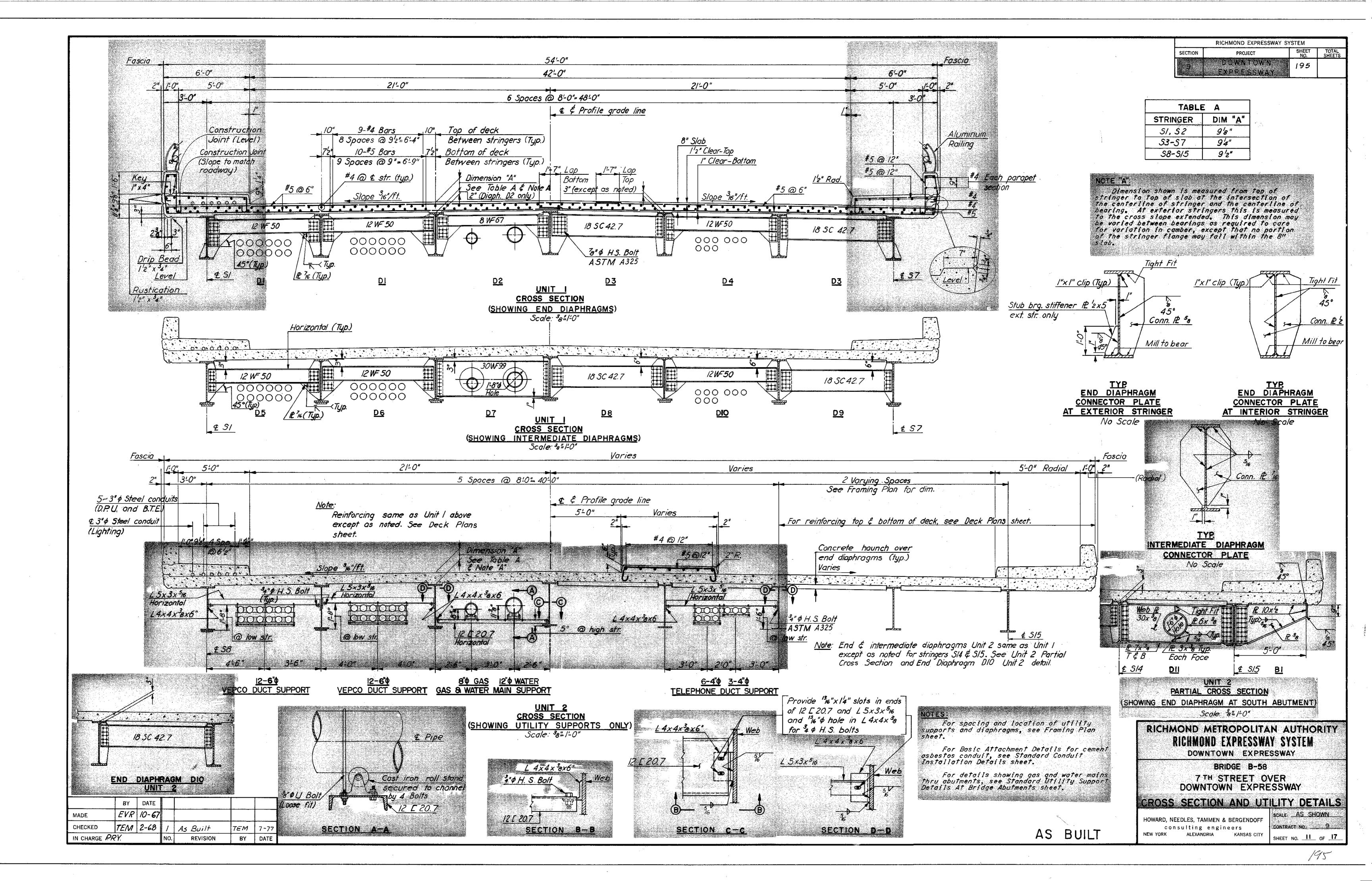
4,668

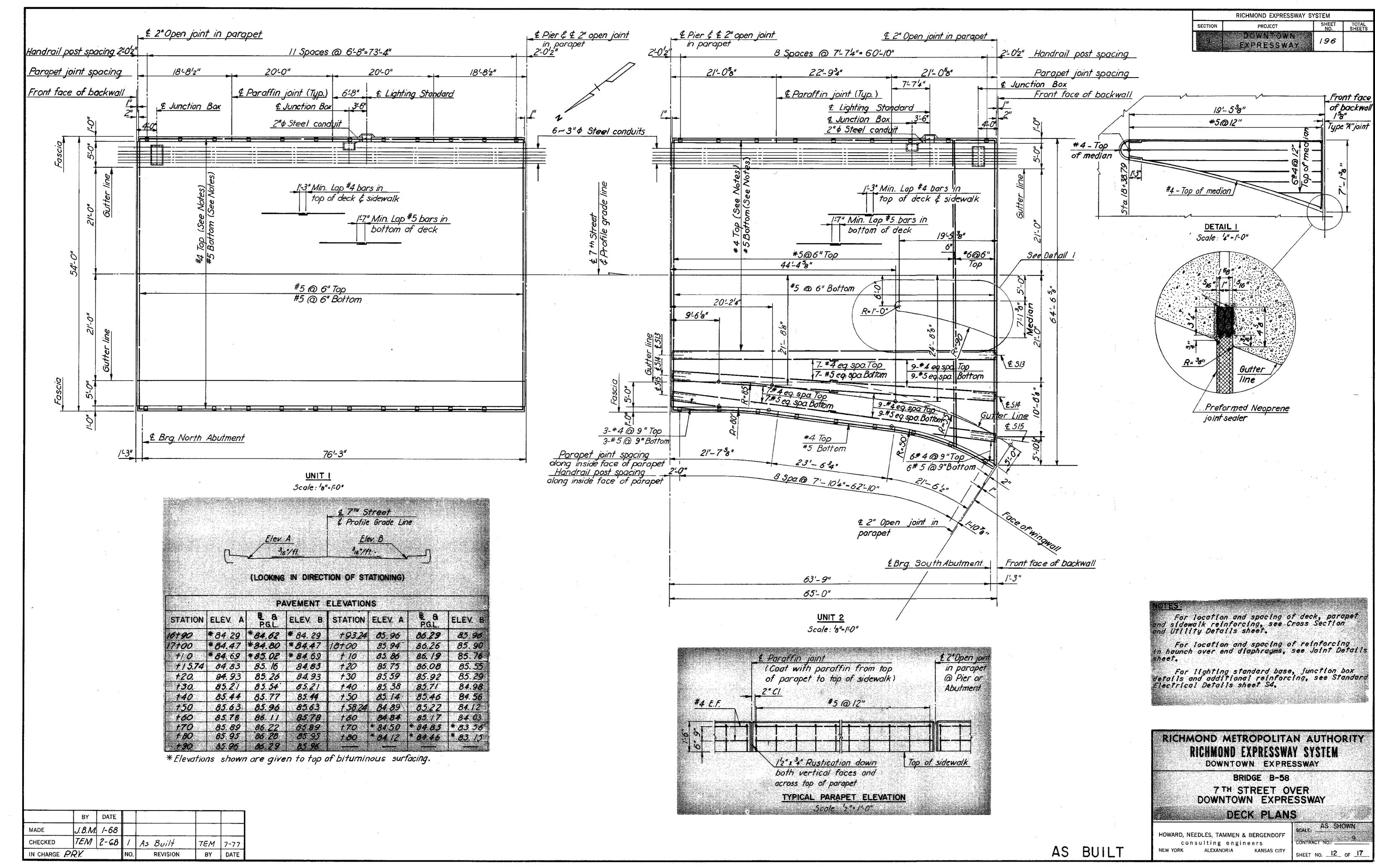
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195.3

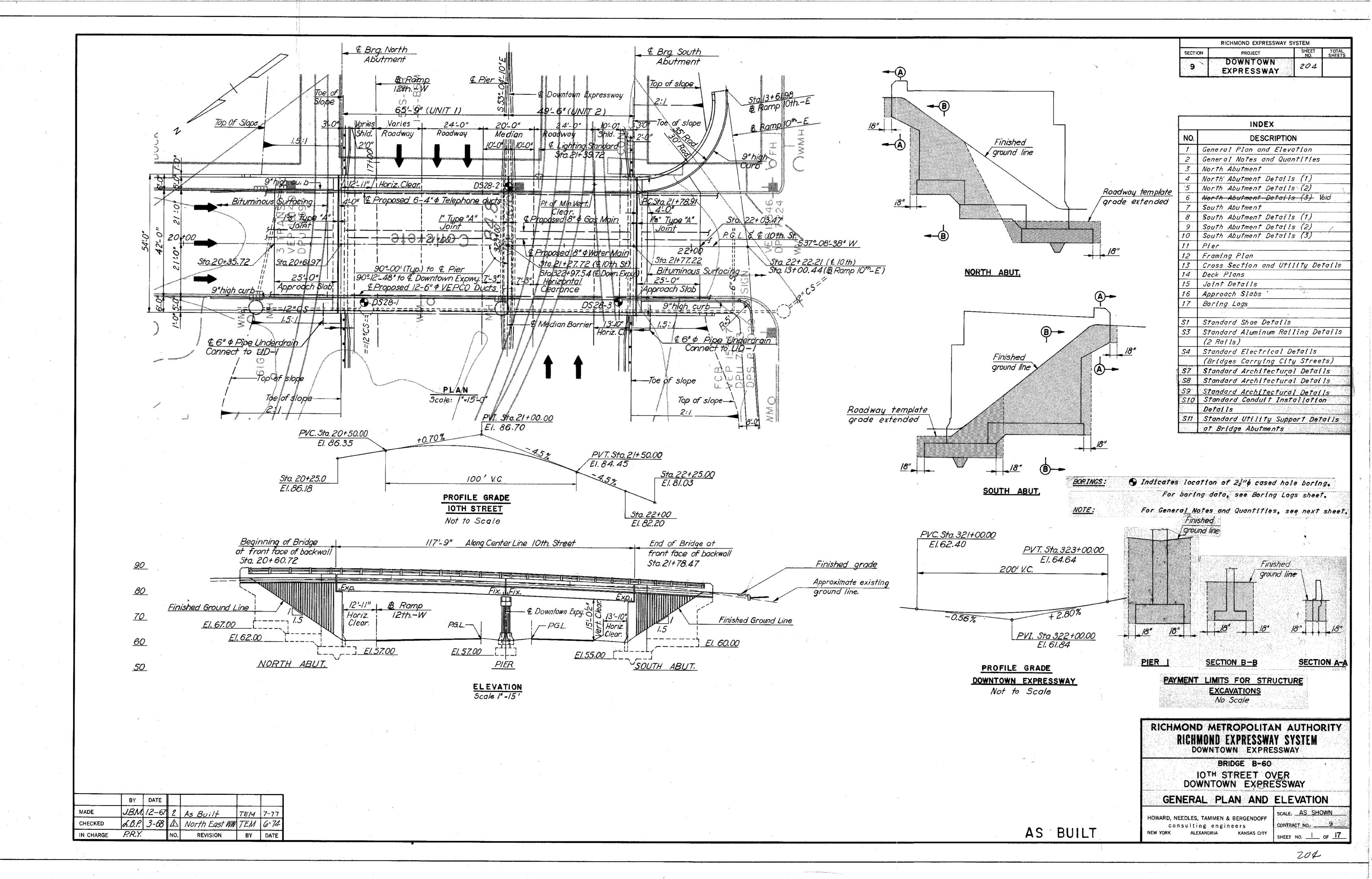








10<sup>th</sup> Street Over Downtown Expressway (VA 195)



CAPACITY.

One 42\*-0" clear roadway. Two 5\*0" slawwalks.

Dead Load-includes 15 lbs. per sq.ft. for future wearing surface.
Live Loads-HS20-44 leading and B.P.R. modified for military vehicles.

SPECIFICATIONS:

GENERAL-Virginia Department of Highway Road and Bridge Specifications, 1970.
DESIGN-A.A.S.H.O. Standard Specifications for Highway Bridges, 1961, modified by Special Design Provisions.
WELDING-1969 Standard Specifications for Welded Highway and Railway Bridges of the American Welding Society.

CONTRACT SPECIAL PROVISIONS

Specifications and Contract Special Provisions referred to above are necessary to make these plans complete.

DATUN: CITY OF RICHMOND

IEMPERATURE: The normal temperature referred to an the plan is 60%. The temperature range for movement is 0% to 120%.

DIMENSIONS: All dimensions are measured horizontally and vertically unless otherwise noted.

EXCAVATION: Excavation below subgrade and cut slape template shall be classified as Structure Excavation. All excavation above these limits shall be classified as Structural Quantities.

Footings shall rest on firm material.
Foundation material shall be kept dry and special attention is called to Section 401.05 of the General Specifications and to the Contract Special Provisions concerning preparation of foundations for factings.

CONCRETE NOTES: Concrete in superstructure shall be Class A4. All exposed edges and corners shall have a 3/4" chamfer or fillet unless otherwise noted. Care in the method of vibration, the use of low-slump concrete, and/or other means shall be employed to prevent downgrade movement of newly placed slab concrete (when gradient is over 2%).

Finishing Concrete Surfaces: See the Standard Architectural Details sheets and the Contract Special Provisions for types and details.

All reinforcing steel shall conform IoASIM A615 Grade 40 All reinforcing bar dimensions on the detailed drawings are to centers of bars unless otherwise noted. Clear distance between reinforcing steel and face of concrete shall be as noted on the plans. All bar laps shall be 30 diameters of the smaller diameter bar unless otherwise noted.

SITEFL NOTES:

Structural steel shall conform to A.S.T.M. Specification A36 except as noted.

All field connections shall be made with high strength boits.

High strength boits shall be 7/8" diameter unless otherwise noted and shall conform to A.S.T.M.

Specifications A325.

BENCH MARKS:

See Reference Ties and Field Control Data sheet in highway pidns.

C-56 Plug, corner E. Cary St. and S. 10th St.,

FINAL QUANTITIES																	
	STRUCTURE EXCAVATION C.Y.	EA PPAID	CONCRETE CLASS A3 BR. APPR. SLABS C.Y	CLASS A4	REINFORCING STEEL LBS:	STRUCTURAL STEEL LBS.	ALUMINUM BR. RAILING (2 RAILS) L.F	POROUS BACKFILL C Y	DAMP- PROOFING S.Y	'UNDERDRAIN 6" () L. F	GAS MAIN 8"0 LF	WATER MAIN 8'0 L.F.	CONDUIT 6"0 VEPCO L.F	CONDUIT 4" ¢ TELEPHONE L.F	METAL CONDUIT 3" 0		
SUPERSTRUCTURE				229.19	48,894	176,516	235.3				171	173	2037	1018.5	879.,		
NORTH ABUTMENT	1418	, 445.53			35,7//		, 69.2	129	272	1,32							
PER I	194	8547		· · · · · · · · · · · · · · · · · · ·	18,598												
SOUTH ABUTMENT	<b>96</b> 0	441.36			35,887		75.8	/33	280	137							
APPROACH SLABS			1/8.03		26,018	<u> </u>											
TOTAL	2572	972.36	118.03	229.19	165,018	176,516	380.3	262	552	269	171	173	2037	1018.5	879		
				·													

 MADE
 EVR
 3-68
 2
 AS BUILT
 HMW
 5-76

 CHECKED
 S.B.P.
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 6-74

 IN CHARGE
 P.R.Y.
 NO.
 REVISION
 BY
 DATE

RICHMOND METROPOLITAN AUTHORITY
RICHMOND EXPRESSWAY SYSTEM
DOWNTOWN EXPRESSWAY

BRIDGE B-60
LOTH STREET OVER
DOWNTOWN EXPRESSWAY

GENERAL NOTES AND QUANTITIES

HOWARD, NEEDLES, TAMMEN & BERGENDOFF
CONSULTING ENGINEERS
NEW YORK ALEXANDRIA KANSAS CITY

SHEET NO. 2 OF 17

RICHMOND EXPRESSWAY SYSTEM

205

PROJECT

DOWNTOWN

EXPRESSWAY

SECTION

AS BUILT

